

University of Bath



PHD

Development of large scale triaxial apparatus for the determination of the shear and damping response of graded granular material subjected to cyclic loads

Ring, Stephen Gordon

Award date:
1993

Awarding institution:
University of Bath

[Link to publication](#)

General rights

Copyright and moral rights for the publications made accessible in the public portal are retained by the authors and/or other copyright owners and it is a condition of accessing publications that users recognise and abide by the legal requirements associated with these rights.

- Users may download and print one copy of any publication from the public portal for the purpose of private study or research.
- You may not further distribute the material or use it for any profit-making activity or commercial gain
- You may freely distribute the URL identifying the publication in the public portal ?

Take down policy

If you believe that this document breaches copyright please contact us providing details, and we will remove access to the work immediately and investigate your claim.

DEVELOPMENT OF LARGE SCALE TRIAXIAL APPARATUS FOR
THE DETERMINATION OF THE SHEAR AND DAMPING RESPONSE
OF GRADED GRANULAR MATERIAL SUBJECTED TO CYCLIC LOADS

Submitted by **Stephen Gordon Ring** B.Sc

for the degree of

Doctor of Philosophy

of the University of Bath

1993

COPYRIGHT

Attention is drawn to the fact that copyright of this thesis rests with its author. This copy has been supplied on condition that anyone who consults it is understood to recognise that its copyright rests with its author and no information derived from it may be published without the prior written consent of the author.

This thesis may be made available for consultation within the University library and may be photocopied or lent to other libraries for the purposes of consultation.

S. Ring

UMI Number: U057361

All rights reserved

INFORMATION TO ALL USERS

The quality of this reproduction is dependent upon the quality of the copy submitted.

In the unlikely event that the author did not send a complete manuscript and there are missing pages, these will be noted. Also, if material had to be removed, a note will indicate the deletion.



UMI U057361

Published by ProQuest LLC 2013. Copyright in the Dissertation held by the Author.
Microform Edition © ProQuest LLC.

All rights reserved. This work is protected against
unauthorized copying under Title 17, United States Code.



ProQuest LLC
789 East Eisenhower Parkway
P.O. Box 1346
Ann Arbor, MI 48106-1346

UNIVERSITY OF BATH
LIBRARY

36 17 MAY 1994
Ph.D.
5080101

To MUM

ABSTRACT

Previous relevant research and literature is reviewed, along with various testing techniques used to establish the properties of granular materials. The developmental potential of these techniques is examined for their application to the testing of large granular material under dynamic/cyclic conditions. The requirements of the laboratory equipment is established and the design and development of a new large scale triaxial rig capable of sample diameters up to 305mm is described along with the associated sensors and data monitoring system. The testing program involving investigations into the scaling characteristics of testing granular material at different sample diameters - 38mm, 100mm, and 254mm - under static loading conditions indicates that sample size influences the sample response, with the 38mm samples giving lower 'strength' values than the 100mm and 254mm samples which gave good agreement. The new large scale apparatus is validated for the dynamic testing regime and future work described.

ACKNOWLEDGEMENTS

The author would like to thank his supervisor Dr. S.R. Ledbetter for originally suggesting the area of study and for his support and encouragement throughout the period of the project, and also the technical support of Mr D. Skinner, Mr J. Olver, and Mr A. Clarke whose help resolving practical problems was incalculable.

The author would also like to extend his thanks to Mr M.A. Wilkinson, Dr C.J.K. Williams, and Mr P. McCombie for their many frank and interesting discussions during the period.

Further thanks are also due to two undergraduate students Mr G. Pegram and Mr S. Brumpton who the author supervised during their final year, and who's interest overlapped the area of study.

The authors gratitude is also extended to ARC Quarries Ltd. who supplied the materials used during this project free of charge.

The initial financial support of the SERC, prior to the appointment of the author to the staff of Bath University is acknowledged.

PREFACE

Due to the developmental and exploratory nature of this project the thesis has been organised into three distinct sections which cover the periods of study associated with the project, with appendices covering the raw data and areas of interest not directly relevant to the final result.

Part I covers the background material, and previous related research results. Details of the various testing techniques used to establish material parameters, and their development potential is located in this part, in chapter 4. This is to signify the importance of considering the test method used to establish the sample response from which material parameters are derived. This argument is developed in the introduction. Readers familiar with the subject matter may wish to move directly to part II.

Part II deals with the requirements of the laboratory equipment for the project, and covers the design and manufacture of the new experimental apparatus. All the necessary aspects of the data monitoring system and sensors are also included here.

Part III details the laboratory work and testing program

to date, the results and consequences. It is here that information pertaining to the tests is located. In order not to clutter the document, all test data is located in Appendix 5. Only relevant examples and cummulatative data is included within the main text.

CONTENTS

ABSTRACT

ACKNOWLEDGEMENTS

PREFACE

CONTENTS

1.0	INTRODUCTION	1
------------	---------------------	----------

PART 1

2.0	MATERIAL PARAMETERS OF INTEREST	6
2.1	Analytical Methods	6
2.2	Damping and Shear Modulus	8
2.2.1	Shear modulus	8
2.2.2	Damping	11
3.0	PREVIOUS DATA ON DAMPING AND SHEAR MODULUS	17
3.1	Dynamic Shear Modulus for Sands	18
3.2	Damping Ratio for Sands	30
3.3	Dynamic Shear Modulus for Gravelly Soil	35
3.4	Damping and Gravelly Soils	39
3.5	Summary Regarding Dynamic Shear Modulus	40
3.6	Summary Regarding Damping	43

4.0	DETERMINATION OF PARAMETERS	45
4.1	In-Situ Tests	45
4.2	Laboratory Techniques	46
4.2.1	Resonant Column Method	48
4.2.2	Triaxial Cell Apparatus	50
4.2.3	Simple Shear Apparatus	53
4.3	Summary of Testing Methods	54

PART 2

5.0	EQUIPMENT DESIGN AND MODIFICATION	56
5.1	Design of Large Triaxial Cell	57
5.2	Modification of Static Loading Facilities	65
6.0	OPERATING PROCEDURES	69
6.1	Variables to be Measured	70
6.1.1	Major Principal Stress	70
6.1.2	Minor Principal Stress	71
6.1.3	Axial Strain	71
6.1.4	Radial Strain	74
6.2	Methods of Measurement Adopted	75
6.2.1	38mm and 100mm Triaxial Cells	75
6.2.2	Large Cell	76
6.2.2.1	Radial Strain Rings & Location Studs	78
6.3	Friction Free End Plattens	82
6.4	Sample Preparation	83

6.5	Membranes	86
6.5.1	Membrane Corrections	87
7.0	INSTRUMENTATION	90
7.1	Strain Gauge Displacement Transducers	93
7.2	Linear Variable Displacement Transformers (Transducers)	94
7.3	Strain Rings	95
7.4	Data Aquisition Unit	97
7.5	Digital Control System for Hydraulic Actuators	99
7.6	Load Cells	101

PART 3

8.0	LABORATORY TESTING PROGRAM	102
8.1	Strain Ring Calibration	105
8.2	Material selection	112
8.2.1	Material for Calibration tests	112
8.2.2	Material for Large scale tests	115
8.3	Calibration Tests	120
8.3.1	Relevant Aspects of Test Selection	121
8.3.2	Mode of Testing	124
8.3.3	Method of Testing	127
8.3.3.1	Friction on Ram/bush Interface	130
8.3.4	Summary of 1st Stage Results	132
8.3.5	Discussion of 1st Stage Results	135

8.3.6	2nd Stage Calibration Tests	137
8.3.7	Summary of 2nd Stage Results	138
8.3.8	Discussion of 2nd Stage Results	140
8.4	Testing of Large Grained Material	142
8.4.1	Mode of Testing	143
8.4.2	Results	145
8.4.3	Discussion	147
8.5	Preliminary Cyclic Testing	149
9.0	CONCLUSIONS AND FUTURE WORK	155
9.1	Conclusions	155
9.2	Possible Avenues for Future Work	157

BIBLIOGRAPHY

APPENDIX 1	Specifications of Large Triaxial Cell
APPENDIX 2	Stepper Moter Modification to air/water constant pressure system
APPENDIX 3	Non Contacting Displacement Transducer Details
APPENDIX 4	Calibration Certificates
APPENDIX 5	Test Data

1.0 INTRODUCTION

With the increasing capacity of modern computers, there is a trend towards the use of ever more complex and detailed analytical procedures in the design of soil structures. However, it must be borne in mind that the accuracy of any analysis, independent of any limitations within the analytical process, is fundamentally dependent upon the accuracy of the material properties entered.

Writers and users of such programs are becoming more removed from the processes, constraints, and limiting assumptions used in the determination of soil properties, from both laboratory tests and in-situ techniques. It is therefore necessary when testing soils to establish properties for analysis and design, to evolve more rigorous techniques and to identify influencing parameters. This will improve the understanding of material behaviour, so that any information derived can be used within the appropriate context.

The objective of the research program of which this thesis is a part, is concerned with investigating the effect of dynamic loading upon large particulate materials. It is of particular relevance to the

response of soil structures such as embankments, utilised for dams and transport infrastructure requirements etc., subjected to dynamic loading. The principal source of dynamic loading considered in this work is that due to earthquakes.

Little information has been reported on the dynamic behaviour of large granular material concerning factors such as shear modulus and damping. This seems suprising, since it is the provision of accurate material properties that allows the most economic design in terms of cost and safety. Further, without an accurate understanding of the material behaviour, the designer is relying on essentially arbitrary 'factors of safety', and the failure of such structures as embankment dams can be catastrophic in both human and financial terms.

In the past, investigations into the dynamic behaviour of cohesionless materials have in the main been restricted to sands, because, as the individual particle size increases, so does the size of sample required adequately to model the material. The dimensions of traditional equipment has thus limited testing to fine grained material.

A large scale triaxial apparatus has thus been developed as part of this research program, enabling sample

diameters up to 305mm, compared with the 38mm and 100mm diameters in standard equipment. The larger sample size allows a wider range of particle size distributions to be examined, with individual particles upto 60mm possible. The maximum individual particle size able to be accommodated within any given sample diameter is also affected by the decision to use either single sized or graded samples. As the particle size increases and the more single sized the sample becomes, the greater the difficulty in achieving a representative failure unaffected by the boundary conditions.

Traditionally, due to the size limitations of testing equipment, when determining the parameters of graded materials involving large particulate matter, only the fines content has been tested. The larger cell allows for the testing of graded samples with a similar grading curve to that of the full size material, but with the grading curve 'shifted' down the size axis. The results can then be compared with tests on the fines content alone.

The use of fines tests to determine the overall behaviour, independant of deformation, for a mixed particulate material, is only justified when the larger particles are fully suspended in a matrix of the fines, and this must be maintained throughout the life of the

structure. Deformations of the structure and water seepage may well modify this 'ideal' matrix and so alter the material parameters. Thus data on realistically graded samples will be useful in helping to determine economic design utilising the minimum of material, and possibly reducing the need for over selective grading assemblies at major projects.

To fulfil the requirements of this project, the large triaxial cell needed to be developed, suitable operational methods established, and calibration tests versus the 38mm and 100mm cells performed. Upon the satisfactory outcome of these, was determined the progression onto the static testing of the large grained material followed by the more detailed investigations and dynamic tests. It was felt that the early gradation tests comparing the results on similar sized material tested in different cells required further investigation, as there was some evidence to indicate that the size of apparatus was having an effect. Without fully identifying this aspect the justification of future dynamic tests would be difficult, and certainly lead to problems with the comparable testing between large and small cells. This dramatically affected the timetable, restricting the later testing program. So whilst this thesis may seem to be out of balance with respect to the background information and the documented

testing, it was the objective of the cyclic results that gave rise to the project and the development of the apparatus. The early chapters place the experimental program in context, and provide the springboard for further work.

PART I

**Background material and previous related research
results**

2.0 MATERIAL PARAMETERS OF INTEREST

2.1 Analytical Methods

Current dynamic analyses of soil structures, generally model the soil as a visco-elastic material, using the familiar spring/dashpot/mass system which results in the following general basic equation

$$M.\ddot{u} + C.\dot{u} + K.u = f(t) \quad [2.1]$$

where

$M.\ddot{u}$ represents the mass and acceleration term

$C.\dot{u}$ represents the viscous damping and velocity term

$K.u$ represents the stiffness and displacement term

$f(t)$ represents the loading term as a function of time

In seismic analysis, for a uniform soil, this equation is re-written to take account of the mode of load application via the ground motion as the result of the upward propagation of shear waves, resulting in an equation of the following form

$$\rho(y) \cdot \frac{\partial^2 u}{\partial t^2} + c(y) \cdot \frac{\partial u}{\partial t} - \frac{\partial}{\partial y} \left[G(y) \cdot \frac{\partial u}{\partial y} \right] = -\rho(y) \cdot \frac{\partial^2 u_g}{\partial t^2} \quad [2.2]$$

where

$\rho(y) \cdot \frac{\partial^2 u}{\partial t^2}$ is a density and acceleration term

$c(y) \cdot \frac{\partial u}{\partial t}$ is a viscous damping and velocity term

$\frac{\partial}{\partial y} \left[G(y) \cdot \frac{\partial u}{\partial y} \right]$ is a stiffness and displacement term represented by the shear modulus

$-\rho(y) \cdot \frac{\partial^2 u_g}{\partial t^2}$ is a loading term in response to the ground disp. due to earthquake motion

For multi layered soils, lumped mass parameter systems can be employed using matrix methods applied to eqn. 2.1 resulting in the following expression

$$[M] \cdot (\ddot{u}) + [C] \cdot (\dot{u}) + [K] (u) = \{R(t)\} \quad [2.3]$$

where the matrices [M], [C], and [K] are of the order of the number of soil layers considered.

Detailed solutions to these various expressions are well known and described in numerous text books. The important point to note however, is the nature of the material parameters involved, i.e. the damping and stiffness. In seismic and associated analysis, due to the nature of the loading, (i.e. propagation of shear waves) and the material behaviour, the stiffness term is usually represented by the shear modulus, which relates the shear stress to shear strain. The damping capacity of the material is a measure of its ability to absorb energy.

2.2 Damping and Shear Modulus

As previously determined the material properties of most use in the response analysis of soils and soil structures are the shear and damping characteristics. These material properties are normally defined from the hysteretic stress - strain relationship at different strain amplitudes as shown in figure 2.1.

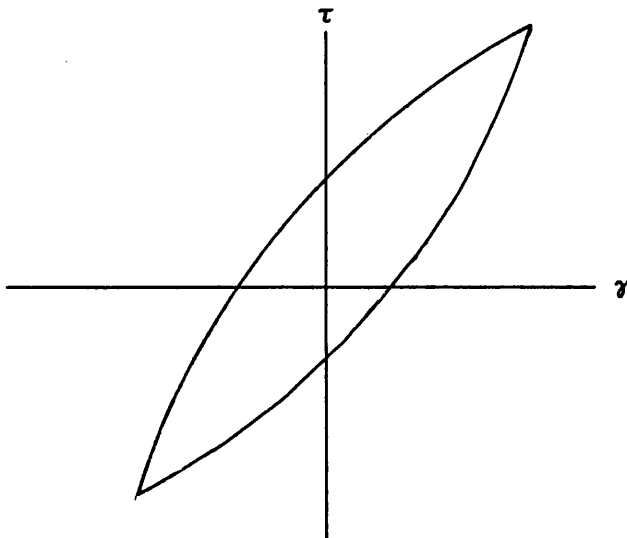


Figure 2.1. Typical stress - strain cycle

2.2.1 Shear Modulus

The shear modulus is the relationship between the shear stress and shear strain and is clearly non linear. However, in order to determine a representative value of shear modulus from each hysteresis loop a line is drawn through the end points, and the gradient of this line is taken as the shear modulus, as shown in figure 2.2, and

represents a secant modulus.

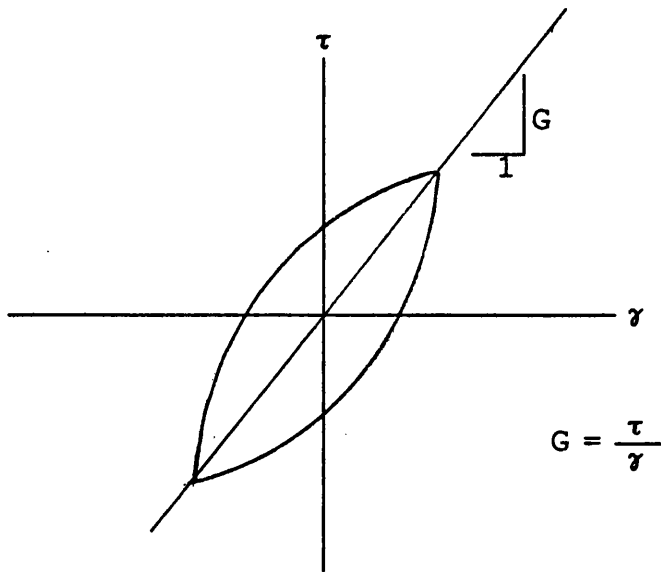


Figure 2.2. Determination of Shear Modulus

If these lines representing the shear modulus and the end point of the hysteresis loop for different strain amplitudes are plotted, then a graph of the form shown in figure 2.3 is produced.

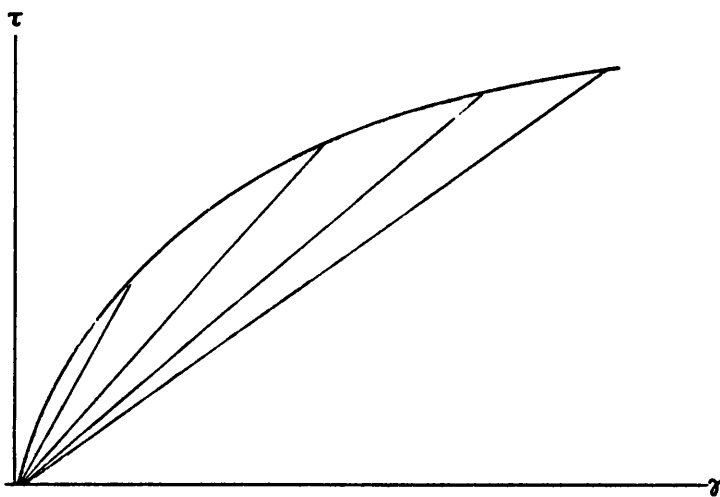


Figure 2.3. Plot of Shear Stress vs Shear Strain amplitude

The representation of data given in figure 2.3 can be used to model the variation of shear modulus with shear strain amplitude, and it was with a hyperbolic relationship that Hardin and Drnevich generated a mathematical model of the soil behaviour, which will be discussed later.

In order to get a more accurate determination of the soil behaviour some analytical procedures attempt to model the stress - strain relationship during each loading cycle more closely. Seed and Idriss (1968) approximated each loading cycle by a bilinear system, shown in figure 2.4, and this coupled with a lumped mass parameter approach has been used in established computer programs (e.g. QUAD 4) in order to determine the response of soil structures to ground motion.

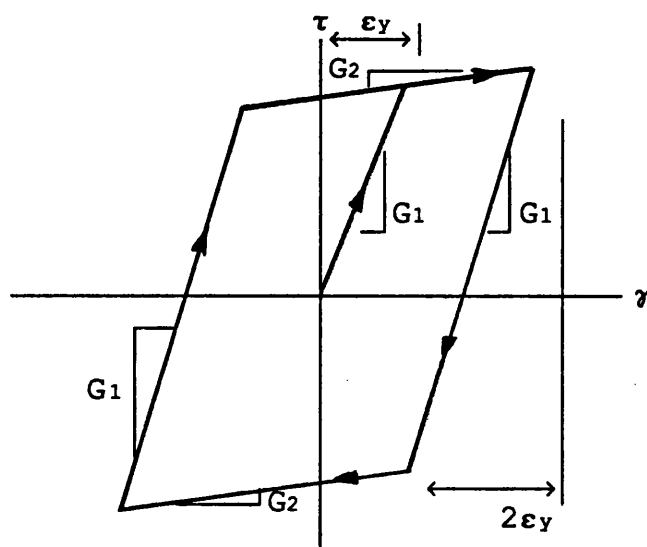


Figure 2.3. Bilinear model from Seed & Idriss (1968)

2.2.2 Damping

The damping, which is a measure of the energy absorbed is less straightforward. It must be noted at this point that we are dealing with the energy dissipated within the soil mass, and not energy lost through any interaction on the foundation boundary. The latter is normally termed radiation damping. Radiation damping is dependent on foundation type and the boundary conditions of the specific design situation, and whilst an important point of consideration often inappropriately dealt with in the design analysis, it is outside the scope of this thesis. With the form of analysis previously described, the damping component was linked to the velocity of the system, which quite clearly presumes a viscous damping coefficient. It is the author's opinion that this is a clear case where the understanding and relative ease of a particular form of analysis has led researchers and engineers into trying to manipulate complex material behavior into a simple all-encompassing coefficient. This in itself is not necessarily a problem, as long as the users are aware of the situation and react accordingly. However, as described in the introduction, there is a tendency for those responsible for design to be ever more removed from the establishment of 'appropriate' material parameters and

associated influencing factors.

Damping in soils is a complex issue. Trying to establish an appropriate measure and quantify the 'damping' for a particular soil type needs careful interpretation. It is certainly possible to establish trends and identify the more apparent contributory factors that influence the damping behaviour of soils, but there is an enormous spread in currently available data that attempts to quantify this factor. This may well be due to the way such data is presented, since the issue is generally simplified to represent the damping capacity in terms of an equivalent viscous damping factor.

The typical stress - strain relationship for a granular soil at a given strain amplitude is hysteretic in nature as shown previously in figure 2.1. The area enclosed within the hysteresis loop is a measure of the energy dissipated. The damping is therefore a function of the stress and strain. This response may be influenced by the rate of strain or stress application, but this is a secondary action. However, as described earlier, the nature of current analytical procedures calls for a viscous damping term which depends solely on velocity.

The idea of equivalent viscous damping for a non linear

hysteretic system was introduced by Jacobson in 1930. In 1960, applying this principle to composite structures, he proposed the following approximate formulae for an equivalent viscous damping factor λ given as a fraction of the critical damping.

$$\lambda \approx \frac{1}{2\pi} \cdot \frac{W_1}{W_2} \quad [2.4]$$

where

W_1 = frictional work area

W_2 = work area under skeleton

This relationship is based on the concept of equating the energy loss of a linear visco - elastic system to that of a non - linear hysteretic system when the maximum applied force on both systems is the same.

It is worth noting that Jacobson qualified the formulae with the following comments:

".....that the concept of an equivalent viscous damping is not easily applied to arbitrary non - linear systems....."

and goes on to say:

"....the problem of finding an equivalent viscous damping coefficient ratio involves additional debatable

assumptions..."

Notwithstanding these reservations the concept of an equivalent viscous damping ratio has become the standard method of quantifying the soil damping for use in response analysis. The standard formula for the determination of this damping factor is as follows, related to figure 2.4, and is essentially Jacobson's 1960 expression.

$$\lambda = \frac{1}{4\pi} \cdot \frac{A_1}{A_2} \quad [2.5]$$

where

λ = Damping factor

A_1 = Area enclosed by hysteresis loop (1-2-3-4-1)

A_2 = Elastic work done. (area 0-3-5-0)

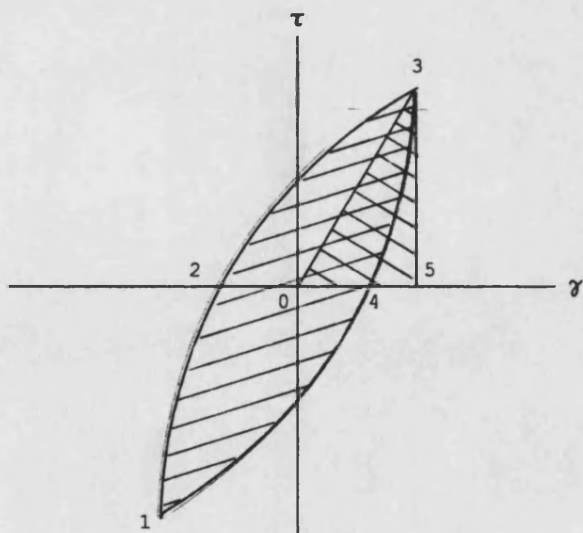


Figure 2.4. Definition of areas for determination of damping ratio.

It could be argued that there is a case for considering

alternative approaches to the problem of assessing the dynamic response of soils; in particular to move away from the standard response analysis based on equation 2.1, and thus remove any ambiguity over concepts of equivalent viscous damping, and for that matter the ever changing shear modulus during a given loading cycle. Any such approach would have to balance the complexity of analysis against any improved modelling of the soil behaviour. The current approach has grown out of a confidence in the understanding and relative ease of the analytical solutions, balanced against problems attached to the justification of material parameters.

Notwithstanding these arguments, this thesis is concerned with the experimental determination of material parameters and expanding the current data base, within the current framework of analysis. As long as the basic source data is available, and influencing factors noted, such data can be utilised and adopted within various analytical models. The use of data from experimental sources must always consider the techniques involved, both in terms of the equipment used and theoretical concepts utilised in the extraction of meaningful results. This thesis will be looking at the scaling characteristics of particulate matter and there is a need to be aware that existing viscous models may no longer apply at the new scales being considered here.

This will be discussed as appropriate when considering the results.

3.0 PREVIOUS DATA ON DAMPING AND SHEAR MODULUS

Investigations into the effects of dynamic loading on cohesionless soils have, in the main, been restricted to sands.

The bulk of data published to date on dynamic loading of large particle granular material has dealt with base materials for highway design. These papers deal with the long term effect upon the deformation characteristics of the material, so data is presented in the form of pseudo elastic moduli analogous to Youngs modulus after a large number of load repetitions. Such loading does not simulate earthquake and other short term or large strain loading conditions very closely, and the moduli determined are not very satisfactory for seismic analysis. Further, the material properties established tend to stabilise to equilibrium values after large numbers of loading cycles, which short term dynamic events do not allow.

The material properties of most concern when dealing with seismic loading are the dynamic shear modulus and equivalent damping ratio, and as already discussed they are defined from the hysteretic stress - strain relationship at different strain amplitudes. Where the dynamic shear modulus (G) is expressed as the secant

modulus determined by the extreme points of the hysteresis loop and the damping ratio (λ) is proportional to the area within the hysteresis loop.

As can be seen from figure 3.1, both these factors are strongly affected by the shear strain amplitude.

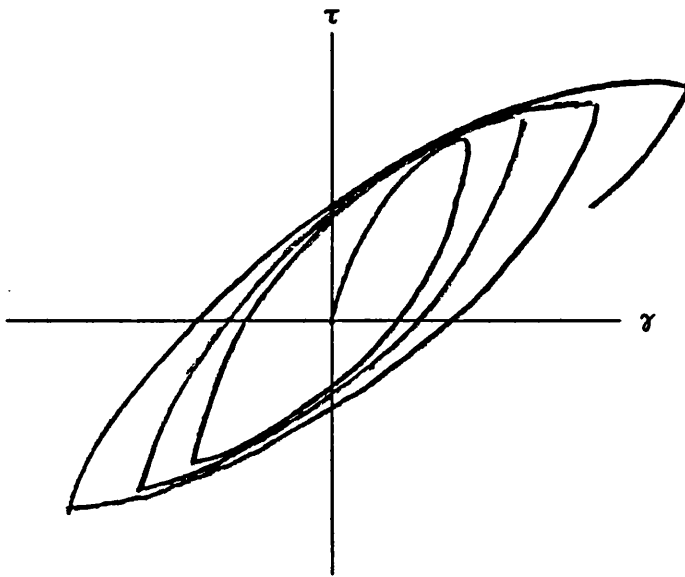


Figure 3.1. Variation of stress-strain relationship with shear strain amplitude.

However many other factors have been shown to influence these parameters to varying degrees.

3.1 DYNAMIC SHEAR MODULUS FOR SANDS

Work carried out by Silver and Seed (1969) showed that the shear modulus was highly dependent on the shear strain and vertical stress. For a given cyclic shear strain, the shear modulus increased with vertical stress, and as shear strain increased, the value of

shear modulus decreased significantly, as is apparent from the earlier explanation of these factors. The relative density was also found to influence the shear modulus, with increasing relative density giving larger values of shear modulus at any given shear strain.

Silver and Seed also noted that the number of stress cycles influenced values of shear modulus, greater number of stress cycles leading to a higher modulus. But this effect was greatest in the first 10 cycles, after which changes were fairly small.

They further noted, relating shear modulus (G) and vertical stress (σ_v) in the form represented by a straight line on a log-log plot, in the form

$$G = k_m \cdot \sigma_v^m \quad [3.1]$$

that values for the exponent (m) varied between 0.5 and 0.7, depending on the shear strain amplitude, m increasing with shear strain.

A comprehensive study by Hardin and Drnevich (1972), investigating the behaviour of soils, suggested that the primary factors affecting the shear modulus characteristics for clean dry sands were as follows

strain amplitude

effective mean principal stress
voids ratio

and less important factors included

effective strength envelope
no. of cycles of loading
octahedral shear stress

Factors such as overconsolidation ratio, frequency of loading, time effects, grain characteristics, and soil structure were found to be relatively unimportant.

Hardin and Drnevich showed that the modulus decreases very rapidly with strain amplitude with no apparent cut off strain level below which the modulus is constant. However, for 'practical purposes' they proposed that the modulus measured for strain amplitudes less than 2.5×10^{-5} could be taken as equal to the maximum value for the shear modulus (G_{\max}).

Increasing the effective mean principal stress was found to lead to an increase in the modulus at a given strain amplitude; further, the effect of changing the number of loading cycles seemed to increase as the effective mean principal stress increased, with increasing number of loading cycles causing an increase in modulus. This

latter effect was more pronounced within the first ten loading cycles than above. There was very little change when the number of loading cycles increased from 10-100, supporting the findings of Silver and Seed (1969).

With the relationship between shear modulus (G) and mean principal stress ($\bar{\sigma}_o$) expressed in the following form,

$$G_{\max} \propto (\bar{\sigma}_o)^n$$

the exponent n was found to vary with shear strain amplitude. At very low shear strain amplitudes when the value of shear modulus approaches G_{\max} , n was 0.5

$$G_{\max} \propto (\bar{\sigma}_o)^{0.5}$$

and at higher strain amplitudes, the value of n approached 1.0,

$$G_{\max} \propto (\bar{\sigma}_o)$$

From their results, Hardin and Drnevich proposed a series of equations which would allow the determination of the shear modulus at any strain amplitude. Simplified for cohesionless materials, these are as follows :

$$G_{\max} = 14760 \frac{(2.973-e)^2 \cdot (\bar{\sigma}_o)^{0.5}}{1+e} \text{ units psf} \quad [3.2]$$

where

G_{\max} = max shear modulus in psf

e = voids ratio

σ_o = mean principal effective stress in psf

the modulus value (G) at any strain level (γ) is then evaluated from

$$G = \frac{G_{\max}}{1+\gamma/\gamma_r} \quad [3.3]$$

where

$$\gamma_r = \frac{\tau_{\max}}{G_{\max}} \quad [3.4]$$

and

$$\tau_{\max} = \left(\left(\frac{1}{2} (1+K_o) \cdot \sigma_v' \cdot \sin \phi \right)^2 - \left(\frac{1}{2} (1-K_o) \cdot \sigma_v' \right)^2 \right)^{0.5} \quad [3.5]$$

where

K_o = coefficient of lateral earth pressure at rest

σ_v' = vertical effective stress

ϕ = effective angle of friction

This method is based on the precept that the shear stress - shear strain relationship can be modelled as a hyperbola as shown in figure 3.1. The apparent complexity of this procedure leads to a feeling of security for the accuracy of the results. But it must be noted that the factors involved in the equations are open to interpretation. K_o is usually a difficult

parameter to establish with certainty, and decisions need to be taken as to whether peak, ultimate or residual values of ϕ are appropriate.

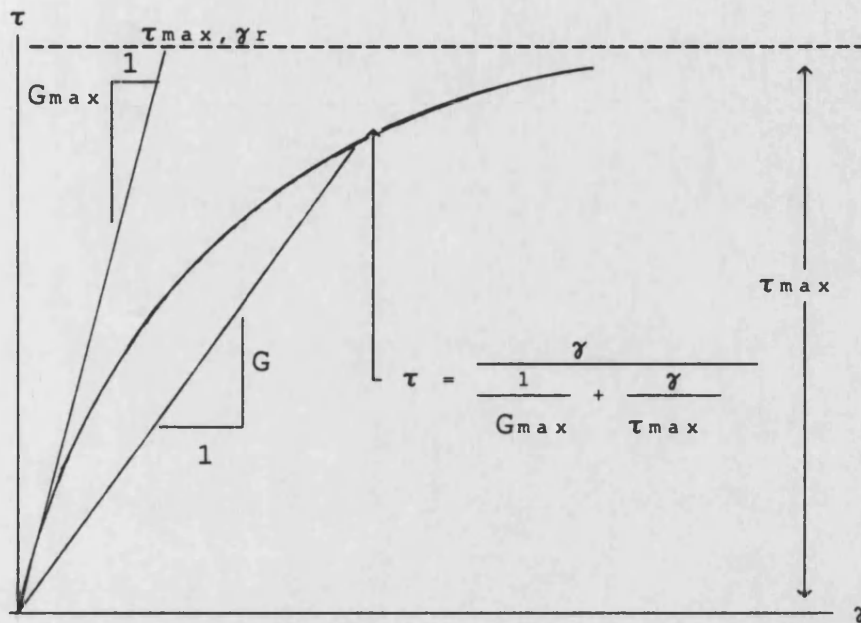


Figure 3.1. Hyperbolic stress-strain relationship

Seed and Idriss (1970) made a detailed parametric evaluation of these equations and proposed a simplified expression for the dynamic shear modulus of sands -

$$G = 1000 \cdot K_2 \cdot (\sigma'_o)^{0.5} \quad \text{units psf} \quad [3.6]$$

where

G = shear modulus
 σ'_o = effective mean stress

and where the influence of various factors are shown through their effect on K_2 .

Using the Hardin and Drnevich equations, Seed and Idriss

found that the effect of the various influencing parameters depended on the strain amplitude and could be summarised by describing their effects on K_2 at low, intermediate, and high strain amplitudes.

1. At very low strains ($\leq 10^{-5}$), K_2 depended only on void ratio.
2. At intermediate strains (10^{-5} to 10^{-3}), K_2 was only slightly influenced by vertical stress, σ , and K_0 , but strongly influenced by void ratio.
3. At very high strains ($> 10^{-3}$), K_2 was slightly influenced by vertical stress, but essentially independent of K_0 , σ , and void ratio.

They thus concluded that for most practical purposes, K_2 could be considered to be determined mainly by void ratio or relative density and the strain amplitude.

Having introduced this factor K_2 , Seed and Idriss noted the results of laboratory tests on samples at different relative densities at very low strains and found values of K_2 in the range of 50 - 75. The general form of K_2 with changing relative density and shear strain is shown in figure 3.2.

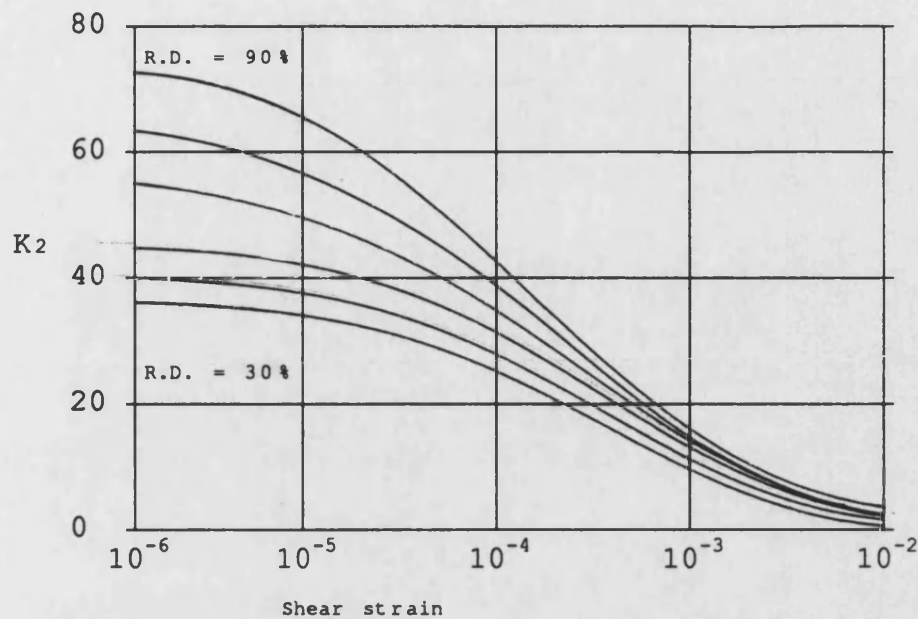


Figure 3.2. Variation of K_2 . After Seed and Idriss (1970)

Comparing these values of K_2 determined from laboratory tests, with values derived from in-situ investigations using shear wave velocity measurements at very low strain levels on dense to extremely dense sands, Seed and Idriss noted favourable agreement, with the in-situ results giving values of K_2 ranging from 44 - 86.

Using the Hardin and Drnevich equations to establish values of K_2 analytically, with $\bar{\sigma}_o = 144 \text{ kN/m}^2$ (3000psf) $K_o = 0.4$, and $\phi = 36^\circ$, Seed and Idriss showed the change of K_2 with shear strain in terms of void ratio as shown in figure 3.3.

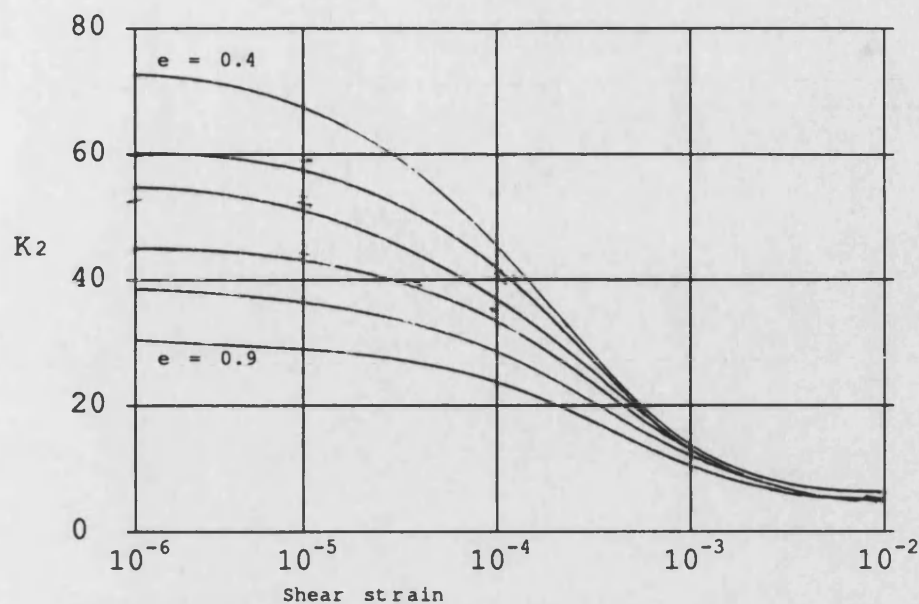


Figure 3.3. Variation of K_2 . After Seed and Idriss (1970)

This showed good agreement with the laboratory results described in figure 3.2. Replotting the data from both experimental and analytical procedures to show the variation of shear modulus with shear strain, and normalising the shear modulus with respect to the maximum shear modulus found at very small shear strains (10^{-6}), Seed and Idriss produced the result shown in figure 3.4.

The data fell within a narrow band and Seed and Idriss thus concluded that a close approximation to the modulus for a sand at any given shear strain could be obtained by determining the shear modulus at very low strains (to establish the maximum value) and then reducing this value for larger strains by using 'reduction factors'

indicated by the mean line shown in figure 3.4.

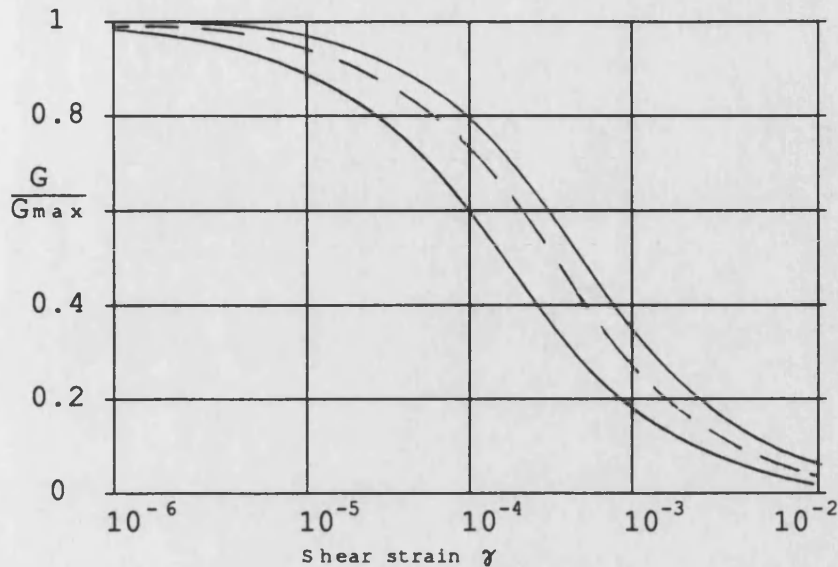


Figure 3.4. Range of data shown in figures 3.2 & 3.3.
After Seed and Idriss (1970)

Reporting on cyclic tests on Toyoura sand, Kokusho (1980) confirmed a similar relationship between shear modulus and shear strain as proposed by Seed and Idriss, showing that the modulus converges to a maximum as the strain falls below 10^{-5} - 10^{-6} and decreases as the strain increases.

For a given shear strain, Kokusho found the value of shear modulus increased with confining stress, as noted by Hardin and Drnevich, and that the number of stress cycles had little effect. Normalising the shear modulus with respect to the shear modulus at 10^{-6} shear strain, again showed a similar pattern to that obtained by Seed and Idriss. But, as confining stress increased, the

curve corresponding to each confining stress shifted consistently to the right, along the shear strain axis, indicating that for a given shear strain as confining stress increases so also does the shear modulus.

Seed et al (1986) reported that the results of Shibata and Soderro (1975) and Iwaski et al (1976) showed that the modulus attenuation curve for sand was influenced by confining stress. This gives some confirmation of Kokusho's results.

Kokusho noted that the effect of void ratio was again similar to that found by Seed and Idriss, i.e. increasing void ratio gave lower shear modulus for a given shear strain. But when the normalised modulus ratio G/G_{\max} was plotted, void ratio had less influence than the confining stress.

In comparison with other data, Kokusho noted that his results followed very closely in form to those reported by Richart, Hall and Woods (1970) who investigated sands with round grains, but differed numerically by approximately 20%, hence whereas Richart, Hall and Woods proposed

$$G_{\max} = 700 \cdot \frac{(2.17 - e)^2}{1 + e} \cdot (\sigma'_c)^{0.5} \quad \text{Kgf/cm}^2 \quad [3.7]$$

Kokusho suggested -

$$G_{\max} = 840 \cdot \frac{(2.17 - e)^2}{1 + e} \cdot (\sigma'_c)^{0.5} \quad \text{Kgf/cm}^2 \quad [3.8]$$

Plotting $G/(2.17 - e)^2/(1 + e)$ vs σ'_c on a log graph revealed straight line relationships with the slope increasing with strain, indicating that the modulus vs confining stress relationship could be formulated as -

$$G \propto (\sigma'_c)^m \quad [3.9]$$

where, m denotes the slope of each line, as noted by previous researchers but expressed in terms of vertical stress, i.e. Silver and Seed (1969).

For the strain range 10^{-6} to $3 \cdot 10^{-3}$ values of m varied from 0.47 to 0.84. This compares with similar findings by Iwaski et al (1976), which gave values of the exponent m in the same strain range of 0.38 to 0.97, and the trend reported by Hardin and Drnevich of values ranging between approximately 0.5 to 1.0; in terms of effective mean stress.

Seed et al (1986) presented a relationship modified from an expression proposed by Ohta and Goto (1976) between maximum shear modulus (G_{\max}) and mean effective stress (σ'_m) and SPT N-values of the form -

$$G_{\max} \approx 1000.20. (N_1) . (\sigma'_m)^{0.5} \quad \text{units psf} \quad [3.10]$$

which is similar to equation 3.6 if

$$K_2 \approx 20. (N_1) \quad [3.11]$$

Using this equation for values of $N_1 = 5$ for loose sands to $N_1 = 44$ for dense sands gave values of K_2 ranging from 34 - 71 which compares favourably with in-situ shear wave velocity measurements giving values of K_2 between 34 - 86 and the laboratory and analytical results previously described.

3.2 DAMPING RATIO FOR SANDS

Silver and Seed (1969) reported that the hysteretic damping increased with increasing shear strain amplitude, but decreased with increasing vertical pressure at any given shear strain. This latter effect, however, was much more pronounced at pressures below $\approx 48 \text{ kN/m}^2$ (1000psf) . The general form of the relationship is shown in figure 3.5.

Further, Silver and Seed reported that the relationship between damping and shear strain was essentially independent of the relative density, and that increasing the number of loading cycles tended to decrease the

damping slightly.

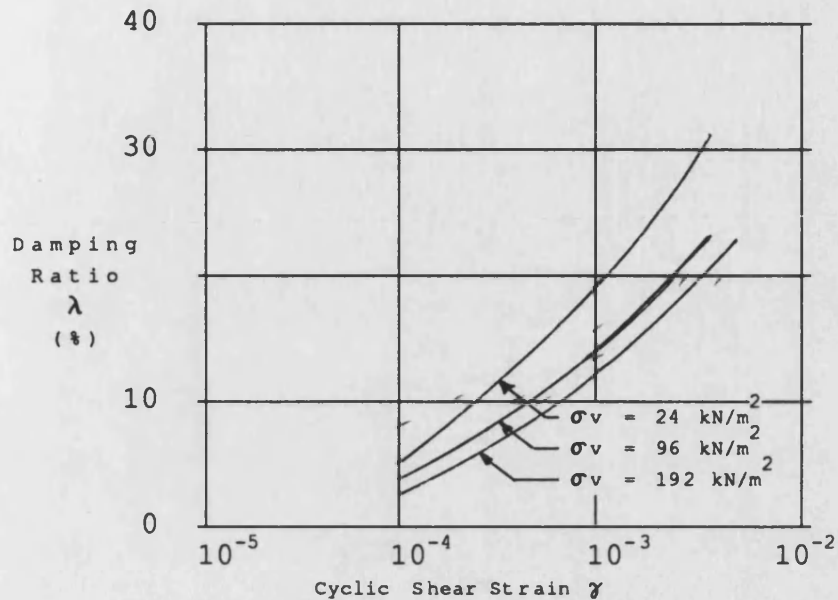


Figure 3.5. Variation of damping ratio vs shear strain after Silver and Seed (1969)

A comprehensive study by Hardin and Drnevich (1972) investigating the behaviour of soils, suggested that the primary factors affecting the damping characteristics for clean, dry sands were :

- strain amplitude
- effective mean principal stress
- void ratio
- no. of loading cycles

and that less important factors included

effective stress envelope
octahedral shear stress

Factors such as overconsolidation ratio, frequency of loading, time effects, grain characteristics, and soil structure were determined to be relatively unimportant.

They found that the damping ratio increased with strain amplitude, with very small values at low strain amplitudes and increasing to an asymptote termed D_{max} at high strain levels.

The effect of increasing mean principal stress at a given strain amplitude, was found to lead to a decrease in the damping factor. Increasing the number of load repetitions was also found to decrease the equivalent damping factor. If one considers the damping of the soil to be a function of its ability to deform, then these effects are to be expected. Both actions lead to a higher density, thus stiffening the material.

From the results of their investigations, Hardin and Drnevich proposed the following equations for the determination of the damping ratio (λ), at a strain level (γ) :

$$\lambda = \frac{\lambda_{max} \cdot \gamma/\gamma_r}{1 + \gamma/\gamma_r} \quad [3.12]$$

where γ_r = reference strain, as determined from equation 3.4,

and λ_{max} is the maximum damping ratio corresponding to very large strains, and for clean sands is :

$$\lambda_{max} = D - 1.5 \log_{10} N \quad \% \quad [3.13]$$

where $D = 33\%$ for clean, dry sands

$D = 28\%$ for clean, wet sands

A similar parametric study by Seed and Idriss (1970) as previously discussed regarding shear modulus, was carried out to determine the effect of variations in the factors mentioned by Hardin and Drnevich as influencing the damping characteristics. This showed that changing ϕ , K_0 , e , and $\%sat$. had relatively minor effects, and considering the equation given for λ_{max} , that within the range of values of N of interest (1 - 30) values of λ_{max} would not be significantly different.

Considering variations in pressure, Seed and Idriss concluded that for pressures below $\approx 24 \text{ kN/m}^2$ (500 psf) which would only apply to the top few metres of deposit, pressure changes may be significant, but that above this figure the effects of changes in pressure were small compared to the effect of shear strain upon damping. They suggested an average damping ratio vs

shear strain relationship for effective vertical stresses between $\approx 96 - 144 \text{ kN/m}^2$ (2000 - 3000 psf). This relationship follows the form shown in figure 3.6 , and compares favourably with data achieved from laboratory testing presented at that time.

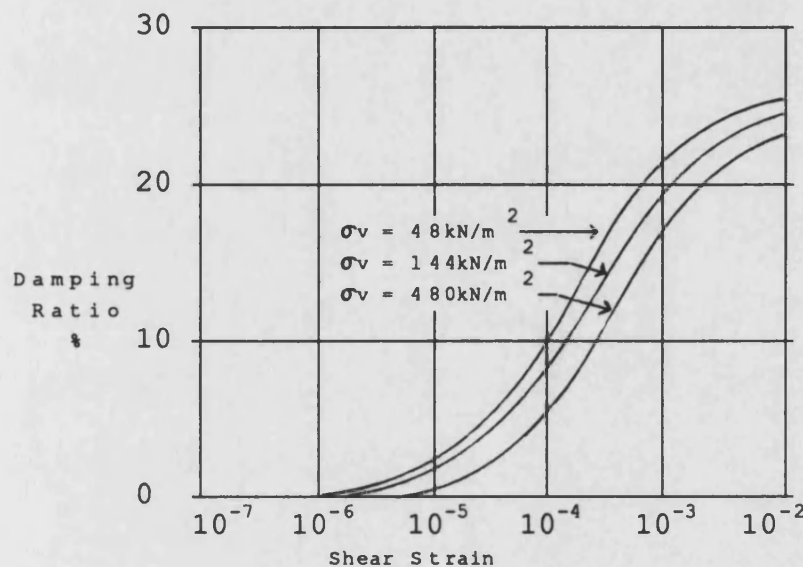


Figure 3.6. Variation of Damping ratio vs Shear strain
After Seed and Idriss (1970)

Kokusho (1980) found the same form of relationship to that proposed by Seed and Idriss and noted that void ratio had little effect, although the curve shifted slightly to the left on the shear strain axis as void ratio increased, indicating an increase in damping at any given strain as density decreased. This is opposite to the behaviour reported by Seed and Idriss (1970). However, as confining pressure increased, the curve was consistently translated to the right along the strain

axis, indicating a decrease in damping as confining pressure increased.

3.3 DYNAMIC SHEAR MODULUS FOR GRAVELLY SOIL

Reporting on a few in situ shear wave velocity measurements on gravelly soils, Seed and Idriss (1970) noted that these indicated modulus values at low strain levels between 1.25 and 2.5 times greater than for sands. They further suggested that the moduli would decrease in a similar manner as for sands, as the shear strain amplitude increased.

As noted earlier, there has been little laboratory work on the dynamic shear modulus vs shear strain relationship for gravels. and as such there is little experimental evidence to report.

However, Seed et al (1980) reported the results of cyclic triaxial tests carried out on large diameter (12") samples of several gravelly materials. These included :

Oroville gravel which was well rounded, while the fine particles were subrounded to subangular and very hard.

Pyramid dam Rockfill whose particles were very angular in shape and could be broken with a hammer.

Venado sandstone which contained fine - medium, very angular particles which were very weak.

Livermore natural gravel, with well rounded to rounded particles which were relatively hard.

Using the relationship proposed by Seed and Idriss (1970) (see equation 3.6), Seed et al presented their data in terms of the factor K_2 .

The results showed that K_2 decreases markedly as the cyclic shear strain increases, and the effect of increasing relative density is to increase the value of K_2 at any given shear strain, although this latter effect was less pronounced at higher strains. The maximum values of K_2 for the materials tested by Seed et al, ranged between 75 - 135 depending on relative density and hardness of particles.

Using equation 3.6 to establish values of K_2 from in-situ shear wave velocity measurements, indicated values for K_2 in the range 90 - 190.

Normalising the modulus values with respect to the maximum shear modulus attained at shear strains of the order of 10^{-6} yielded a similar 'modulus attenuation' with strain amplitude as found with sands, although the relationship is slightly flatter as shown in figure 3.7.

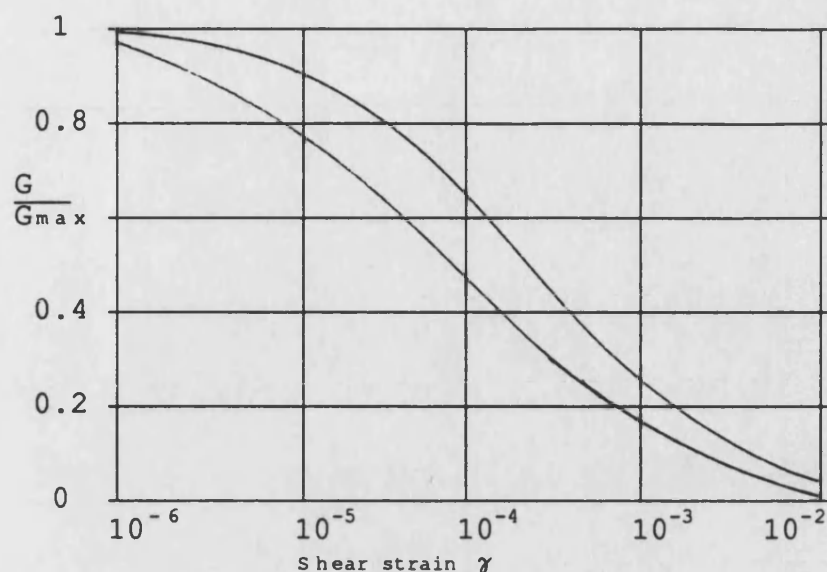


Figure 3.7. Modulus attenuation curve for gravels. After Seed et al (1986)

Seed et al reported that the effect of gradation did not appear to be a significant factor, as there was only a maximum 10% difference between the results from the largest and smallest particle material, with this effect reducing as strains increased. No comment was made regarding any effect the angularity of the particles may have had on the response of the samples.

Kokusho (1981) reported the results of a series of cyclic tests on two kinds of course grained material: one a crushed rock of angular particles, the other a rounded river washed gravel.

The modulus values for the crushed rock were noticeably greater than for the rounded gravel at low strains, although this difference reduced considerably at higher

strains. The decrease in modulus as the strain increased for the crushed rock was much more drastic than that for the rounded gravel.

The maximum modulus values for the two coarse grained materials tested by Kokusho, exceeded the values for dense Toyura sand at equivalent confining stresses by factors between 1.25 to 1.7 for the rounded gravel, and 2.4 to 3.0 for the crushed rockfill. This compares well with the findings of in-situ shear wave velocity measurements.

Since the two coarse grained materials differed little in terms of density and grain size, Kokusho suggested that particle shape and angularity appeared to play a significant part in the determination of shear modulus for coarse grained materials at low strain levels

Although Seed et al made no comment on the possible effects of angularity of the particles in their tests, using the material descriptions given in the papers allows comparisons to be made. The Livermore rounded material showed lower values of shear modulus than the angular Pyramid material, which supports the comments made by Kukosho regarding the effect of angularity. The Oroville gravel was apparently less angular than the Pyramid material, yet it showed higher values.

However these two materials further differed in terms of particle size, the Oroville material having the larger particles and possibly a more even particle distribution, and also in terms of individual particle strength with the Oroville particles being stronger.

3.4 DAMPING AND GRAVELLY SOILS

Again, very little data has been published. Reporting on the tests carried out on the material described in the previous section, Seed et al (1986) found that the equivalent damping ratios were similar for the different materials tested.

This would seem to indicate that material gradation and individual particle strength has little effect upon the damping of a particulate material. The influence of relative density amongst the different materials appeared limited and inconsistent. However when the values for damping ratio for all the materials were averaged, a trend related to relative density did emerge, with increasing relative density leading to larger values for the damping ratio. This correlates with a similar effect noted by Kukusho for sands but opposite to that reported by Seed and Idriss (1970).

Comparing the data from their tests on gravels to the

limits proposed by Seed and Idriss (1970) for sands, Seed et al found good agreement.

Thus it would appear there is little difference in the values of damping between sands and gravels, although there are intriguing disparities on the effect of relative density, with some researchers reporting an increase in damping with increasing density whilst others indicate the opposite. However bearing in mind the spread in the data reported for sands, and the lack of data for gravels, it is difficult to reach any conclusions regarding any comparison. At this point it is worth bearing in mind the interlocking nature of graded samples versus single sized samples, and the effect upon the damping such differences would have.

3.5 SUMMARY REGARDING DYNAMIC SHEAR MODULUS

All reported data confirms the high dependency of dynamic shear modulus upon shear strain amplitude, stress level and void ratio (relative density).

The effect of increasing shear strain is to decrease the shear modulus (G). Maximum values of G are found at very low strains of the order of 10^{-6} , with the modulus decreasing rapidly through strains 10^{-5} to 10^{-3} , and

levelling off at higher strains.

The stress level has been related through either the effective mean principal stress ($\bar{\sigma}_m$), or confining stress (σ_c), or vertical stress (σ_v). Increasing these factors leads to an increase in shear modulus. However, the rate of increase in shear modulus depends on the level of shear strain amplitude, related in the form -

$$G \propto (\bar{\sigma}_m, \text{ or } \sigma_c, \text{ or } \sigma_v)^m \quad [3.14]$$

where the values of the exponent (m) vary through 0.38 - 0.97 with increasing shear strain amplitude. Most researchers relate G_{max} at small strains to a value of $m = 0.5$.

At any given shear strain, increasing the relative density (decreasing void ratio) leads to an increase in shear modulus.

The number of loading cycles has been shown to have a slight influence on the shear modulus. Increasing the number of loading cycles tends to raise the modulus value, although this is most noticeable in the first 10 cycles with little effect thereafter.

When the normalised modulus ratio G/G_{max} vs shear strain relationship is plotted, the data falls within narrow

limits with G/G_{\max} increasing slightly with stress level at any given shear strain.

The general relationship between shear modulus and shear strain for gravels is very similar to sands but the normalised G/G_{\max} vs shear strain relationship appears slightly flatter.

Quantitatively, in-situ shear wave velocity measurements have indicated that shear modulus values for gravelly soils are between 1.25 and 2.5 times greater than for dense sands, and the limited laboratory data available tends to corroborate this.

A number of researchers have presented explicit relationships for G_{\max} at very low strain levels, e.g. Richart, Hall and Woods (1970), Hardin and Drnevich (1972), Seed and Idriss (1970), Kukosho (1980), and Seed et al (1986), this latter relationship being a direct development of that presented by Seed and Idriss (1970). But only Hardin and Drnevich (1972) and Seed and Idriss (1970) (and Seed et al 1986) have indicated methods of establishing values of shear modulus at any strain amplitude. That presented by Seed and Idriss is the most straightforward, but requires knowledge of the factor K_2 , which, as the data base stands at the moment concerning gravelly materials, is very limited. Although

equation 3.11 may enable correlation with standard SPT values.

3.6 SUMMARY REGARDING DAMPING

All reported data on the equivalent damping ratio for granular materials show its dependency on shear strain amplitude and the same general form of the relationship. i.e. very low values of damping at shear strains of the order 10^{-6} , with the damping increasing rapidly through shear strains 10^{-4} to 10^{-3} and appearing to level off at large shear strains around 10^{-2} .

The laboratory determination of damping ratios has been characterised by a wide degree of scatter, even within the data reported by the same researchers using the same apparatus. So it is difficult to reach any definitive conclusions about the influence of various factors. It is worth noting at this point that the determination of damping values is highly dependent on the testing method employed. Some techniques rely on the manipulation of mathematical models, whilst others rely directly on the stress - strain measurements themselves. This is covered in more detail in section 4.0.

Most reports have indicated that increasing the stress level decreases the damping at a given strain amplitude.

This effect appears most significant at lower values of stress, and bearing in mind the scatter in the data, Seed and Idriss (1970) concluded that as far as sands are concerned, the average damping relationship at $\sigma_v \approx 90 - 140 \text{ kN/m}^2$ (2000 - 3000 psf) would be adequate for most purposes.

Regarding gravels, the data so far published falls within the limits previously determined for sands, with any effects of gradation and individual particle strength being obscured by the spread of data. The effect of relative density appears to be inconsistent.

Thus, there appears little difference in the damping ratios between sands and gravels, although due to the scatter of results this confirms little other than that the damping ratio in cohesionless soils increases up to approx 30 - 40 % with increasing strain.

For particulate cohesionless materials, the published data shows that within the range of strains applicable to seismic conditions ($10^{-4} - 10^{-2}$) values for the damping ratio differ by as much as 500% at any given shear strain, ranging from 3% at the lower strain range to as high as 28% at higher strains. This makes it impossible to give precise quantitative values for design purposes.

4.0 DETERMINATION OF PARAMETERS

When dealing with material parameters it is important to be aware of the procedures and processes employed in their determination. There are various ways and means of expressing, and establishing the response of materials, and these often have a bearing on the interpretation and manipulation of the data into material properties. As this thesis required the development of a new piece of testing apparatus it is necessary to consider appropriate testing methods and their influence on the data produced.

This chapter outlines the various laboratory methods available, and those employed to produce the results detailed in the previous chapters. These techniques are discussed in terms of their development potential for testing large grained particulate material. In-situ tests are briefly covered, as results from these methods have been used in the in the past as verification of laboratory tests.

4.1 IN-SITU TESTS

The in-situ determination of the dynamic properties of soil rely on the inducement of ground motion and measurement of the soil response. The material

parameters can then be determined from equations that relate the speed of the induced vibrations to the material property required via mathematical models.

The dynamic shear modulus can be determined from the measurement of the shear wave velocity using the following equation -

$$v_s = \sqrt{G/\rho} \quad [4.1]$$

where

v_s = shear wave velocity determined from field tests

G = Dynamic Shear modulus

ρ = material density

However, this technique of measuring the shear wave velocity from induced ground vibration gives a value for the shear modulus (G) corresponding to G_{max} . This is due to the fact that the vibrations induced within the soil are at very low strain levels. This method is therefore not suitable for determining the relationship of shear modulus with increasing shear strain. Results from these tests have been used to confirm laboratory and analytical values for G_{max} determined at low strain levels.

4.2 LABORATORY TECHNIQUES

There are many methods of determining the dynamic shear modulus and damping of soil samples in the laboratory.

Triaxial compression, and simple shear tests conducted under cyclic loading conditions provide hysteretic stress - strain relationships for samples, and thus allow direct determination of the modulus and damping.

Forced vibration tests, such as the application of longitudinal or torsional vibrations to cylindrical samples or shear vibrations to layers of soil on shaking tables, allow the investigation of the response of samples at various frequencies. Material properties such as the dynamic shear modulus can be determined from various mathematical models that relate these factors to the natural frequency.

Free vibration tests where measurements are taken of the decay in response of a soil sample to certain excitation loadings allow the determination of a damping value. Use of mathematical models that relate the logarithmic decrement factor to the damping ratio give a damping value that may be compared with that determined from constant amplitude vibration tests. But, once again, account needs to be taken of the validity of the mathematical representations used.

4.2.1 Resonant Column Method

This is a forced vibration test allowing either longitudinal or torsional excitation. The longitudinal vibration tests allow the determination of Young's modulus, whilst the torsional vibration tests are used to determine the shear modulus.

A soil sample, usually a cylindrical column, is vibrated in torsion to determine its resonant frequency, from this the appropriate wave velocity can be established and hence the shear modulus.

A variety of resonant column devices have been used by researchers in the past. The major differences between these devices that relate to the interpretation and determination of the modulus from the results are the constraints applied to either end of the soil sample. These factors are important since the equations used to establish the modulus are derived from response calculations in which the end conditions need to be modelled appropriately. End conditions used previously are free - free (in other words the specimen is modelled as unrestrained at either end), and fixed - free (in which the specimen is modelled as being fully restrained at one end and unrestrained at the other). An example of the type of expression derived for the

fixed - free end condition is given below

$$G = 39.48 (f_n^2 L^2 / \alpha^2) \rho \quad [4.2]$$

$$\text{and} \quad \alpha = \omega_n L / v_s \quad [4.3]$$

where

G = Dynamic shear modulus

f_n = Undamped natural frequency

L = Height of sample

ρ = Density of material

ω_n = Undamped natural circular frequency

v_s = Shear wave velocity

A feature of torsional excitation within these devices is that the shear strain within the sample varies from zero at the central axis of the sample to a maximum value at the perimeter of the sample. Thus when testing solid samples only very low strains can be applied and the values of shear modulus determined relate to G_{\max} . To overcome this limitation hollow samples can be used to assess the response at high strains. However, the thickness of the sample needs to be kept as small as possible, since there will still be a variation of shear strain between the inner and outer diameters of the sample. Hollow samples incur additional complexity in the loading and pressurising system, related to the internal and external pressures of the sample, and set up a complex regime of surface stresses on the sample.

Due to the complexity of the end platens of these devices the samples often have to be prepared within a separate triaxial cell.

A value for the damping capacity of soil samples can also be determined from the resonant column test by making use of the logarithmic decrement factor. Once a sample has been set into steady state vibration the driving force is removed and the amplitude of vibration allowed to decay. The decay vs corresponding number of cycles can be plotted, and the logarithmic decrement factor evaluated from the gradient of the resulting straight line. Note that a correcting factor must be applied to take into account the effect of any mass that the apparatus may place on the sample. As explained in section 4.2, further numerical models are required if the decay of free vibrations is to be related to damping at constant amplitude.

4.2.2 Triaxial Cell Apparatus

This is an established piece of apparatus much used within geotechnical laboratories. In its simplest form it is essentially a pressure cylinder within which a sample of soil can be sustained at an all round confining stress and an additional vertical force is applied to the sample within the cell. The cylinder (or

cell) can become more complicated with the addition of special drainage plattens, drainage lines, and fitments for the measurement of the sample response (i.e. load and displacement) etc within and external to the cell.

The traditional apparatus allows for samples of 38mm diameter, which limits possible sample materials to fine grained sands, silts, and clays. Cells capable of taking samples up to 100mm in diameter are available but less common, which allow the testing of coarser sands and possibly very fine gravels.

The loading method employs an all round confining stress with provision made for the application of an additional vertical force to be applied to the sample, usually via a loading ram passing through the cell to an external loading device.

Using these devices, a sample's response to dynamic loading can be recorded and stress - strain relationships plotted from which the damping and shear modulus can be determined directly. There are no limitations on the strain amplitudes that can be applied to the sample inherent to the technique itself, although researchers have reported some difficulty at very low strain levels. This seems to be related to the difficulty in actually measuring the very small

responses from the sample at such low levels of strain. As use of this apparatus requires the suitable determination of the stress-strain of the sample, the technique relies on actual measurement of the sample's axial and radial deformation, and of the load applied to the sample. Traditionally, such measurements are made external to the cell, relying on the transfer of the sample's response through the mechanism of the apparatus. At very low strain levels this is inappropriate and methods that allow measurement on the sample within the cell are required.

There is no inherent limitation in the technique in terms of the size of sample that can be tested, other than the need to build a cell capable of holding it, thus large grained particulate material may be tested using this method. In fact most dynamic testing of large grained material uses this method, although some Japanese research has used shaking tables.

When using the triaxial cell apparatus it must be noted that the stresses set up within the soil sample do not model seismic loading conditions very well. This can be overcome by careful assessment of the loading conditions and adapting the results accordingly.

The triaxial cell apparatus is used for a wide variety

of testing requirements under static loading conditions, and is probably one of the most common items found in geotechnical testing and research labs in its 38mm and 100mm diameter variants.

4.2.3 Simple Shear Apparatus

This is a complicated piece of apparatus that simulates the conditions of simple shear, and was first developed by Roscoe (1953) at Cambridge University.

This type of test models the stresses induced by seismic loading fairly closely, and has been used to study the effect of liquefaction on fine grained material.

With this apparatus, as with the triaxial cell, data results from the direct measurement of the deformation and loading of the sample and so it is possible to plot the stress - strain relationship directly in order to determine the damping and shear modulus.

There is no limitation inherent in the method on the size of sample that can be tested, other than the need to construct a sufficiently large apparatus, and provide the associated large loads and energy input that such a device would require.

4.3 SUMMARY OF TESTING METHODS

In the past, the resonant column device using cylindrical samples subjected to torsional vibration, has been the most commonly used apparatus to establish the dynamic properties of clays and sands. However as already discussed, due to the variation of shear strain across a cylindrical specimen when torsionally excited, hollow specimens have generally been used. This allows the accurate determination of parameters for fine-grained material and clays, but not large grained material. This technique is not suitable for the testing of large grained particulate material since the thickness of the ring, of the hollow sample, needed adequately to model the material would re-introduce the problems of variable shear strain that the technique was established to avoid and lead to a very large diameter.

The triaxial apparatus was selected as the most suitable compromise between the requirements for testing large samples and for providing realistic loading conditions, considering both the cost and time constraints of developing a new piece of testing equipment. Further, the development of a large scale triaxial cell capable of testing large particulate material would enable direct comparison with similar

tests carried out on the 38mm and 100mm diameter cells already in use, in order to assess the effect of scaling and gradation on the response of the specimens. This could then lead to the development of tests that can be run on modified 38mm and 100mm samples allowing the determination of the properties of graded samples as previously outlined, whereby the material as a whole can be taken into account and not just the fines content.

The use of the triaxial apparatus for the determination of dynamic characteristics is hindered by the difficulties of making reliable measurements at very low strains of the order of 10^{-6} . If this can be overcome, then valid results can be achieved which compare very favourably with other devices, as demonstrated by Kokusho (1980) on small diameter samples. Both Seed et al (1986) and Kokusho (1981) used large triaxial cells to provide the only as yet published data on the dynamic shear modulus and damping for large particulate material.

Triaxial cells (up to ≈ 150 mm) have also been used with success by Brown and Snaith (1974), Hicks (1970), Boyce (1976) and others in the determination of the response of road base material subjected to long term cyclic loading conditions.

PART II

**Development of experimental apparatus and requirements
of data monitoring system**

5.0 EQUIPMENT DESIGN AND MODIFICATION

At the start of this project the facilities available were limited to the normal teaching requirements of the department, with the addition of some 100mm stress path cells. Specialist equipment was thus required to enable this program to be undertaken.

The standard equipment consisted of a number of static 38mm and 100mm triaxial cells. These were loaded either using geared electric motors, giving strain controlled tests, or from a constant pressure system, giving stress controlled tests.

In terms of the requirements of the testing program there was a need to develop a large triaxial cell capable of containing samples of large grained particulate material, and to enable the dynamic loading of both the large cell and the existing 38mm and 100mm cells.

New laboratory facilities were expected to be provided within an appropriate timescale and the large cell was designed to make use of these new facilities, including digitally controlled fatigue rated loading jacks. The design of the large scale cell is detailed in section 5.1.

In order to proceed with the testing of smaller grained material, work was undertaken to modify the existing static loading facilities to enable dynamic loading of the 38mm and 100mm cells. This work is detailed in section 5.2.

5.1 DESIGN OF LARGE TRIAXIAL CELL

The design, manufacture, calibration and operation of a large triaxial cell was integral to the objectives of this program. The design was however constrained within tight financial and manufacturing limits.

Loading would be provided by external hydraulic actuators under either manual or digital control. This would allow for a wide range of loading ranges, and ensure the versatility of the system. The triaxial cell itself could therefore be kept relatively simple, and the manufacturing costs and time kept to a minimum.

As part of the program of testing was to determine the effect of scaling on the sample response by comparing the results of tests carried out on the existing 38mm and 100mm diameter cells with tests conducted on the large cell, it seemed sensible to maintain the same ratio of sample sizes. This determined the expected sample diameter to be tested within the cell to 264mm

($\approx 10.5''$), which tallied favourably with the particle size of the materials expected to be tested. Material available for testing had an upper size range between 20mm - 30mm, which would require minimum sample diameters of the order of 254mm - 305mm, obviously the larger the better, in order to allow reasonable stress distribution within the sample.

There was a physical limitation on the manufacturing side due to the manufacturer's inability to turn metal above a particular size. Allied to this were limitations on the length of elements that could be turned within the diameter limitations. This latter constraint could be overcome to some extent by the manufacture of special tooling, that could hold elements within the over-all diameter limitations of the lathe.

The nature of the loading jacks, the need to keep the design as simple as possible, and the requirement to ensure the versatility of the apparatus, meant that the loading ram had to be placed through the top of the cell. This necessitated careful design of the bush arrangement through which the loading ram would pass, and tight tolerances on the shaft - bore arrangement, in order to ensure there would be no pressure loss from within the cell. The friction on the loading ram / top cap interface had to be kept to a minimum to ensure the

smooth operation of the cell. Further, in the case of dynamic testing the loading ram must operate equally well in both directions. This latter requirement has a bearing on the type of seals that can be used within the bush arrangement to ensure no pressure loss.

The final arrangement utilised a sealed bush in the top cap of the triaxial cell which incorporated PTFE bearing surfaces to reduce friction to a minimum.

Due to the tight tolerances on the bush arrangement, deformations of the top cap of the cell had to be kept very low, otherwise the running fit of the loading ram would have been impaired. The stiffness of the top cap was simply related to its thickness, which at overall diameters approaching 0.5m starts to have serious weight considerations.

The maximum size of sample that could be accommodated within the cell was restricted to 305mm (12") diameter. The need to ensure sufficient space around the sample for instrumentation, and to allow deformation of the sample, coupled with the limitation of a maximum overall diameter for the cell of 457mm (18") (due to manufacturing constraints), meant there was no room within the cell for supports for the top cap. This meant that the pressure wall of the cell would have to support the top cap, resist the internal pressure, and carry the

prestress applied between the top cap and base plate to seal the connections with the pressure wall.

Traditional triaxial cells use pressure walls formed out of tubes of perspex, which can be reinforced if required. These have the advantage of being translucent and thus allow visual inspection of the sample throughout the test. The internal diameter required for this large cell, coupled with the internal pressure and vertical loading requirements, ruled out the use of perspex since the required thickness would be too great. Stainless steel was therefore selected for the pressure cylinder, and in order to keep fabrication costs down, standard tubing of an appropriate diameter was found to be adequate. The ends of the tube were machined and a groove cut to allow for an 'O' ring seal between the cylinder and both the top cap and base plate. Appropriate grooves were machined into the top cap and base plate to accept the pressure cylinder.

Due to friction between the sample and the top and bottom plattens standard triaxial cells normally make use of samples with a height to diameter ratio of 2:1, in order to achieve a realistic failure mode within the sample. With sample diameters upto 305mm, this would require sample heights of 610mm. The higher the sample, the taller the pressure cylinder would need to be and

bearing in mind the limited thickness of standard tubing, the more likely that buckling failure might occur. At these sample heights the overall length of the tubing exceeded the tooling capacity of the manufacturer. Also, the taller the sample the greater the difficulty in preparing the sample, since the preparation of granular samples requires the use of formers and vacuums in order to ensure their stability prior to the application of the confining stress (i.e. the cell pressure). This situation is further complicated by the loading arrangement of the hydraulic jacks which require the cell and sample to be assembled and then moved under the loading frame manually. The taller the sample the more susceptible it would be to disturbance by such movement prior to the testing. For these reasons it was decided to limit the height of the cell and use samples with height to diameter ratios near to 1:1. This necessitates the use of friction free end plattens in order to limit the effect of end constraints on the sample deformation. These friction free end plattens will be discussed in greater detail later.

Due to the restricted size of the apparatus, the whole assembly needed to be dismantled in order to prepare the sample within the cylinder. For this reason and to ease manufacture, all connections utilised screw threads (mostly stainless steel cap screws) with high

strength steel tie studs being used between the top cap and base plate to prestress the whole apparatus and thus seal all joints between the external elements. Two cylindrical steel rings distribute the load from the eight tie rods evenly around the top and bottom plates. Drainage and pressure lines with appropriate fittings for the laboratory were provided within the base plate with appropriate detailing to the top and bottom platens, allowing the full range of drained and undrained tests to be performed on dry, partially saturated, and saturated materials.

Sealed access points, similar to those employed on the Imperial College stress path cells were incorporated in the base plate and top cap, which allowed wiring etc from any sensors placed within the cell to be connected to recording apparatus external to the cell.

An essential safety device provided on the top cap was a pressure release valve set to relieve any excess pressure induced within the cell as a result of say, a sudden collapse of the specimen followed by the rapid insertion of the loading ram. The setting of the valve needs to be well below the design limit for the cell.

As the pressurising medium was water all internal elements were made from stainless steel, except the top

sample platten, which for lightness, was made from aluminium. This helped on a number of counts; ease of assembly, to minimise disturbance to and dead weight upon the sample upon formation. To reduce costs the external elements utilised mild steel in various grades.

Due to the large size of the cell, and weight of the individual elements, the dismantling and assembly of the cell at the beginning and end of each test required the use of lifting devices, with precise handling control, which is necessary when positioning the top cap. The whole apparatus was positioned on a greased track placed beneath the loading frame which allowed easy smooth movement of the cell in order to allow sample preparation, assembly and dismantling adjacent to the loading frame.

A diagrammatic view of the cell is shown in figure 5.1, and views of the actual apparatus are shown in plates 5.1 to 5.5. Detailed specifications and design methods are given in appendix 1.

The cell was satisfactorily pressure tested to 4137 kN/m^2 (600 psi) , and for the tests carried out for this thesis the pressure release valve was set at 1379 kN/m^2 (200 psi) .

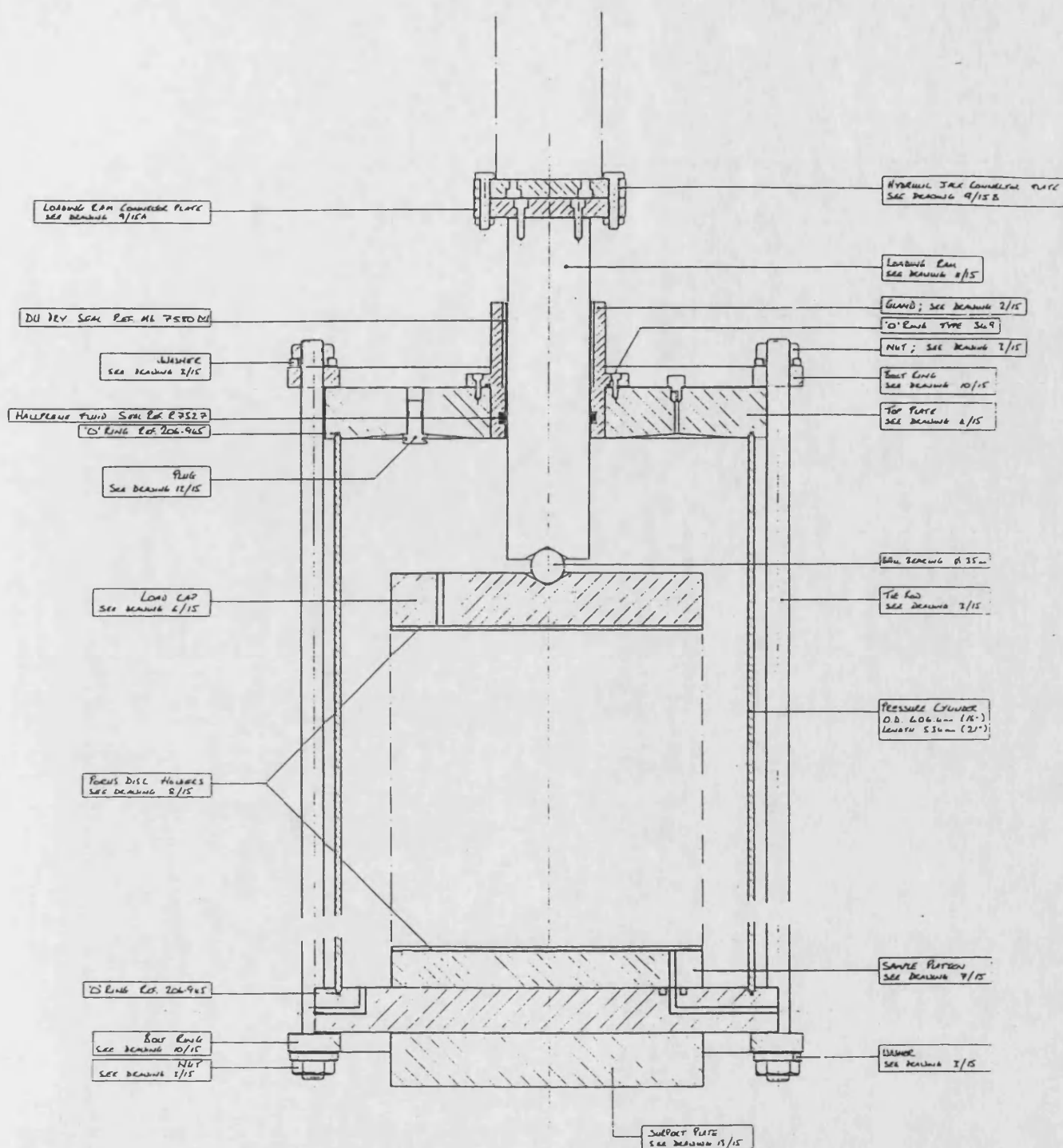


Figure 5.1. Detail of large triaxial cell.

5.2 MODIFICATION OF STATIC LOADING FACILITIES

The original loading equipment was only capable of applying static loads and consisted of geared electric motors and a constant pressure system. The constant pressure system operated off an air line via a pressure regulator, which, via an enclosed air bladder/water interface provided either a constant air pressure or constant water pressure. The regulator was manually operated and it was hoped that by connecting a stepper motor controlled by a computer to the regulator the system could be used to provide variable loads. The control and feedback system had to be designed and built from scratch. The standard and modified systems are shown in figures 5.2 and 5.3 respectively. It was found that due to the air/water interface and the flexible nylon pipework the system was not 'stiff' enough, in that pulsing was set up within the pipework. This resulted in a loss of sensitivity and control since the system became unstable and could not respond in an appropriate manner for the feedback loop to function correctly. See appendix 2 for details of this system.

Full hydraulic systems are more appropriate to the application of this technique. The new laboratory facilities would include suitable loading frames for the dynamic tests, and by this stage of the project these

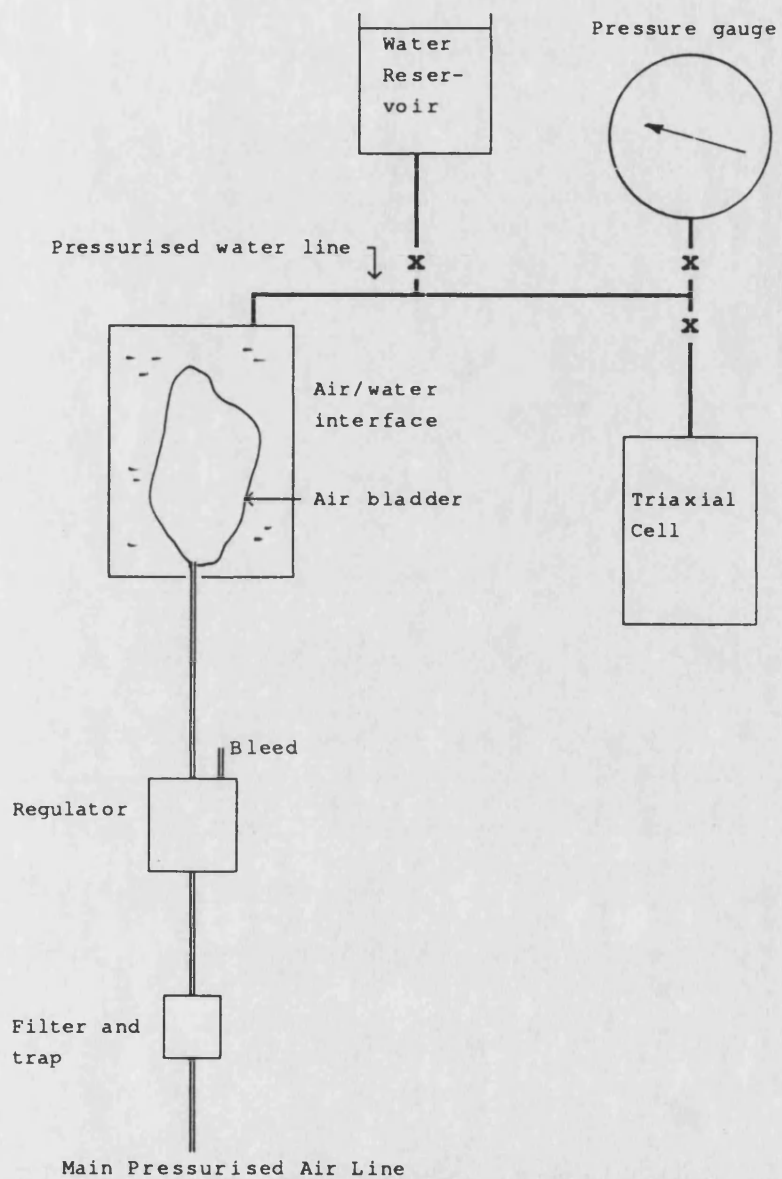


Figure 5.2. Standard pressurising system

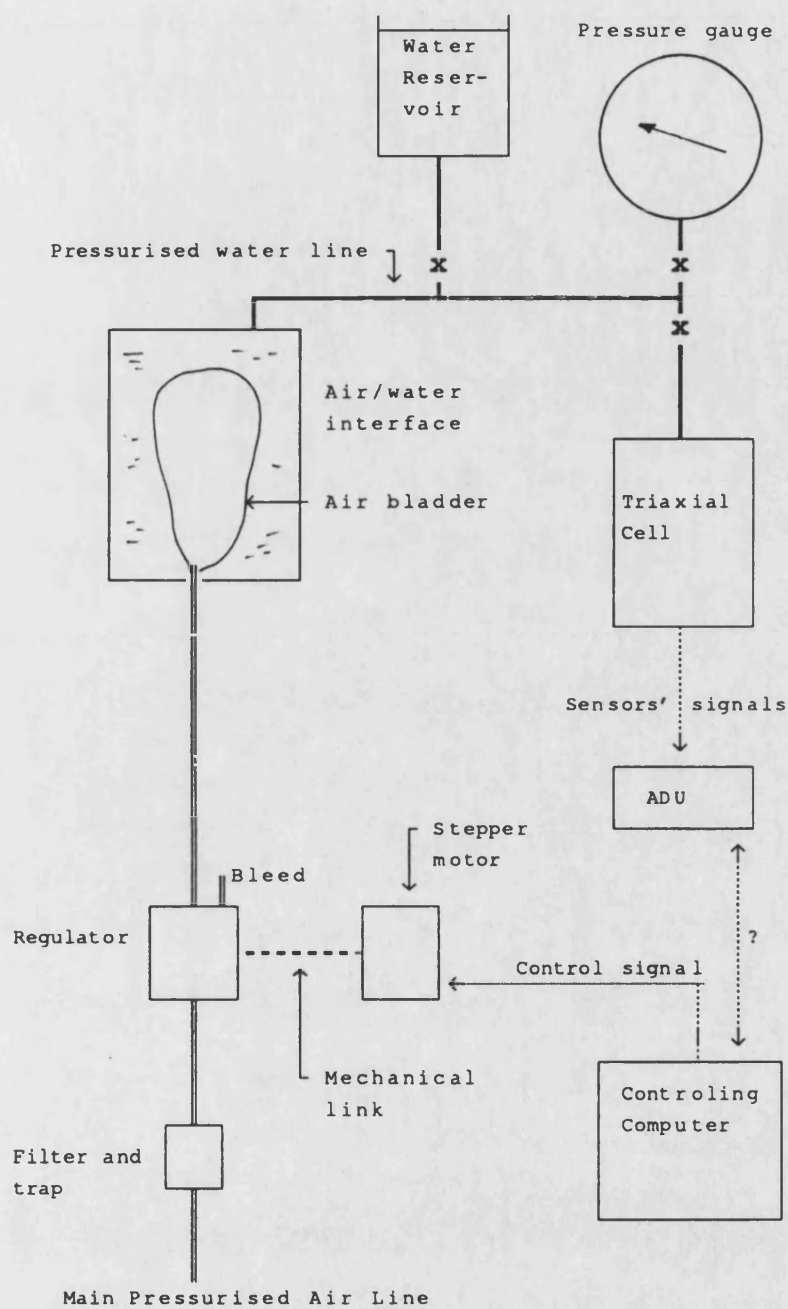


Figure 5.3. Modified dynamic pressurising system

laboratories were expected to be operational within a sufficiently short period of time to make the expenditure necessary to adapt the air/water system to a hydraulic system uneconomic.

The dynamic loading arrangement for the 38mm and 100mm triaxial cells was thus to utilise the DARTEC 100kN loading frame using a digital control system.

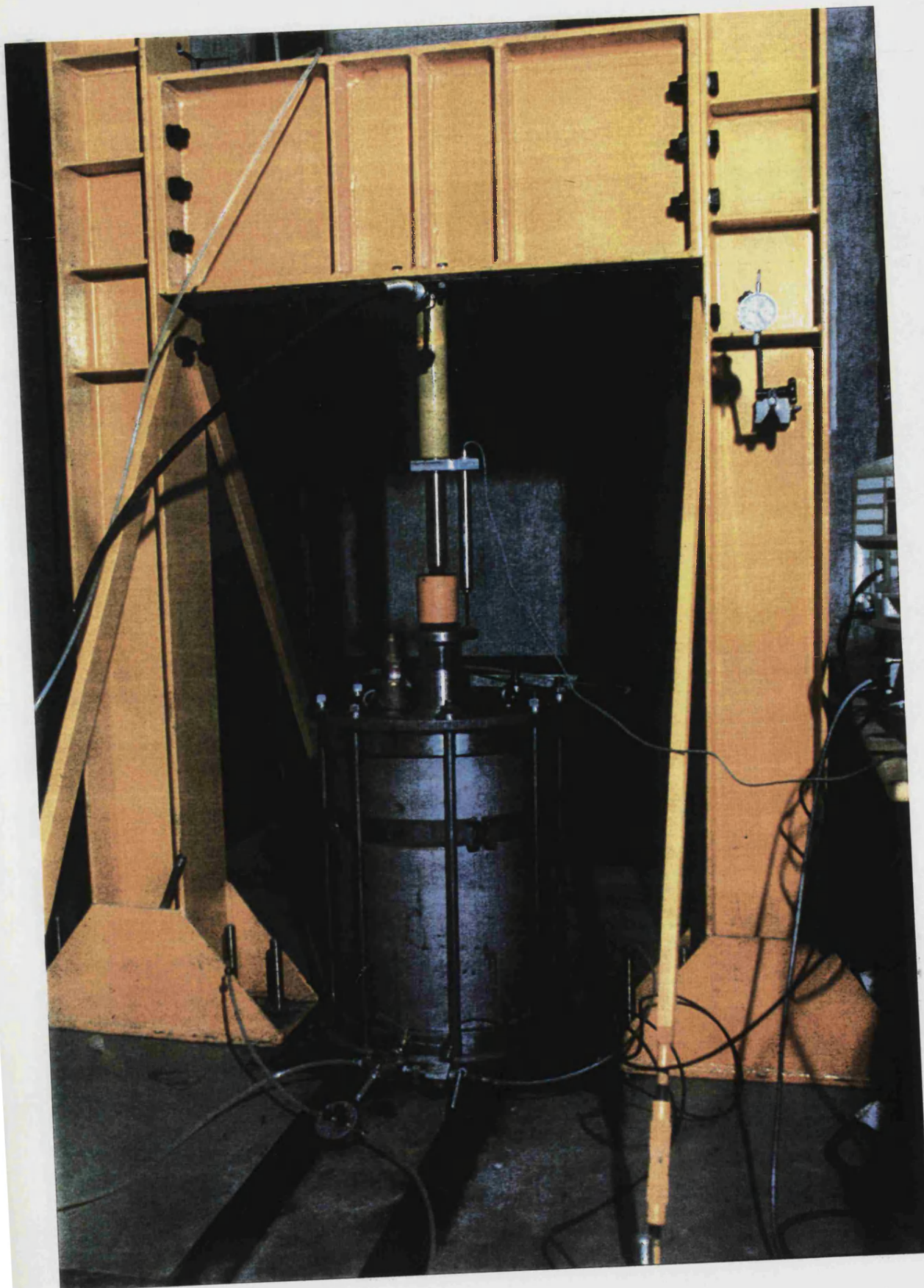


Plate 5.1. Cell assembled in the static loading configuration.



Plate 5.2. Free standing sample sealed around top and bottom plattens. Top drainage fitting also shown.

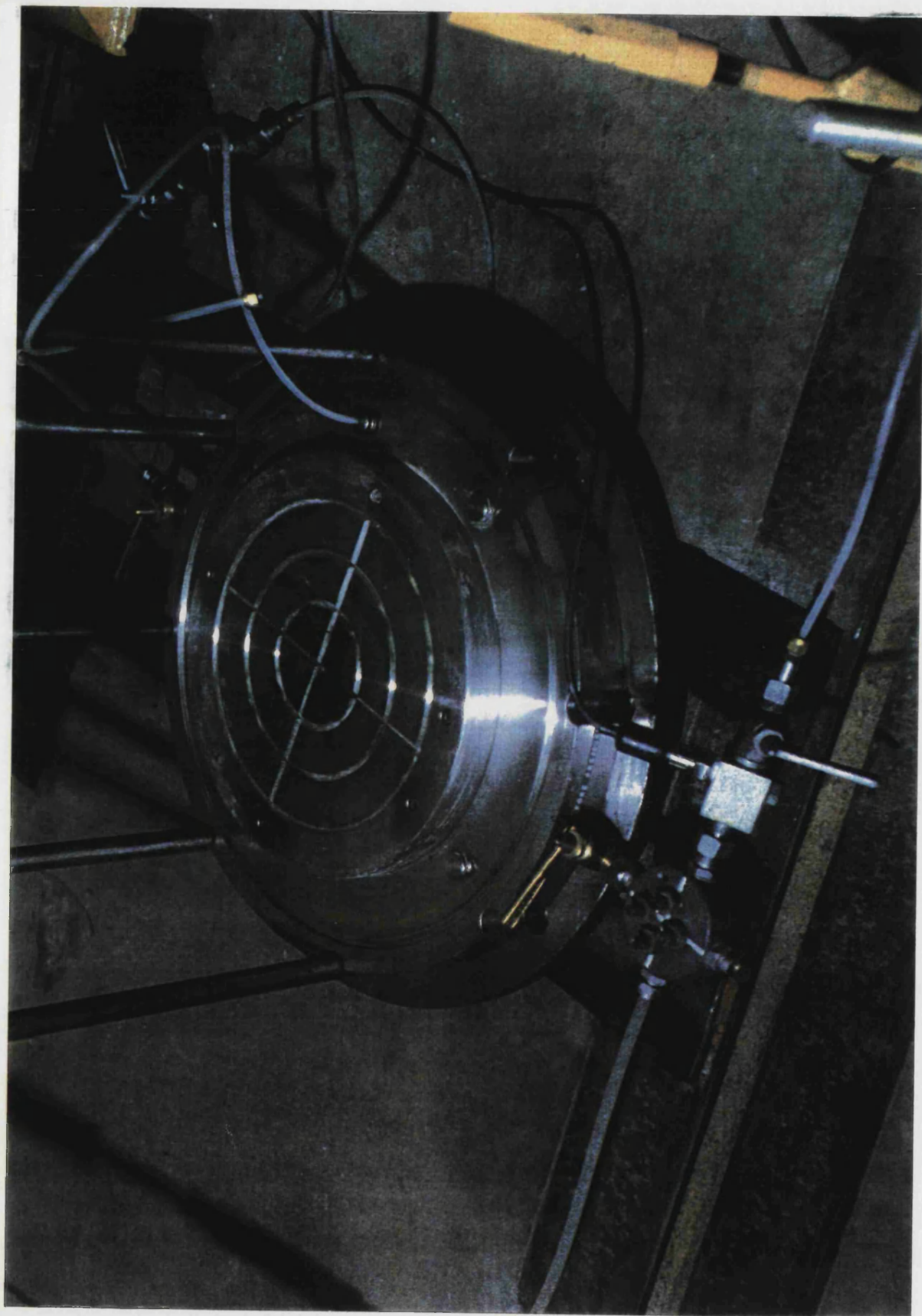


Plate 5.3. Base of cell.

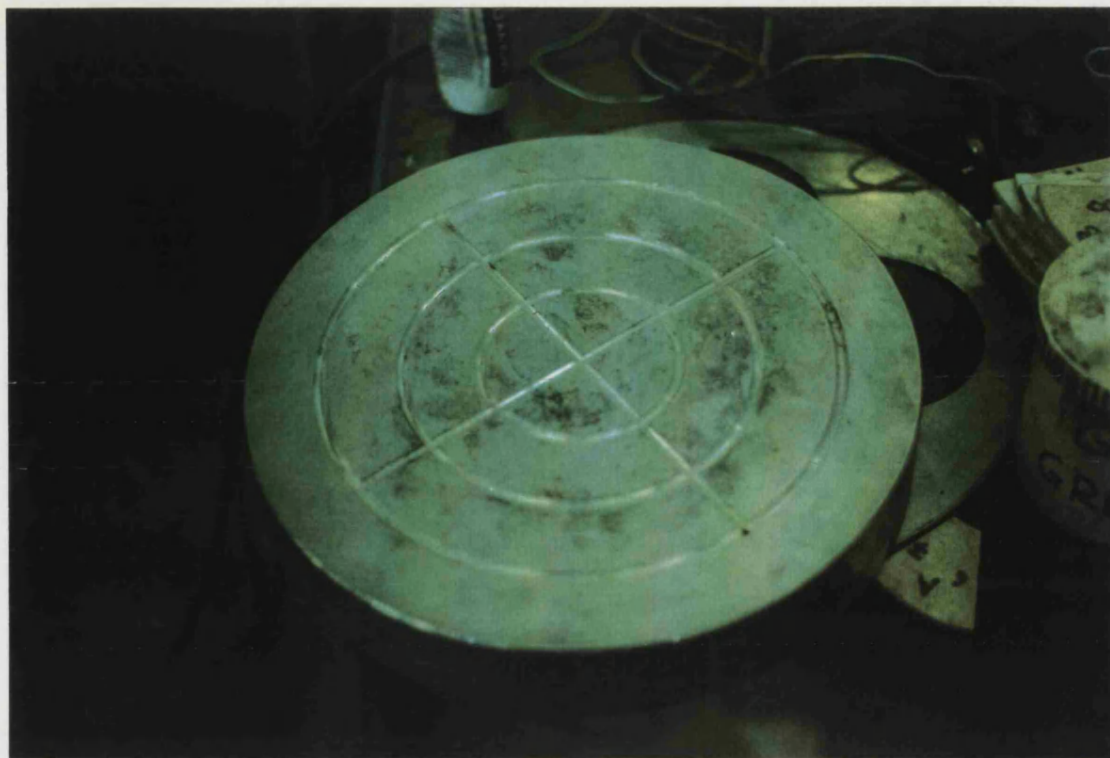


Plate 5.4. Underside of top platten.

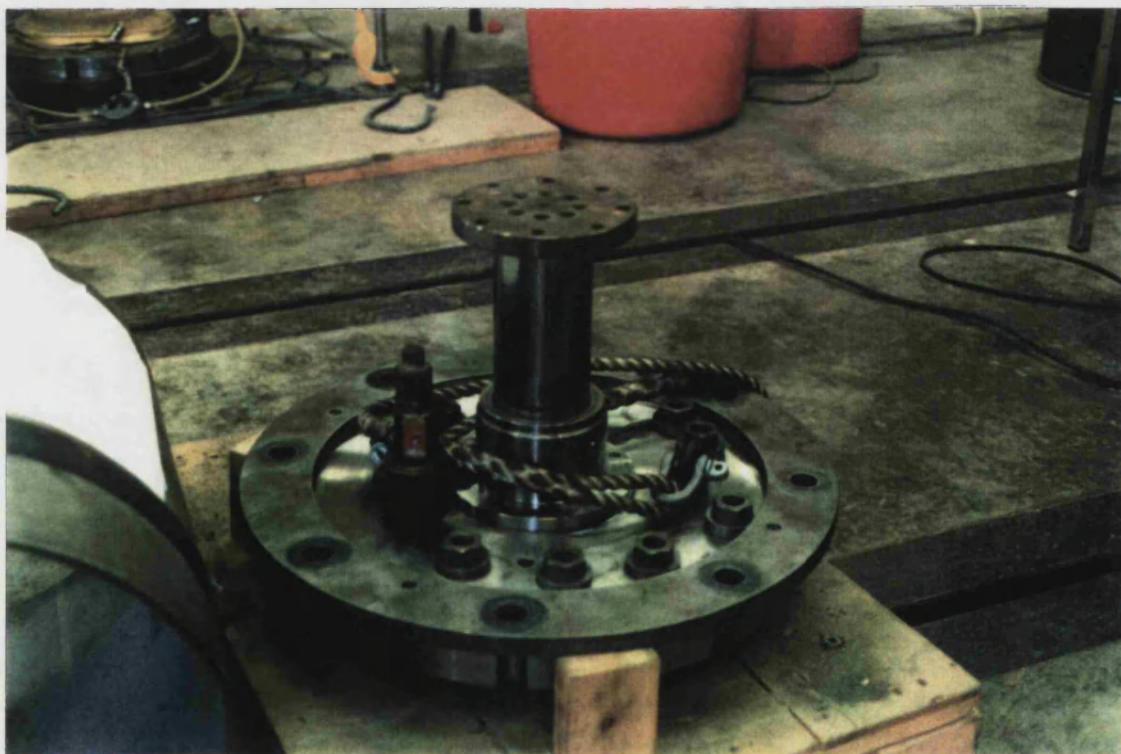


Plate 5.5. Top of cell.

6.0 OPERATING PROCEDURES

This chapter examines the data required in order to abstract the necessary information from the tests and the methods employed to establish this data. Further, techniques and procedures specific to the apparatus and testing methods used will be discussed.

The tests for this study were essentially comparative in nature to determine the damping and shear response of granular samples. Since the extraction of the required data and determination of influencing factors such as gradation and angularity was expected to be difficult to start with, it was felt sensible to limit other factors that might mask these effects. The tests were therefore run dry, removing the need to consider scaling of pore water effects, which might give rise to liquefaction problems in the fine grained samples during the calibration phase, and alleviate difficulties involved with determining pore water pressures. The sample preparation procedure would also be simplified, as factors associated with creating a sample with a uniform porosity would be avoided. This factor can dramatically complicate the formation of samples, especially at higher densities, which can otherwise utilise relatively straight forward techniques, such as the tamping of layers of material.

6.1 VARIABLES TO BE MEASURED

In order to produce values of shear stress and shear strain from triaxial tests, the following factors need to be monitored throughout the test procedure -

- major principal stress
- minor principal stress
- axial strain
- radial strain

From these , the shear stress and shear strain can be determined, and hence the shear modulus and damping ratio.

6.1.1 Major Principal Stress

This is determined from the axial load applied and the cross-sectional area of the sample within the cell. The axial load for dynamic tests is usually continuously monitored by load cells, and recorded in an appropriate manner.

In order to neglect the effect of friction between the loading ram and bush interface, the load cell can be placed within the triaxial cell , either on the cell base beneath the sample, or on the end of the loading

ram above the load platten.

6.1.2 Minor Principal Stress

This is easily established by monitoring the medium used to pressurise the triaxial cell. If static cell pressures are used, then a standard pointer gauge may be adequate. Otherwise a pressure transducer connected to a datalogger can be used continuously to monitor and record the cell pressure. Under dynamic loading the cell pressure may not be constant even if it is meant to be. This is due to a possible time lag for the pressurising system to respond to the sample deformation.

6.1.3 Axial Strain

In standard static triaxial testing, it is common to measure the axial deformation of the sample by monitoring the external movement of the loading ram. However, using optical trackers sighted on to targets attached directly to the soil sample to record the compression of large diameter specimens of granular material, Moore et al (1969) showed that at very low strains, the movement of the loading ram did not represent the true compression of the sample. The relative motion of the loading plattens was also shown to over-estimate the vertical strain. These disparities

are caused by strains resulting from the seating of the loading rod, top plate and specimen ends. Moore et al also noted that direct measurement using strain gauges attached to the sample underestimated the vertical strain. This was attributed to the reinforcement effect of the foil gauges on the specimen in the region of their attachment.

The need to measure small strains accurately during dynamic tests has led most researchers to devise means of measuring the vertical strains by recording the relative movement of points located on the sample itself.

Hicks (1970) located transducers around the sample, secured by springs. However as well as constraining the sample, this arrangement allows the transducers to slip on the surface of the membrane

Brown and Snaith (1974) testing 150mm diameter samples of bitumen macadam secured locating targets to the side of the specimen with araldite. Perspex collars, split to allow lateral deformation and held together by springs, were placed around the specimen on the locating points. The action of the springs kept each collar located on the targets, with small pieces of rubber placed between the collars and the sample to prevent tilting. LVDT's

were then secured between the collars to measure their relative displacement.

A system used by Boyce (1976) on 6" diameter specimens of large grained granular material was to secure locating studs into the side of the sample that projected through the rubber membrane to which LVDT's were attached to measure the relative displacement between two studs in the vertical plane.

Jardine, Symes and Burland (1984) reported on the use of electrolytic levels to measure the axial displacement over a central gauge length on 38mm diameter samples of sands and clays. The devices described could resolve to less than $1\mu\text{m}$ over a range of 15mm. The principle of operation is that a hinged arrangement converts displacements between two footings mounted on the sample into a rotation of a capsule containing an electrolyte and electrodes. The resulting signal gives a measure of the rotation of the capsule, and with knowledge of the spacing of the footings the axial displacement can be calculated. Whilst these instruments appear to give good response during static tests, their use for cyclic loading would need to be examined. The reversing of the loading would be expected to set up oscillations within the fluid of the transducer giving the probability of erroneous results.

Kokusho (1980) used a highly sensitive "gap sensor" consisting of pairs of electromagnetic discs which were sensitive to relative movement. These were situated above the top platten within the cell and could reportedly detect strains as low as 10^{-6} .

6.1.4 Radial Strain

The accurate measurement of radial strain is a particular problem as one needs to record the expansion/contraction of the sample diameter without constraining the material significantly

Brown and Snaith (1974) described the use of hinged perspex collars secured on to targets located on the surface of the specimen, and held together by a spring. As the sample diameter changed, so did the gap between the free ends of the collar, and this was measured by an LVDT.

Boyce (1976) used flexible strain gauged rings to measure the radial strain on 150mm diameter specimens of granular material. These rings were attached to two diametrically opposed locating studs secured into the sides of the specimen. An increase or decrease between the attachment points flexes the rings into an oval shape which is detected by strain gauges bonded to the

ring surface. The rings were manufactured from a casting araldite and made narrower at the strain gauges to give a better deflected shape and increased sensitivity.

6.2 METHODS OF MEASUREMENT ADOPTED

The various methods used in this study to determine the response of specimens tested differed, depending on which size cell was used and whether the loading was static or cyclic.

6.2.1 38mm and 100mm Triaxial Cells

On the 38mm and 100mm static tests using the standard triaxial cells, proving rings were used to determine the load on the sample. These were strain controlled tests and thus the axial deformation of the sample was a function of time, but was also determined with a dial gauge. The cell pressure was provided by pressurised water via an air/water constant pressure system, as described in section 5.2, and was measured using a standard analogue gauge.

For the dynamic tests, the cells were placed within a digitally controlled 100kN fatigue frame, and the load and stroke determined via the frame sensors.

6.2.2 Large Cell

Static cell pressures were provided using a similar air/water constant pressure system as utilised for the smaller cells, and measured in the same way, bearing in mind the comments made in section 6.1.2.

When the static hand operated hydraulic jacks were used for the static calibration tests the load was measured using appropriate external load cells, and the axial deformation determined by strain gauge displacement transducers monitoring the movement of the ram.

When the digitally controlled jack was used for the dynamic tests the external stroke and load could be monitored using the in-built load cell and Linear Variable Displacement Transformer (LVDT). Unfortunately, lack of resources prevented the purchase of an internal waterproof load cell, and thus careful determination of the frictional effect of the bush/load ram arrangement was necessary. This will be dealt with later.

As already noted, in order to assess the true response of the sample to low strain levels the 'on sample' axial and radial deformation was required. The use of the electrolytic level type transducer was considered, but they were not available. The axial deformation was therefore, also established using waterproofed LVDT's

placed inside the cell. These were positioned to measure the relative movement, in the vertical plane, between location studs secured to the sample. These location studs are described in section 6.2.2.1. Each LVDT was secured to the lower location stud and a lipped target element screwed onto the upper stud, so that the LVDT spindle could not slip. The waterproof trunking, secured to the base fittings of the cell, containing the electrical wiring of the LVDT was utilised to carry the self weight of the transducer, thus keeping the weight off the sample. This trunking was coiled in a loop to allow sufficient flexibility to the arrangement for location purposes, and also to provide for the unrestrained deformation of the sample whilst loaded. This arrangement is illustrated in plate 6.1.

In order to determine the radial deformation it was hoped to use non-contacting displacement transducers sighted onto targets attached to the sample. These types of transducer utilise the inductive technique which allows highly sensitive displacement measurement. The major advantages of this system are that the technique does not impose any constraint on the sample, and the system will 'stand alone', in that all the signal conditioning is self contained giving a linear and repeatable, within certain limits, analogue signal that can be recorded directly. Further, the system is robust

within the testing environment. Cost constraints eventually ruled out the use of this method. Since much time and effort was expended on this system, the principal details are contained in Appendix 3.

It was therefore decided to use radial strain rings similar to those utilised by Boyce (1976) made from casting araldite. This necessitated the development of the necessary tooling and skills, and is detailed in the following section.

6.2.2.1 Radial Strain Rings & Location Studs

The strain rings work on the principle of placing a strain gauged flexible ring around the sample on two diametrically opposed locating studs secured to the sample, any expansion/contraction of the sample flexes the strain ring, which can be monitored via the strain gauges, see plate 6.2.

The strain rings were manufactured from a casting araldite which consists of two components, resin and hardener. A variety of resin/hardener combinations, as well as other components such as mineral fillers, are available to produce finished materials with particular properties and workability. The main industrial useage of these products is within the electronics industry and

the material properties are generally directed to that field. The combination of resin/hardener considered most appropriate to the strain ring application was epoxy resin CY 1302 GB, which was available in the appropriate quantities and included mineral fillers to reduce shrinkage and decrease the coefficient of expansion, and hardener HY 1300 GB. This combination had the most suitable thermal expansion coefficient, elasticity, strength, workability and reaction time.

The procedure for the manufacture of the strain rings was as follows.

1. Former prepared with cleaning agent.
2. Release agent QX11 applied to former. This release agent is based on caruba wax. It is important that the release agent does not contain silicon, otherwise the strain gauges will not adhere to the strain rings.
3. Resin and hardener mixed in the ratio of 100:11.1 by weight or 100:19.5 by volume, taking care to minimise air bubbles within the mixture, and then...
4. poured into former whilst placed on a vibrating table/support so that air bubbles were forced to the surface.
5. Left to cure for 48 hours.
6. Removed from mould and checked for imperfections.
7. Rings placed in oven at 40°C for 28 days, to ensure material fully cured and material properties stable.

8. The rings were then placed in the digitally controlled 100kN frame and subjected to a series of displacement cycles.
9. Rings that survived stage (8) were then strain gauged, with each pair of gauges wired in a half bridge arrangement, and calibrated.

As the rings would be used immersed in water, the strain gauges, wiring and connections to the gauges had to be waterproof. This was achieved by selecting appropriate cabling and sealing the gauges and direct connections with a wax compound. Each individual strain ring was calibrated such that the readings from the strain gauges could be correlated to the expansion/contraction of the rings. Only those rings with repeatable calibration graphs could be used for the dynamic testing program. This calibration procedure is detailed in section 8.1.

In order that the strain rings could be located around the samples location studs were secured to the sides of the soil samples. These studs were also used to provide fixing points for the LVDT's measuring the axial deformation. These location studs are shown in figure 6.1 and plate 6.3, and consisted of two elements which sandwiched the membrane providing a secure location position. It was important to ensure a watertight seal around the membrane, otherwise the cell

water could enter the sample.

A hole was needed in the membrane at the position of each location stud where the two elements screwed together. In order to prevent tearing, the membrane was reinforced at these points with ordinary cyclist puncture repair material, and a hole was then punched through the resulting 'sandwich'.

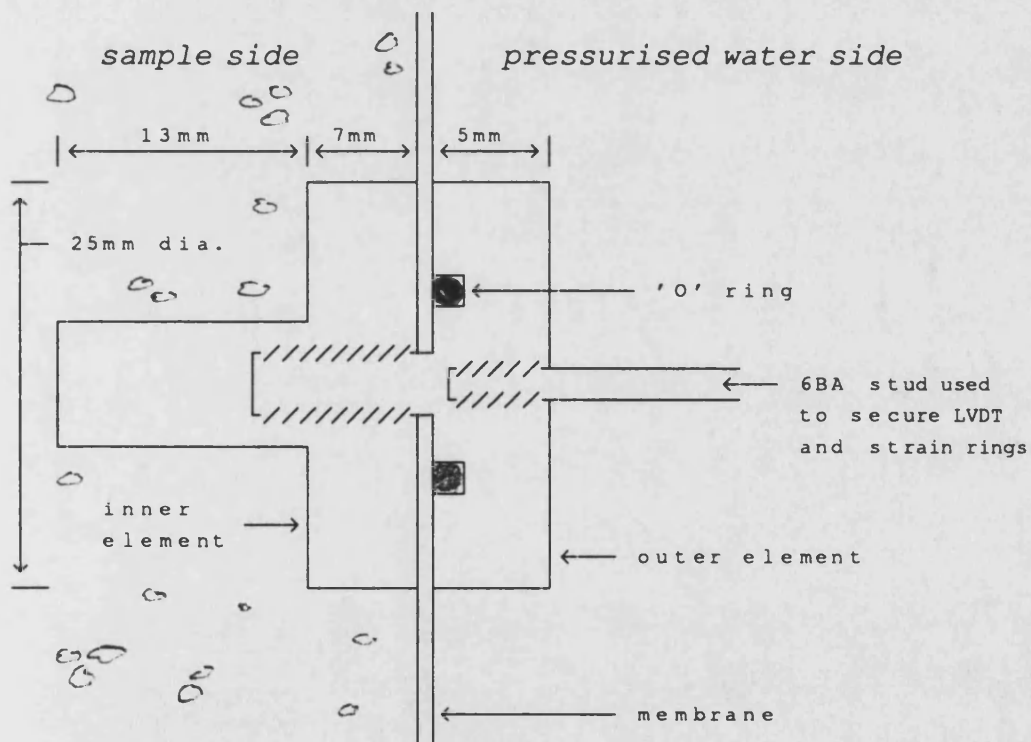


Figure 6.1. Details of location studs

6.3 FRICTION FREE END PLATTENS

Traditional triaxial cells use samples with a height to diameter ratio of 2:1. This is in order to ensure that a representative failure is achieved within the body of the sample, free from the end effects of the top and bottom plattens. It is the friction between the plattens and the ends of the sample that causes the characteristic barrelling of triaxial cell samples.

As discussed in section 5.1 due to a number of reasons the large cell was constructed with insufficient height to allow 2:1 ratios to be used for the larger sample diameters. At the larger diameters tested, ratios approaching 1:1 were used, necessitating the use of friction free end plattens in order to allow free movement of the sample on the end plattens.

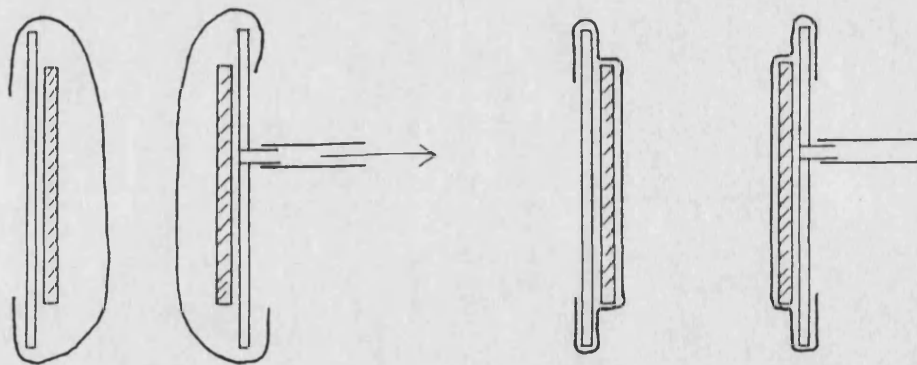
Various arrangements were tried to achieve friction free ends. The most successful consisted of PTFE discs cut into segments placed between the sample and end plattens, plate 6.10 shows the placing of the upper 'free end'. One face of the PTFE was 'dimpled' and this face was placed towards the platten. Silicon grease could be placed between the PTFE and end platten to lubricate the assembly. Experiments with smooth face materials and various lubricants proved unsatisfactory

since in such circumstances the lubricants acted more like glues preventing sliding. The 'dimples' reduced this problem. However, it should be noted that this solution did not prove to be entirely successful due to the use of an internal vacuum to form the free standing samples of granular material. This will be expanded upon in section 8.3.

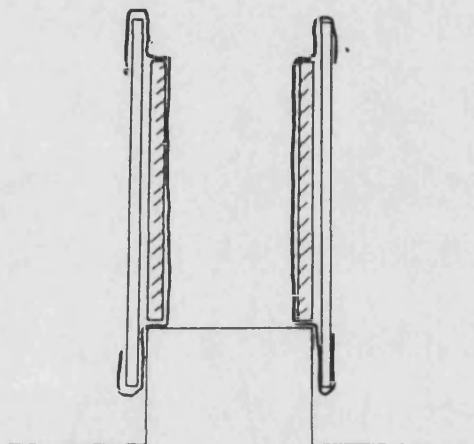
6.4 SAMPLE PREPARATION

The testing of granular material in triaxial cells requires the ability to prepare free standing samples of particulate material. This is achieved in the following manner.

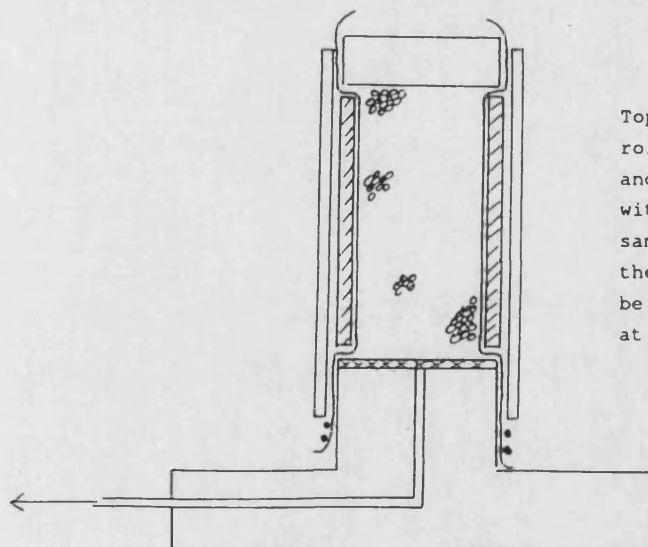
The membrane is first stretched within a metal former and a vacuum applied to pull the membrane flat against the inside of the former. The former with membrane attached is then placed over the bottom platten. The material that will constitute the sample is then placed within the former in an appropriate manner in order to achieve the correct density, this will be discussed later. Once all the material is appropriately compacted the top cap/platten is placed on the top of the sample. The membrane is then rolled off the outside of the ends of the former and sealed around the top and bottom platens with O rings. A vacuum is then applied to the



Membrane stretched over formers and vacuum applied to pull membrane flat against former to produce a flat surface.



Former assembly placed over base platten, ensuring any base fittings, eg. friction free end plattens or filters, are already in position. Sample material can then be placed within the former assembly.



Top platten inserted, and membrane rolled off top and bottom of former and sealed around top and base plattens with 'O' rings. Vacuum applied to sealed sample allowing removal of former. With the sample now free standing the cell can be assembled and the cell pressure applied, at which point the internal vacuum is released.

Figure 6.3. Sample preparation sequence

now air tight sample, which allows the former to be removed and the sample to remain free standing. The cell apparatus can then be assembled and the confining cell pressure applied at which point the internal vacuum can be released. This sequence is illustrated in figure 6.3, and plates 6.5 to 6.7 for the 100mm cell, and 6.8 to 6.11 for the large cell.

Each cell size used different formers and methods to apply the vacuum/suction pressures. The large cell used an inner and outer former. The outer was a split steel cylinder which fitted over the outside of the base platten, whilst the inner was a split wooden former that produced a nominal sample diameter of 254mm, and incorporated cutouts for the location studs so that a smooth sided sample could be produced. This inner former also allowed an even distribution of the vacuum to pull back the membrane. The use of lubricated end plattens meant that the base lubricated end had to be prepared prior to the placement of the former over the bottom platten, and at the top prior to the top platten. The vacuum was applied using an electric vacuum pump. The 38mm and 100mm cells used smaller sized metal outer formers and split plastic inners around which the membrane was stretched. Manual suction was sufficient to pull the membrane back against the formers, but the electric pump was used to evacuate the samples.

The various formers for each of the cells are shown in plate 6.12.

6.5 MEMBRANES

The membranes used in this study were made from latex rubber. Standard sizes existed for the 38mm and 100mm sample sizes, although these were more appropriate for use with clay samples and very fine sands. Care had to be taken when using larger grained material in order to avoid tearing. For the large cell special membranes had to be made. The standard material thickness, as used on the smaller cells, made to the required diameter was suitable for fine grained material, but was punctured by the larger particles. The thicker the membrane, the more resistant to tearing, however it also provides more restraint to sample deformation so influencing the material response. Tests with a selection of different membrane thicknesses at various cell pressures enabled 'appropriate' membranes to be selected. Problems were encountered during the testing due to membrane failures. These membranes deteriorate over time, as well as with use, and are very susceptible to sunlight. As will be discussed later, it was found necessary to utilise additional protection to prevent the membranes being punctured by the angular material used in this study at high cell pressures.

An illustration of the different membranes is given by plate 6.12.

6.5.1 Membrane Corrections

As mentioned in section 6.5 the membrane will provide some restraint to the sample. This can be accounted for by considering the nature of the sample deformation and the stiffness of the membrane. The nature of sample deformation is traditionally considered as either plastic, barrel type failure, or brittle, where a defined slip plane develops prior to failure.

For plastic type failure a correction factor may be calculated using an equation proposed by Henkel and Gilbert (1952), which in terms of kN/m^2 units is given below

$$C_m = \frac{0.4.M.\epsilon.(100-\epsilon)}{D} \quad [6.1]$$

where

C_m = membrane correction, units kN/m^2 .

M = Modulus of membrane material, units MN/mm width, as determined from extension tests on samples of the membrane material.

ϵ = axial strain of sample, units %.

D = Initial diameter of sample, units mm .

However, more recent research - Wesley (1975), Sandroni (1977), Gens (1982) - has indicated that this equation overestimates the correction to a varying degree. Wesley concluded that the correction factor depended on the deviator stress, but was unlikely to be more than 3% of the peak value. Gens proposed that no correction was required at all. Sandroni produced his results in the form of graphs, indicating that for a 38mm sample with a membrane 0.2mm thick, the correction factor would be a maximum of 2kN/m² at 20% strain. Factors for other diameter samples and/or membrane thicknesses could be ascertained by multiplication by the following;

$$\frac{38}{D} \times \frac{t}{0.2}$$

where

D = sample diameter [mm]

t = membrane thickness [mm]

With sample diameters of 254mm, as used in this program, any correcting factors thus determined become negligible.

The correction for the brittle type failure is generally given in graphical form, and is again dependent on factors such as sample diameter, membrane stiffness and axial strain. The correction values for the brittle type

failure are generally greater than for the plastic type failure. However in both cases, unless the membrane offers excessive resistance, the effect is small when dealing with high strength materials. Also, as the size of sample increases the effect is reduced, as can be deduced from equation 6.1.

A further membrane effect that needs to be mentioned is that due to the indentation of the membrane around the surface of the sample due to the confining pressure. This has the effect of reducing the volume of the sample and can influence volume change measurements. It is of most concern during undrained saturated tests where any volume change of the sample may affect the pore water pressure measurements. Obviously this factor will be influenced by the particle size and grading of the sample, being greatest for single sized large grained particulate material.



Plate 6.1 LVDT fixed in position between two location studs.



Plate 6.2 Strain Rings positioned around the sample.



Plate 6.3a Close up of location stud and fixing for strain rings.

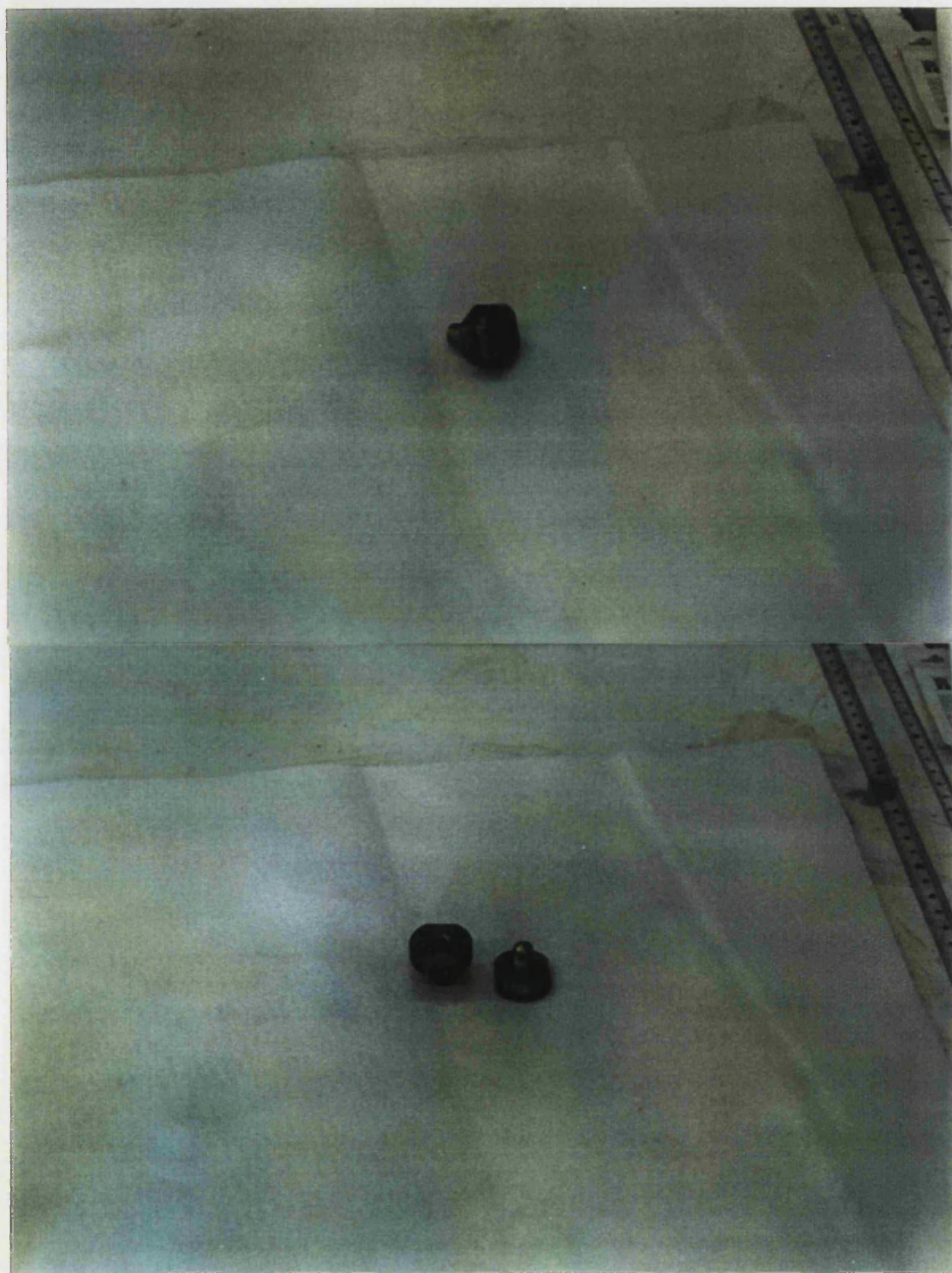


Plate 6.3b Location stud in more detail.

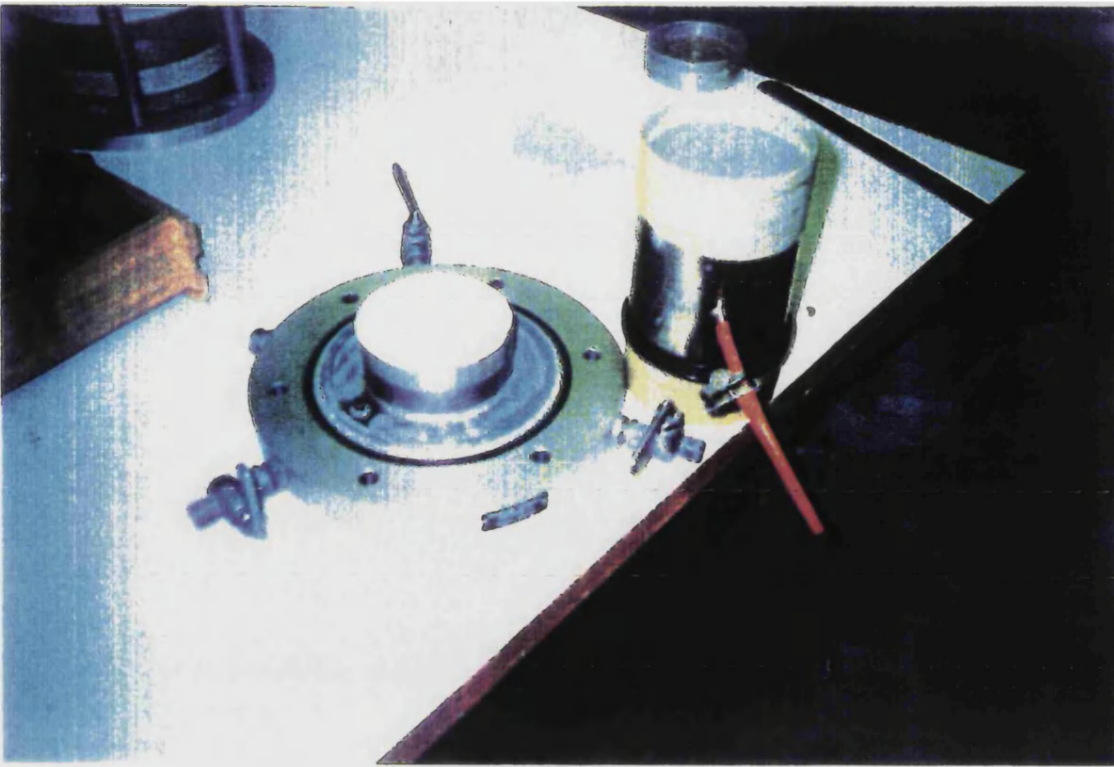


Plate 6.4 100mm sample preparation sequence. Base plate and former with membrane held back by vacuum.

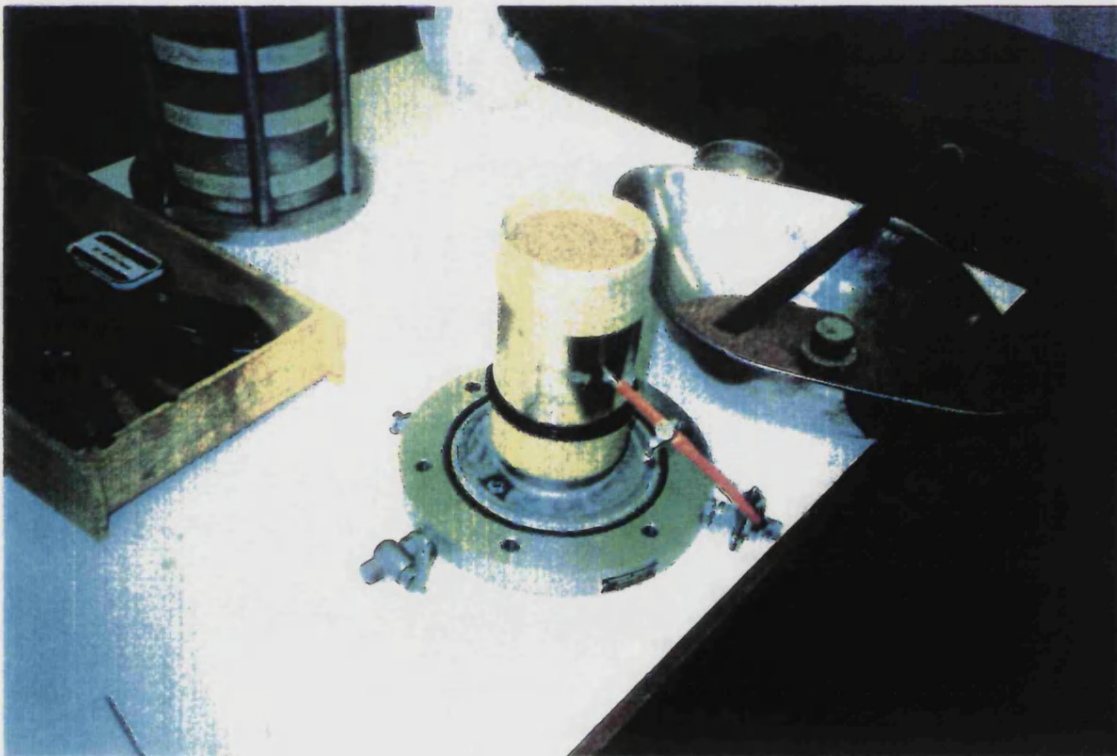


Plate 6.5 100mm sample preparation sequence. Sample material placed in former on base plate.

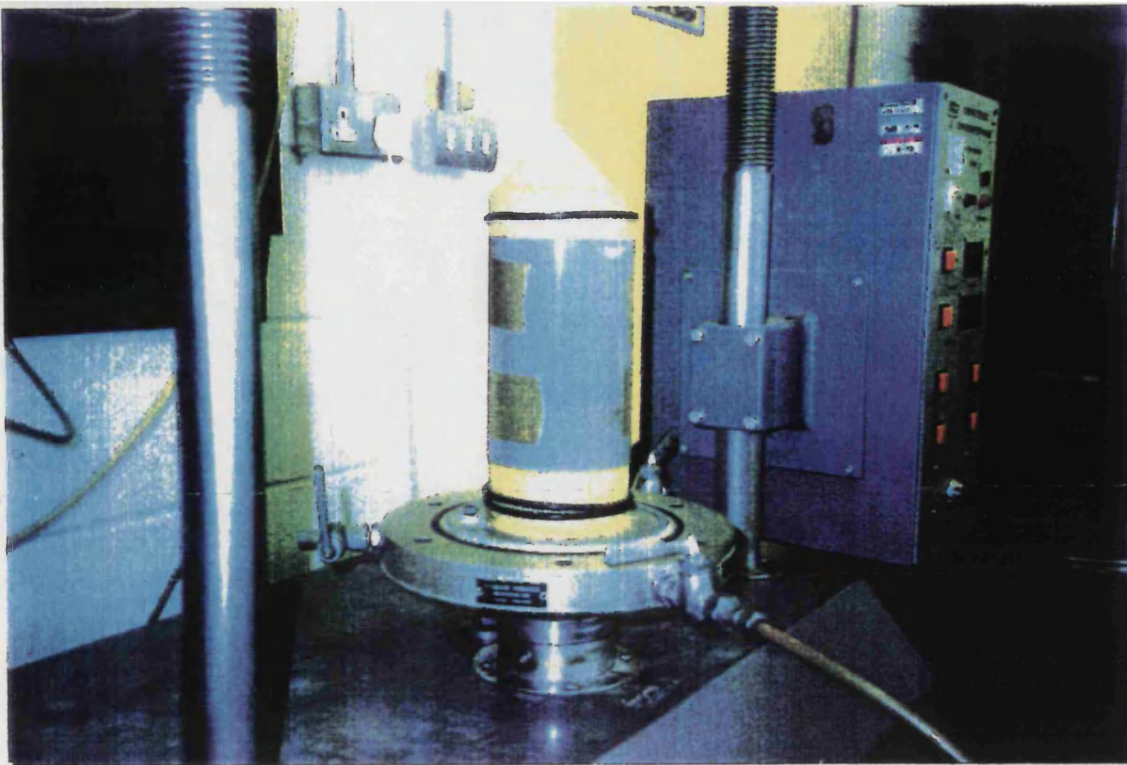


Plate 6.6 100mm sample preparation sequence. Top platten placed in position and membrane sealed to top and bottom platens. Vacuum applied to sample.

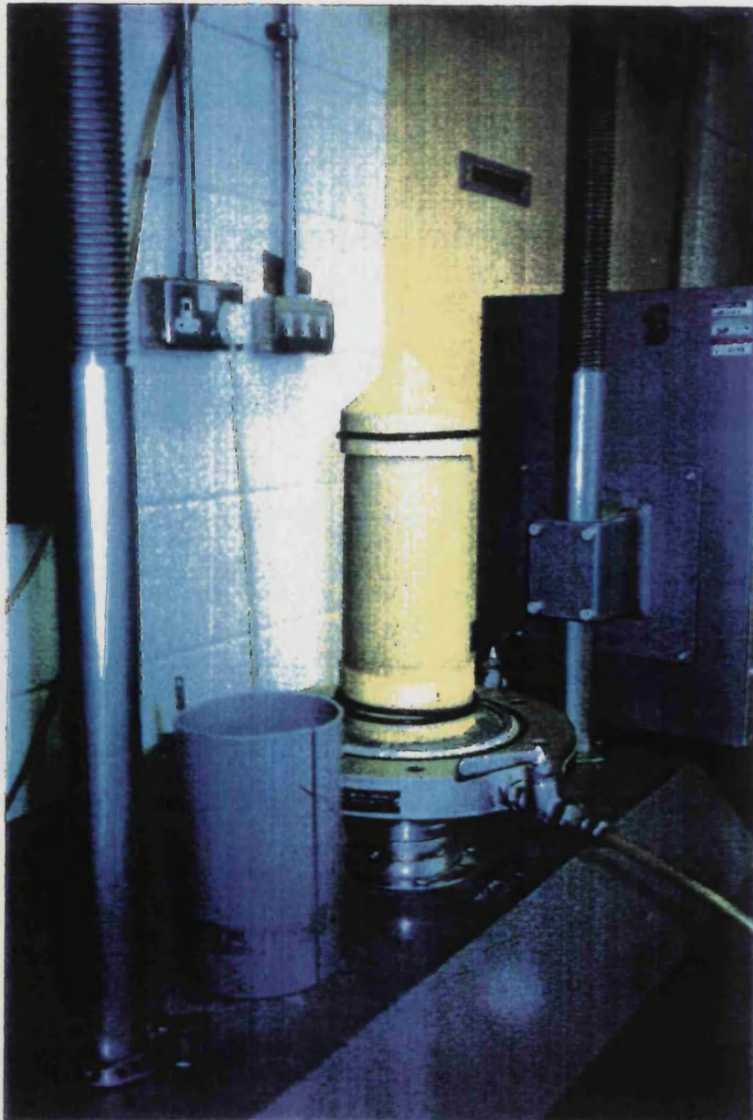


Plate 6.7 100mm sample preparation sequence. Former removed, and sample free standing prior to cell assembly.

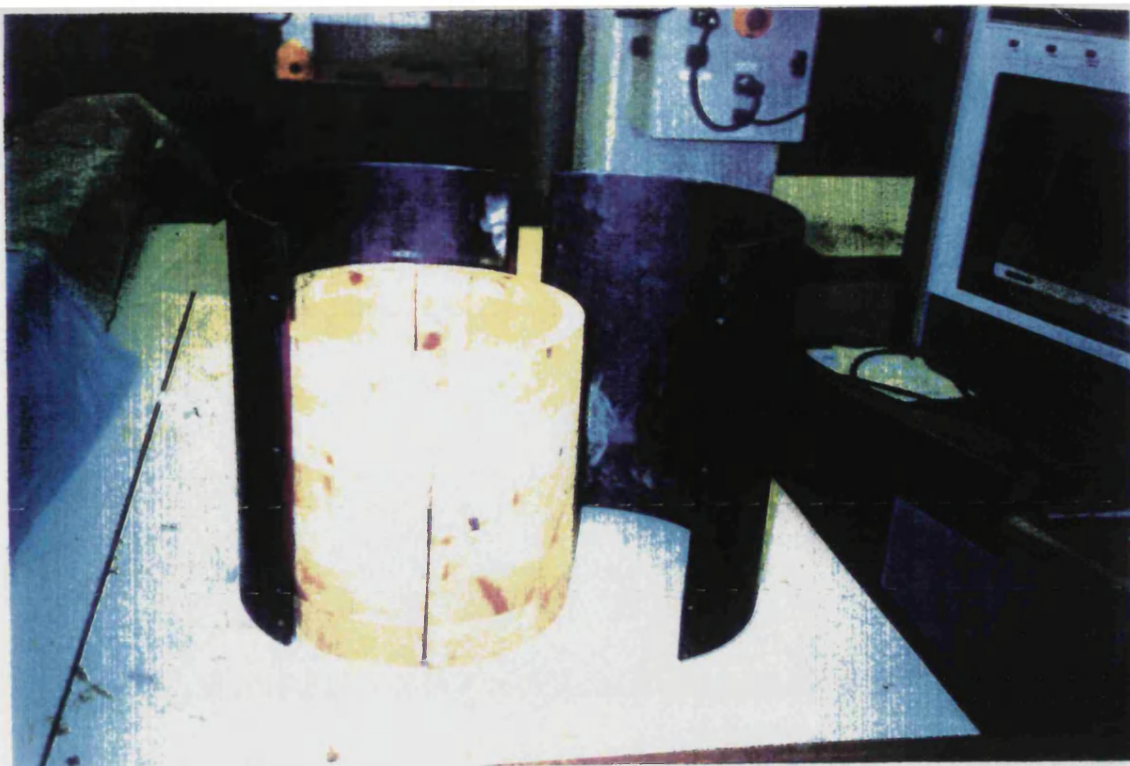


Plate 6.8 254mm sample preparation sequence. Internal and external formers.



Plate 6.9 254mm sample preparation sequence. Formers placed over base platten with membrane held back by vacuum.

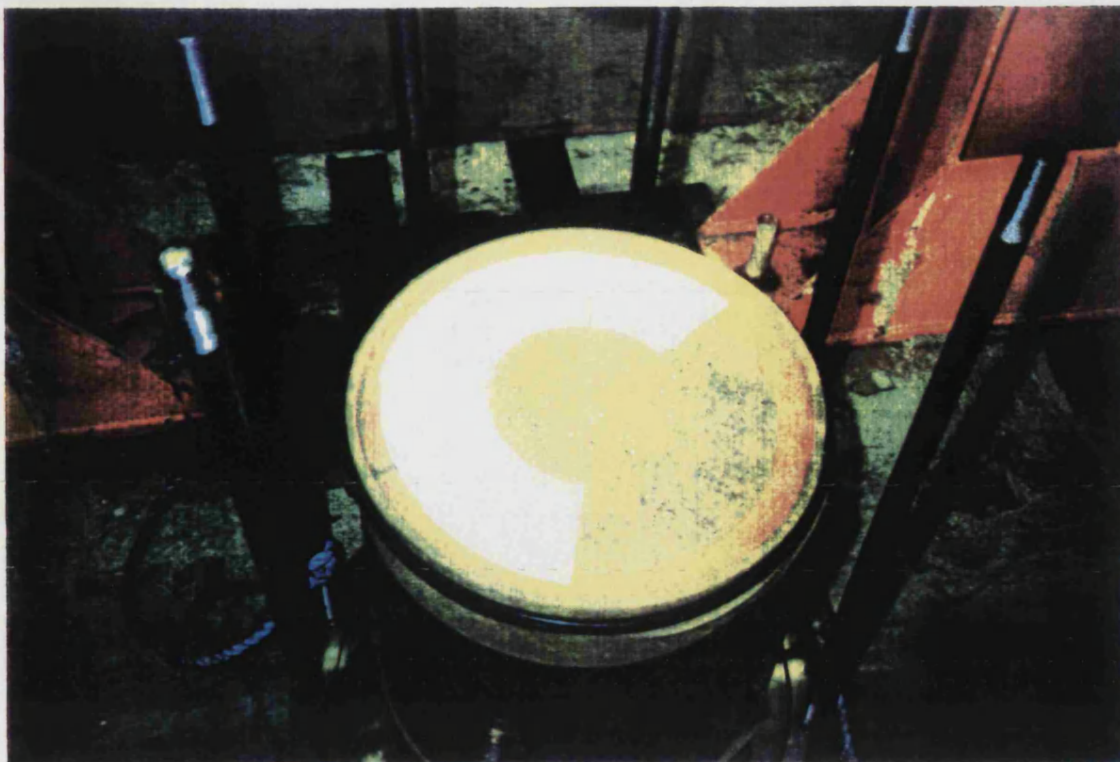


Plate 6.10 254mm sample preparation sequence. Material placed within formers, and friction free top platten being positioned.



Plate 6.11 254mm sample preparation sequence. Free standing sample with top platten in position and mambrane sealed top and bottom. Vacuum applied.

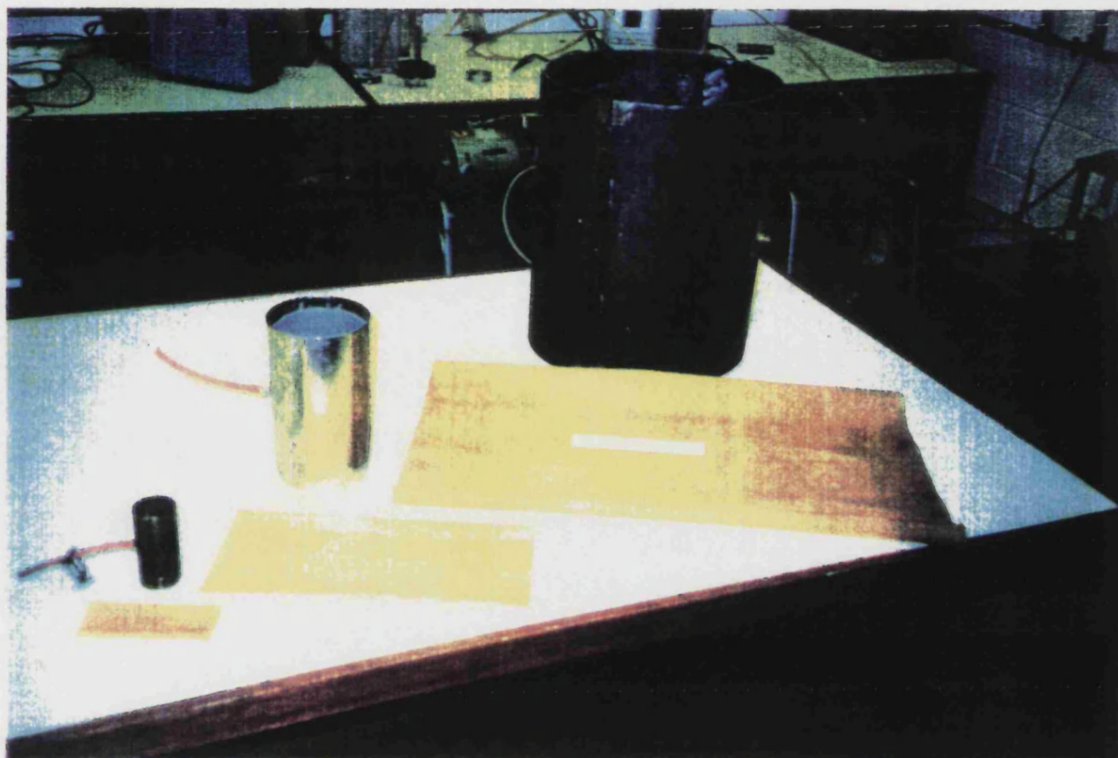


Plate 6.12 Comparison between formers and membranes for the 38mm, 100mm, and 254mm diameter samples.

7.0 INSTRUMENTATION

The way in which data is recorded and presented often affects our comprehension of the mechanisms that the data represents and also our response to such data. It is important to be aware of any influence the recording methods and apparatus may have on the data being monitored. Early work on the dynamic behaviour of soil samples that relied on the real time plotting of the hysteresis loops from dynamic tests, led to the 'real' data being modified as a result of the damping inherent in the mechanism of the plotter. This additional damping resulted in the hysteresis loops having rounded ends which in turn affected the calculation of the damping component, and also complicated the determination of the shear modulus. The wiring and interconnecting elements of a monitoring system need careful attention in order to not modify the 'signal' and to thus record the true sample response.

Within this study various forms of sensors were used to monitor the response of samples. The static tests on the 38mm and 100mm samples utilised various direct read instrumentation such as proving rings, dial gauges and analogue gauges. As long as these instruments were read perpendicular to their faces, and correct conversion charts used, and suitable calibration certificates

available, they presented few problems. The remote monitoring of electrical sensors required appropriate consideration.

The remote data logging system was based around a programable data acquisition unit (ADU) that could be configured to monitor up to 64 channels, dependent on the modules fitted. A PC compatible computer was connected to the ADU via an RS232 interface to provide control; this also allowed real time screen plotting on the PC of the monitored channels against time. As the frequency of the loading cycles was low the data transfer rate of the serial interface was not a problem. Data was also logged by the ADU, then down loaded to the PC after each test, at which time the data could be processed and X,Y plots produced. The sensors used in this program varied from proprietary systems, to instruments manufactured by the author specifically for this program of study. The sensors used and components of the monitoring system will be explained in the following sections. The complete system is shown diagrammatically in figure 7.1.

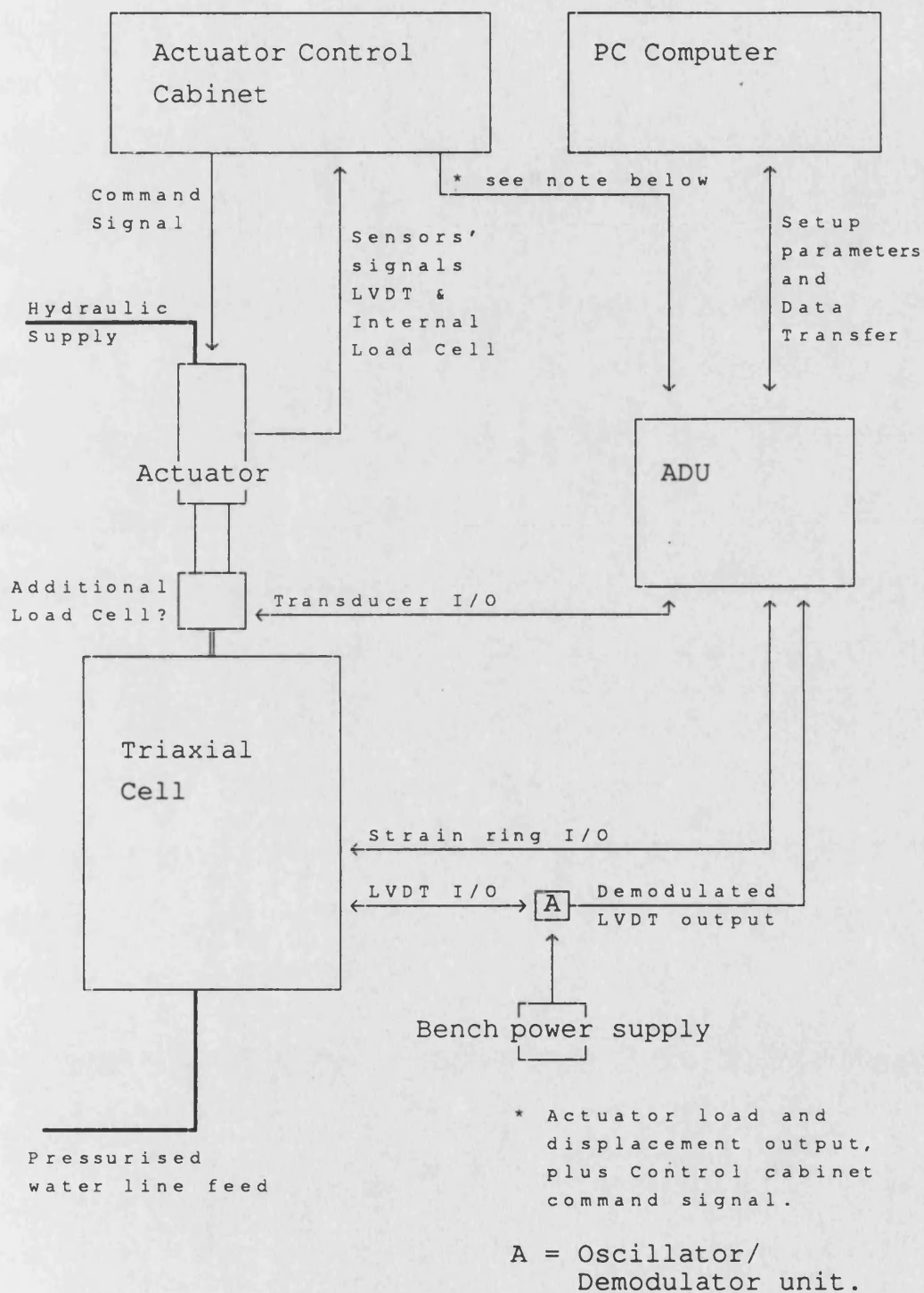


Figure 7.1. Monitoring and control system (Diagramatic)

7.1 STRAIN GAUGE DISPLACEMENT TRANSDUCERS

These are proprietary displacement transducers that utilise the strain gauge Wheatstone bridge arrangement to give an electrical signal proportional to linear displacement. The use of strain gauges is commonplace within the industry, and there is no need to go into detail as to their workings. The transducers available for this project utilise the cantilever principle, whereby a wedge connected to the spindle forces apart two cantilever beams housed within the body of the transducer. Transducers of this type tend to have a long body length relative to the range of movement. Each cantilever beam has two strain gauges wired in the half bridge arrangement. The resulting wiring out of the transducer is a four wire connection giving supply +ve/-ve and signal +ve/-ve. The instruments each have a calibration certificate detailing the response characteristics and sensitivity, and these are included within appendix 4. The transducers require a DC supply voltage which was supplied from the ADU in the form of +5V to -5V, giving 10V. The resulting output of the gauges used was between 37mV and 54mV.

7.2 LINEAR VARIABLE DISPLACEMENT TRANSFORMERS (LVDT)

These are proprietary instruments that work on the principle of the variation in current induced by the movement of a conductor within an alternating electrical field. The system involves a primary coil connected to the supply current, and two secondary coils connected such that with the spindle in the middle of its travel there is no phase difference between the excitation and output signal. Any movement of the spindle from this null position results in a phase shift between the supply and output current which can be analysed and converted into a variable voltage signal proportional to the displacement of the spindle. The primary coil requires an AC power source in order for the instrument to function. This is normally provided via an oscillator demodulator unit connected in line with the transducer, which also deals with the initial signal conditioning, converting the phase difference into a variable signal that can be monitored with a normal data logger. These transducers are normally much shorter relative to their range of movement because the operating mechanism is much more compact. LVDT's allow very high resolution which is useful when measuring low strain levels. The devices used in this study were placed within the cell and thus the main body and connecting wires had to be waterproofed. The oscillator demodulator unit required a

supply voltage between 12 - 24V DC, which was outside the range of the ADU necessitating the use of a separate bench power supply. The output signal from the oscillator demodulator unit varied between $\pm 6V$ and was logged by the ADU.

7.3 STRAIN RINGS

These were used to assess the radial expansion of the samples. The principle, design and manufacture has been detailed elsewhere, we deal here with the handling of a signal from these devices. The strain rings as already described each made use of two pairs of strain gauges wired in 1/2 bridge arrangements. Due to the versatile nature of the ADU, these could be directly wired into the appropriate board. The ADU powered the bridge circuit with a constant current source, and monitored the output signal. It must be noted that extreme care must be taken when wiring up bridge arrangements to ensure that only the variable resistance of the strain gauges themselves is recorded. In particular, the wiring connections to ensure the common potential of the measuring circuit must be made at the gauges, and not at the connector. This is illustrated in figure 7.2. Figure 7.2(a) illustrates the correct wiring connections showing the sense, input, and output connections being made at the gauges themselves. A common misconception

is that since the pairs of wires ab,cd, and ef are connected at common points, that the same electrical configuration is achieved by the simpler arrangement illustrated in figure 7.2(b). In this case the wires are paired at the connector. Whilst this simplifies the wiring, it adds extra resistance by including part of the wiring circuitry within the 'bridge' circuit. This reduces the sensitivity, and increases the possibility of changes in reading without a change in the resistance of the gauges themselves.

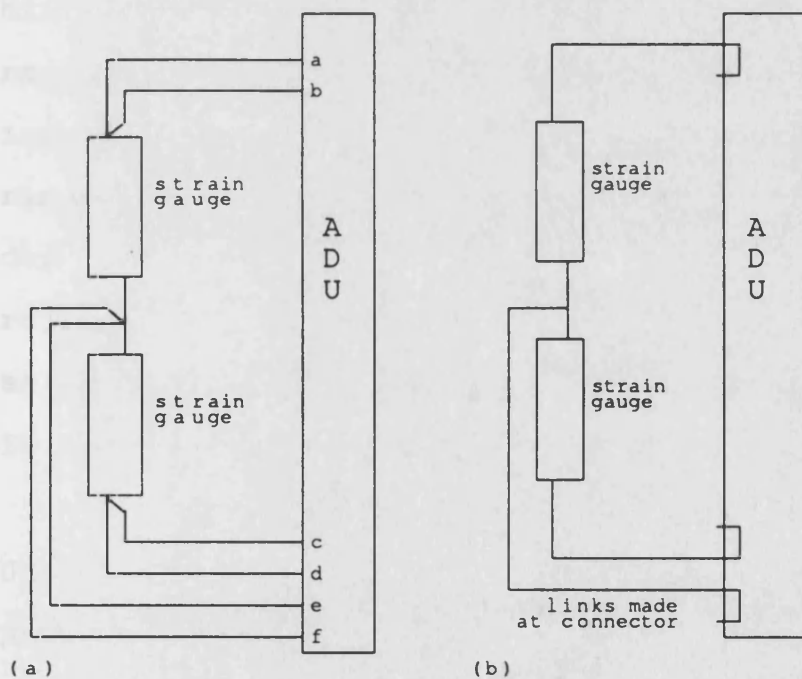


Figure 7.2. Common potential wiring details for 1/2 bridge arrangement. Showing (a) correct, and (b) incorrect method.

Each of the strain rings had to be individually

calibrated to give factors to convert the output readings from the 1/2 bridges to expansion/contraction of the diametrically placed fixing points. These tests and charts are detailed in section 8.1.

The supply source and wiring details for the 1/2 bridge arrangement is given in section 7.4.

7.4 DATA ACQUISITION UNIT (ADU)

The ADU is a programmable data logger utilising a 12 bit A/D converter, which allowed for a maximum possible resolution of 4095 units over the +ve/-ve full scale input range. This was split between 2048 for the +ve range, and 2047 for the -ve range; the difference is due to the use of 2's complement notation by the ADU to represent negative numbers. The system allowed selectable gains over a range between $\pm 10\text{V}$ to $\pm 9.8\text{mV}$ in 11 ranges as shown in 7.1.

Using the calibration certificates provided for the proprietary transducers and determining the appropriate gain ranges allowed the resolution of the measurement signals to be maximised. Entering conversion factors into the controlling software meant that the various sensors could be monitored in real engineering units.

Gain No.	Signal Range	Resl. ⁿ	Gain
0	±10V	4.88mV	*1
1	±5V	2.44mV	*2
2	±2.5V	1.22mV	*4
3	±1.25V	0.61mV	*8
4	±625mV	305μV	*16
5	±312mV	153μV	*32
6	±156mV	76μV	*64
7	±78mV	38μV	*128
8	±39mV	19μV	*256
9	±19mV	9.5μV	*512
10	±9.8mV	4.8μV	*1024

Table 7.1. Gain Ranges and Resolution of ADU.

The PC controlling computer utilised software provided by the suppliers to configure the ADU and process data; direct commands were also sent to the ADU on a number of occasions to simplify the logging procedure. A number of software packages have been developed by the author during this programme that provide more direct control of the ADU functions. Use of the RS232 interface with direct ASCII 'string' commands enabled a small hand held computer/processor to control the data logging operations. This increased the portability of the system and proved ideal for the early preliminary work. For the screen display of a number of channels and graphical output of data the PC was the more appropriate mechanism and provided the facility of screen dumps to printer.

The excitation voltage for the 1/2 bridge strain gauges utilised a 8.33mA dual current source, which resulted in the six wire connection shown in figure 7.3.

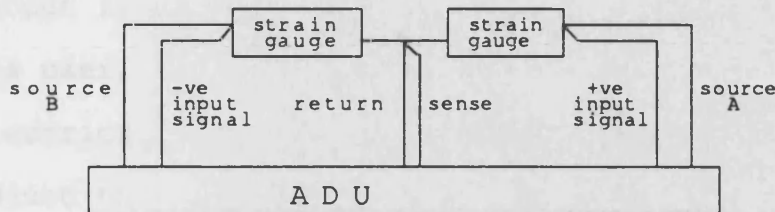


Figure 7.3. Six wire connection for 1/2 bridge.

7.5 DIGITAL CONTROL SYSTEM FOR HYDRAULIC ACTUATORS

This is a state of the art modular digital control feedback, monitoring, and plotting system, manufactured by DARTEC under their 9500 series, incorporating analogue monitoring devices and digital processors.

The hydraulic actuators connected to this control system all incorporate load cells and LVDT's which provide signals that can be monitored by the system controller. These signals indicate the exact position of the actuator ram; the digital control system operates by incorporating these signals into a feedback loop. This allows the operation of the actuators in either stroke or load control. Although, in essence, all tests could be considered to be 'positionally' controlled, since, it

is hydraulic fluid under pressure acting against the ram that induces the load. This hydraulic fluid passes through a Moog valve which allows remote control by electrical signal, thus varying the rate of flow, and porting of fluid pressure to the ram. By comparing the output from the sensors with expected values entered by the user, or calculated from setup information, the electrical input to the Moog valve can be varied to adjust the direction of movement of the ram and thus the load applied to the sample to follow a predetermined ramp. It is the fast and accurate control of this electrical system, and matching to the hydraulic response of the mechanical hardware that allows the successful operation of this system. The hardware provides the response of the actuator sensors as a $\pm 10\text{v}$ signal, and these were also recorded by the ADU.

The actuator response is a function of the electrical feedback loop and the mechanical hydraulic system providing the hydraulic pressure, which in turn is dependent on the capacity of the valves incorporated on the actuators. The response of the feedback loop was dependent on the electrical gains incorporated within the system, which could be varied to suit the stiffness of the sample under test. It is important to match the feedback parameters of the control system to the expected stiffness of the sample if testing 'flexible'

specimens. The only variable on the pressure supply system was the pump supply pressure, and this was held constant at the actuator setting of 207 bar (3000 psi).

7.6 LOAD CELLS

All the load cells used in this study employed the full strain gauge bridge principle, in basically the same manner as a strain gauge displacement transducer. In fact, as far as the ADU is concerned the two instruments are identical. Two types of load cell were used: indicating load blocks, and those requiring an external data logger. The indicating type contain their own power source (9V battery, or mains adapter) and signal conditioning, giving a direct readout of load on a digital display. The resolution is predetermined by the inbuilt electronics and limitations of the display. Those requiring an external data logger utilised the ADU to provide a 10V supply and to monitor the output signal. The resolution of the recorded data could be varied, within the tolerance of the bridge arrangement itself, by selecting different gain settings on the ADU. The output is scaled by user defined variables.

PART III

Testing Program

Includes all information pertinent to the tests so far completed and their interpretation, the results, conclusions and suggestions for further work.

8.0 LABORATORY TESTING PROGRAM

This section deals with the testing program carried out to date, and covers theoretical aspects associated with selection of test method and data interpretation. The development of the strain rings was an integral part of the apparatus for the dynamic testing, and so their calibration is covered in some detail.

The developmental nature of this thesis work has meant that certain areas of 'laboratory' work have already been covered. The full laboratory program consisted of various stages, a brief listing of which follows.

(1) Modification of original static loading equipment to allow dynamic testing. This involved developing a computer controlled stepper motor system to adapt the constant air/water system for cyclic loading. This was outlined in section 5.2 with additional information in appendix 2. In the event this work was not carried forward.

(2) Commissioning of large cell and pressure testing. Following manufacture of the new triaxial cell, the apparatus was assembled and pressure tested to ensure testing could be performed safely. As detailed in section 5.1 the cell was pressurised to 4137 kN/m^2 (600

psi). The pressure release valve was set at 1379 kN/m^2 (200 psi) providing a substantial factor of safety.

(3) Fabrication of strain rings. This was described in sections 6.2.2.1 and 7.3, covering the methods and materials required..

(4) Calibration of strain rings. The nature of the strain rings meant that they had to be carefully calibrated in order to provide useful data. Full operational characteristics of the rings is detailed in section 8.1.

(5) Initial static comparison tests between 38mm, 100mm, and 254mm diameter samples on similar material. The primary aim of these tests was to establish if there was any scaling effect between the different cell sizes.

(6) Extended program of comparable static testing. As outlined in the introduction, the developmental phase comparing the results from the different cells on similar material, suggested that the different cells yielded different results. This required further investigation to verify comparisons between the cells. Whilst smaller grained material could be tested in each of the triaxial cells, only the 254mm cell could be used

for the larger grained material, and thus cell size effects would need to be assessed if valid comparisons were to be made.

(7) Static tests on large grained particulate material (Limestone grit) in 254mm cell. Single sized material tests to enable comparisons between graded and different single sized samples.

(8) Cyclic loading tests. The additional static tests detailed above, and operational difficulties curtailed much of this testing program. Preliminary testing is described and operational characteristics detailed.

A preliminary requirement to the initiation of the testing program was the commissioning of the new structures laboratory facilities. These included a hydraulic ring main feeding four outlet stations to which hydraulic actuators could be connected.

Hydraulic pressure was provided by two pumps interlinked by a common oil reservoir. The pumps fed independently into the ring main, with an isolation valve allowing independent and simultaneous use of the two pumps to feed different actuators. Alternatively, if the pressures of the pumps were matched they could both feed into the open system giving an increased flow rate, resulting in a faster response of the actuators. A

wiring loom linked the four outlet stations, both pumps, and pump control panels allowing the interlinking of the digital control cabinets between the various framed and loose actuators, as required. The control cabinets could be interchanged between stations, which was essential for the dynamic testing.

The large triaxial test facility, which consisted of the triaxial cell, steel 'H' frame with stabilising arms, and either 50kN or 100kN hydraulic jacks, and associated data logging and control equipment, was placed on an area of the strong floor as shown in plate 8.1. The horizontal cross member of the 'H' frame was movable which allowed the interchange between the 50kN fatigue actuator, and the 100kN static jack arrangements.

8.1 STRAIN RING CALIBRATION

Following the procedure outlined in section 6.2.2.1 each of the strain rings required calibration to determine their suitability for the test programme.

Each ring was placed within a hydraulic testing frame as shown in plate 8.1, and subjected to a cyclic ramp under displacement control, at a frequency of 0.1 Hz. Maximum stroke amplitude was $\pm 2.5\text{mm}$. The response of the strain rings, ram displacement and load applied as determined

from the control LVDT and load cell, were all monitored and logged using the ADU. This allowed the various responses of the rings to be displayed on a linked PC screen as real time plots against time. Sample readings were also logged and the resultant displacement against strain ring response graphs were plotted, for each 1/2 bridge. Examples of these plots are shown in figures 8.4 and 8.5. Full print outs of the logged data and graphical plots are included in Appendix 4.

From this graphical output, calibration values could be established to convert the signals from the strain rings to displacement, giving the lateral expansion/contraction of the rings, and hence the soil sample when the rings are connected. Both the response against time and response against displacement graphs were used for these calculations. On both types of plot the vertical axis (labeled NON) is the response of each 1/2 bridge in ADU units (as described in section 7.4). Selection of the highest gain possible for each bridge gives the best resolution. The gradient of a line drawn through the end points of the hysteresis loop plots gives a direct conversion factor in appropriate units. This of course, is only valid for the purpose if the hysteresis loop is very narrow and follows a regular 'closed' path on repeated cycles. This value is representative of the loop as a whole, on both the extension and contraction

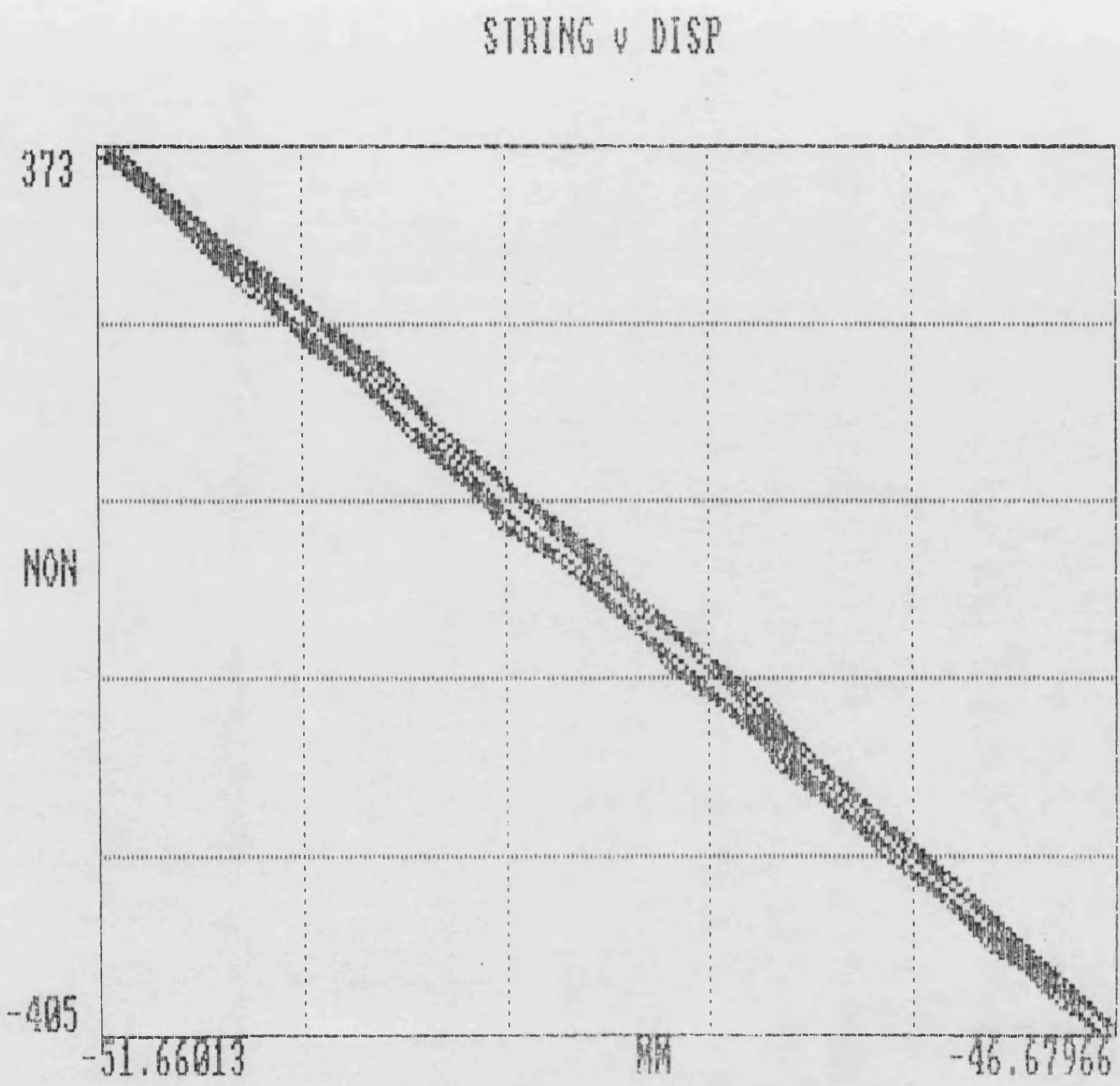
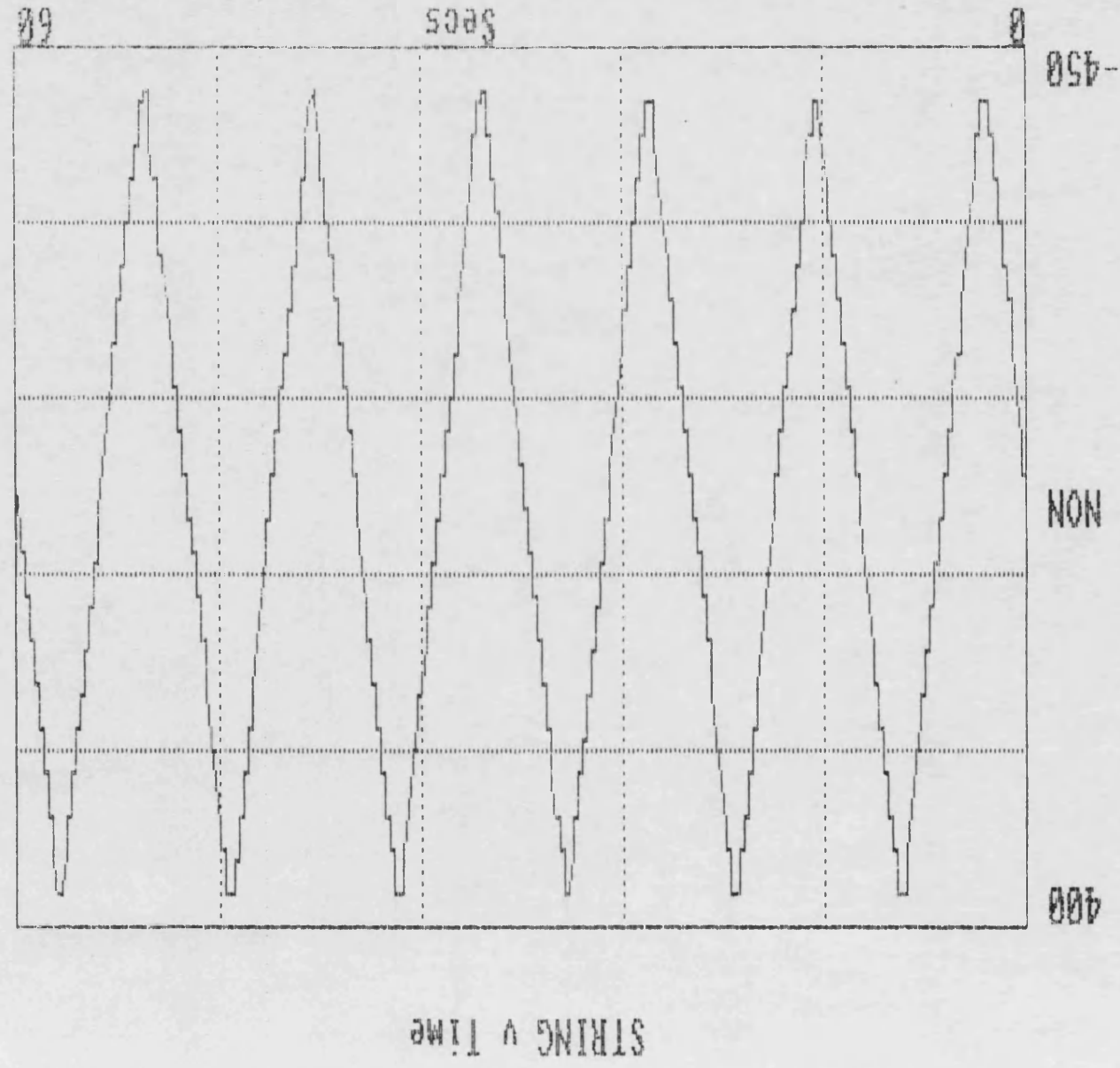


Figure 8.4. Sample plot for strain ring 1/11
Response vs Displacement plot.



phase. The wider the loop the less accurate this figure. Given that the tests were run under stroke control the amplitude of the response measured peak to peak on the time plots, must match the stroke limits of the ramp. As given previously, this was $\pm 2.5\text{mm}$. Comparison with the ADU unit scale again gives a conversion factor in the appropriate form. Care must be taken with this procedure, as the recorded points may miss the extreme points of the ramp.

When recording data versus time, the rate of data acquisition must be matched to the response of the specimen and rate of load/displacement application. In dynamic situations this is especially important. If the recording rate is too slow the true nature of the response is averaged over a wide time frame, and in extreme cases can mask any movement at all. Ideally the rate of data acquisition should be at least an order of magnitude greater than the induced motion. A further problem arises if the recording is out of sequence with the response. In this case the peaks may be missed. When this occurs the measured response gives a distorted pattern which itself is cyclic in nature.

The calibration values for each ring are tabulated in table 8.1. Each strain ring required two channels, one for each $1/2$ bridge, and the conversion factors are

identified with respect to the ring number and channel used to monitor the respective bridge.

Ring No.	Bridge Channel	Conversion mm/ADU unit	Gain
1	11	6.403×10^{-3}	10
	15	7.525×10^{-3}	10
2	13	14.23×10^{-3}	9
	14	15.09×10^{-3}	9
3	12	8.125×10^{-3}	10
	16	8.91×10^{-3}	10
4	9	16.12×10^{-3}	9
	10	31.13×10^{-3}	8
5	12	8.893×10^{-3}	10
	16	6.985×10^{-3}	10

Table 8.1. Conversion factors for strain rings

The stiffness of the rings was so low that the load cell could not register any change in resistance as the rings flexed. This indicated that the rings would have negligible influence on the expansion/contraction of the soil samples when placed in position.

In order to quantify the stiffness of the rings a series of simple static loading tests involving hanging small weights from each ring and recording the resultant deflection was carried out. The deflection range was limited to that used for the cyclic calibration tests. These tests yielded an average stiffness value for the rings of 2.0 mm/N. Full results are included in

appendix 4.

Moving back to the output produced from the calibration tests, the plots of displacement against output from the gauges with their hysteretic form, indicates that there is some damping within the system. Whilst it is not possible to quantify directly from these plots due to the absence of a load response from the cyclic testing, this will need to be considered when utilising the strain ring output to determine the damping of soil samples.

Both plots, output against time and output against displacement, indicate a high degree of uniformity for the response over a number of loading cycles. On the displacement plots the hysteresis loop does shift very slightly with increasing load cycles. This is most noticable during the early stages, and reduces after a number of cycles have occurred. It was noted that the fixing of the rings was of prime importance in achieving consistant results. The fixings used to locate the rings in the testing frame for the calibration exercise were the location studs previously described, and as used for locating the rings into the soil samples in the triaxial cell.

The 'absolute' output from the strain rings does tend to

drift with time. The type of araldite was selected to reduce the temperature effects to a minimum. Whilst the drift in 'absolute' output does not generally affect the conversion factor, re-calibration should be undertaken prior to each testing period.

8.2 MATERIAL SELECTION

As previously mentioned, this project required the formation of samples of graded granular material, utilising the additional size of the large cell. The results could then be compared to those from smaller more traditional cells in order to assess the effects of scaling upon the response of graded samples. A prerequisite to this work however, was the validation that all the different sized cells yielded similar results on similar grained single size material.

8.2.1 Material for Calibration Tests

Material for the 'calibration' exercise had to be uniform and allow suitable samples to be formed in sizes varying from 38mm to 254mm (1.5" to 10") diameter. The 38mm diameter samples thus limited the upper size range. Three gradings of sand were used for the initial batch of tests, and are detailed below.

- (1) A fine sand, grade 52/100.
- (2) A medium graded, Holm Sand.
- (3) A course graded, Leighton Buzzard calibration sand, grade 14/25.

The grading curves are shown in figure 8.1.

These materials were already available in sufficient quantities within the department. A range of gradings was selected in order to establish if grain size had any bearing on the response as sample size varied.

Inspection of the 52/100 grade, revealed the individual particles to be very uniform (rounded) in shape and colour, suggesting some artificial processing, e.g a grinding mill. The Holm sand is dredged from the Bristol Channel, and is quartz based. As can be seen from figure 8.1 this material has a wider spread of particle sizes and will thus allow a more densely packed structure with fewer voids. The Leighton Buzzard sand, usually used in the replacement method determination of bulk density, was more single sized than the Holm sand. It is a river washed material, with a more irregular particle shape than the finer sand.

Figure 8.1. Grading curves for calibration tests

PARTICLE SIZE DISTRIBUTION

Location:

Borehole No.:

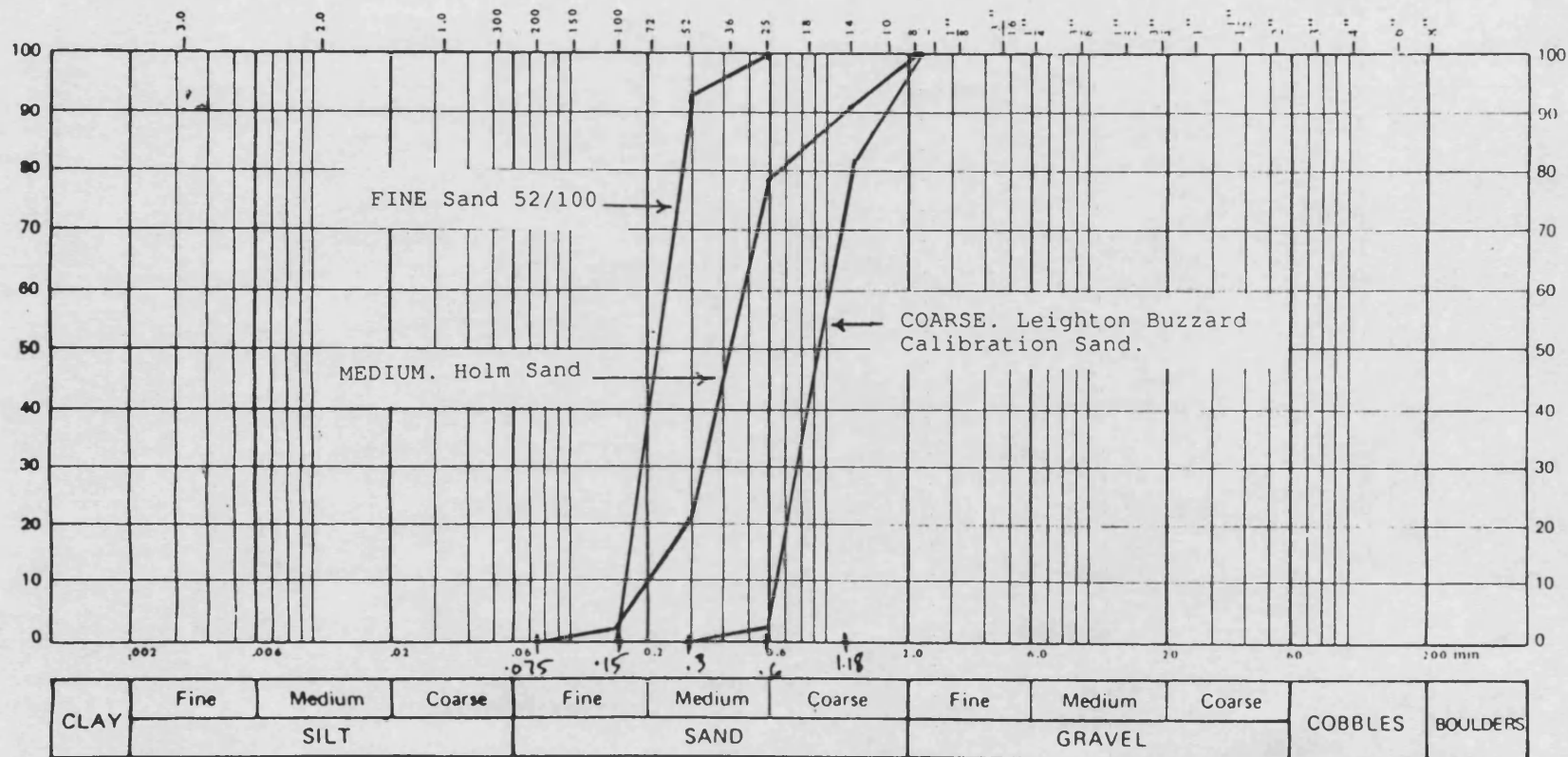
Carried out for:

Depth:

Sample Description:

Log settling velocity (cm/sec)

BRITISH STANDARD SIEVE SIZES



University of Bath,
Soil Mechanics and Concrete Laboratory,
Claverton Down, Bath BA2 7AY

8.2.2 Material for Large Scale Tests

The requirement for these tests was to model a realistic material used in soil structures. The motivation for this investigation grew out of an interest in large earthfill and rockfill dams subjected to earthquake loading, thus the material to be modelled was selected from grading curves from actual built structures.

Information was available concerning the construction of Mangla dam, built in Pakistan and completed in 1967. The grading curves for that project are shown in figure 8.2. This gave an indication of the range of sizes required to reproduce a graded sample that fell within the limits applicable to dam material. With the curve thus formed it may be shifted down the size axis to produce a well formed graded sample (i.e. no large voids, with the space between the largest particles filled with smaller grains, etc), suitable for testing within a 254mm diameter.

Continuing the initial link with embankment dams, the material for which is generally quarried/blasted local to the project, resulting in angular particles, a limestone grit was selected as a suitable material for testing. This was available from a local source and donated for the purposes of this investigation. The material was delivered ready sieved in the following

Figure 8.2. Grading curves for Mangla dam project

PARTICLE SIZE DISTRIBUTION

Location:

Carried out for:

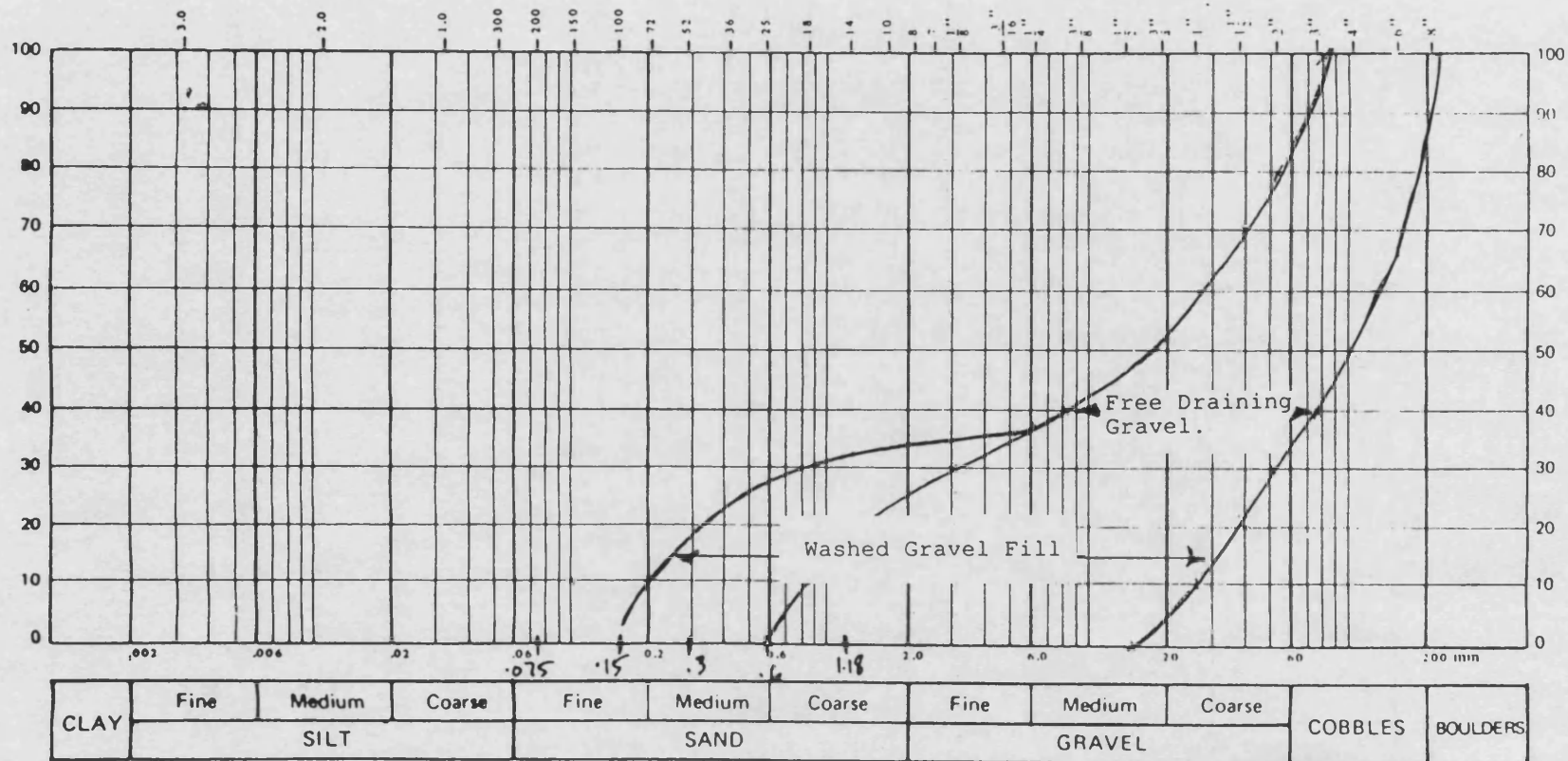
Sample Description:

Log settling velocity (cm/sec)

Borehole No.:

Depth:

BRITISH STANDARD SIEVE SIZES



University of Bath,
Soil Mechanics and Concrete Laboratory,
Claverton Down, Bath BA2 7AY

sizes, and was highly angular;

3mm to dust

10mm nominal

30mm to 20mm nominal

further sieving of the 3mm - dust element yielded the following sub divisions;

size 3mm to 2mm

size 2mm to 1mm

size 1mm to dust

This would enable a further reduction of the grading curve down the size axis to produce a graded sample for later testing in the 100mm cell, allowing investigation into the nature of producing 'scaled' graded samples.

In order to achieve a scaled down graded sample, the allowable limits of the full scale material were shifted down the size axis as shown in figure 8.3. The amount of shift was determined such that the upper size range lay within the practical particle size range that the large triaxial cell could accomodate. The size range that would be applicable is also affected by whether the sample is made from single sized or graded material. The more single sized the sample becomes, the smaller the

Figure 8.3. Grading curve shift.

PARTICLE SIZE DISTRIBUTION

Location:

Carried out for:

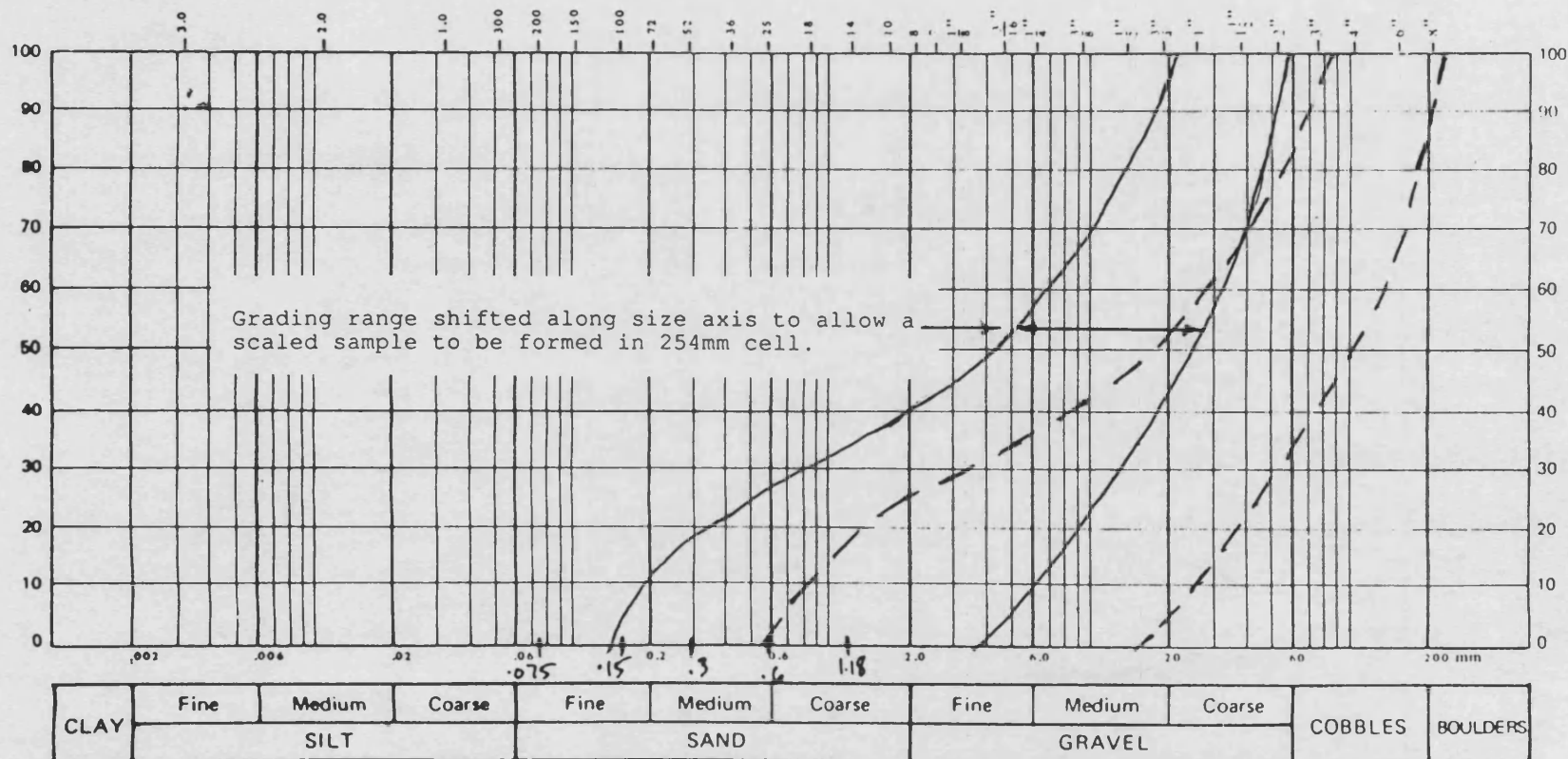
Sample Description:

Log settling velocity (cm/sec)

Borehole No.:

Depth:

BRITISH STANDARD SIEVE SIZES



University of Bath,
Soil Mechanics and Concrete Laboratory,
Claverton Down, Bath BA2 7AY

individual particle size used to form the sample can be. This is in order to maintain a truly representative failure mode within the sample limits. In order to fully establish the effects of testing graded samples, each of the constituent sizes would have to be tested as single sized samples.

On the basis of the ratio of sample sizes between the 100mm and 38mm cells, it was felt that the large cell could handle material upto 60mm diameter. This is a conservative figure based on the extrapolation of data from table 13.4 given in "Manual of Soil Laboratory Testing", Vol 2, by K.H. Head. This gives suggested maximum size of particles for various nominal sample sizes, some of which are shown below.

Nominal sample size	Max. particle size	Particle/Dia. ratio
38 x 76	3.35	0.088
100 x 200	20	0.2
150 x 300	37.5	0.25
conservative suggestion for new 305mm diameter		
305 x 335	60	0.20
max. size used in this work		
254 x 335	30	0.12

The largest material available, as previously stated, was between 20mm and 30mm which was acceptable. This upper size range determined the shift of the grading curve and enabled a graded sample to be produced from

the material available.

The graded sample was calculated on a percentage basis of material retained on selected sieves related to the limestone grit available in order to give a reasonably even distribution of the different sizes throughout the sample without cavities forming.

It was also the author's intention to consider the effect of angularity upon the response of large particulate material. A rounded gravel in suitable sizes proved difficult to locate initially, and time constraints eventually ruled this out.

8.3 CALIBRATION TESTING

These tests were conducted to establish the validity of comparative data produced from the different cell sizes. There is general appreciation that the sample size limits the size of particulate material that can be sensibly tested. Even with clays, the nature of the material, e.g. fissured or not, influences the choice of sample size, or at least the interpretation of the results. The sample has to be large enough relative to the constituent particle size to allow realistic deformation (particle rearrangement) to occur. One of the reasons for developing the large cell was to

allow comparative tests on different scaled samples between different cell sizes, and so any effect of cell size had to be accounted for.

Care must be taken in the application of the quantitative results from these tests. They were of a comparative nature to compare values between cells, not primarily to determine absolute values. This is applicable to all the tests, and allowed greater freedom in the methods of sample preparation used, simplifying procedures and thus reducing time spent preparing samples. This is important, since it has been shown that various methods of preparation of granular samples influences the mode of deformation and thus affect the resulting values of friction angle.

8.3.1 Relevant Aspects of Test Selection

In order to provide a robust and simple - with careful interpretation - comparison between each of the cells, it was decided to establish the angle of friction (ϕ) for the material described in section 8.1.1, when tested at different sample diameters.

When dealing with granular material it is important to be aware of the differences between peak and ultimate shear strength, and thus peak and ultimate angles of

friction respectively, hereafter referred to as ϕ_p and ϕ_u . These can be explained with reference to figure 8.6.

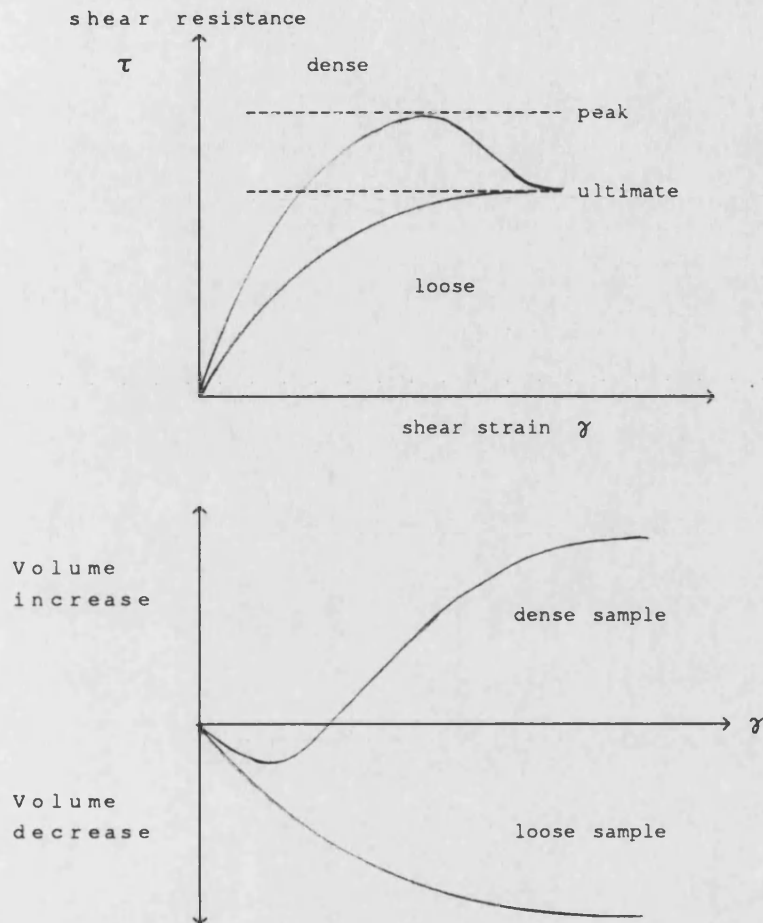


Figure 8.6. Explanation of Peak & Ultimate shear strength

For a granular material in its loosest possible state under the normal loads applied, the maximum shear resistance attained is the ultimate value. However, if the density of the material is increased and the test run at the same normal load a higher shear strength, the peak shear resistance is achieved at a lower strain level. If straining is continued then the strength

drops, eventually to the ultimate value. These effects are related to the mode of deformation of, and particle rearrangement within, the different density materials. The ultimate value is related to a 'critical' void ratio under the specified normal load. Samples with lower densities than the critical state will contract when sheared, whilst those at higher densities will dilate giving rise to the peak values. The basic aspects of this mechanism are explained at length within standard reference books, and it is not considered necessary to explain the classic dilatancy model here. Of more interest is the effect of density on the angle of friction as shown in figure 8.7, after data from Bolton.

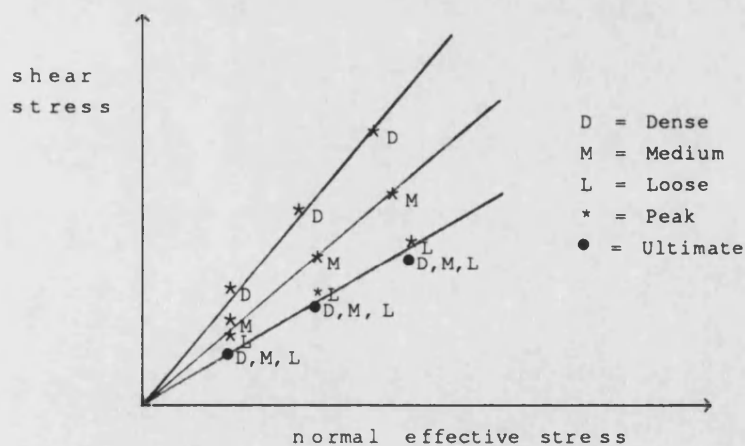


Figure 8.7. Change of ϕ with density. (After Bolton)

Bolton demonstrates that ϕ_u is unaffected by density, remaining constant. However, ϕ_p varies considerably with increasing density. Values suggested for clean dry sands

are $\phi_u = 35^\circ$, and the range for $\phi_p = 35^\circ$ to 50° .

Hettler and Vardoulakis (1984) presenting the results of tests on a medium sand in their large oversquare triaxial rig gave the relationship for the change in ϕ_p with density, shown in figure 8.8. Note, this density refers to the initial sample density at the start of the test.

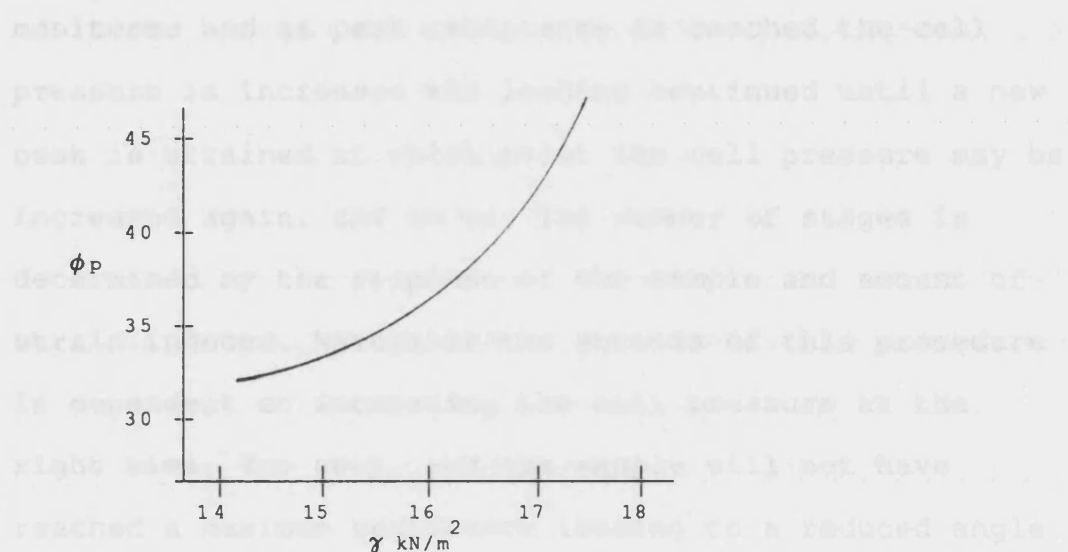


Figure 8.8. Density dependence on the friction angle
(after Hettler and Vardoulakis 1984)

From this discussion it can be appreciated that interpretation of the angle of friction must consider the relative density of the sample.

8.3.2. Mode of Testing

To get representative values for the friction angles, it was necessary to test samples at a range of cell pressures and initial density. Due to the difficulties

of forming samples of granular material, it was felt sensible to maximise the data from each test. Multi-stage tests were therefore utilised, yielding information for a range of cell pressures from each sample. The principle of multi-stage testing is as follows. The sample is set up at a cell pressure and the loading applied. The response of the sample is carefully monitored and as peak resistance is reached the cell pressure is increased and loading continued until a new peak is attained at which point the cell pressure may be increased again, and so on. The number of stages is determined by the response of the sample and amount of strain induced. Naturally the success of this procedure is dependent on increasing the cell pressure at the right time. Too soon, and the sample will not have reached a maximum resistance leading to a reduced angle of friction. Too late, and the sample may have 'failed' thus affecting the results for the following stages. Often the final stage offers the most accurate results since the loading can be continued through the peak, although the deformation must be kept within sensible strain limits otherwise end effects may influence the response. The resulting Mohr circle plots give a good indication of the consistency of the readings.

In order to successfully establish the appropriate point to increase the cell pressure, knowledge of the 'real

time' stress resistance of the sample is required whilst the test is underway and the sample is deforming. It is when this response reaches a peak that the cell pressure can be increased. This was achieved by plotting the load, as measured on a proving ring or indicating load cell, on area correction paper. This is a standard technique used in strain controlled tests, whereby, assuming constant volume and uniform deformation, and knowing the rate of axial compression, the increase in cross-sectional area of the sample can be calculated for any point during the test. Utilising this information, an area correction graph can be constructed, whereby the vertical load axis reduces as the time axis increases. This takes account of the reduction in stress due to the increase in cross-sectional area as the sample compresses.

The validity of assuming constant volume needs to be clarified, as this procedure is more usually applicable to tests on saturated undrained tests. As previously mentioned all the tests were carried out on dry samples. Further, the sample preparation procedure required an internal vacuum to be applied to the sample. This internal vacuum was released prior to the commencement of shearing, and the valve left open whilst shearing occurred, thus allowing the possibility of volume change. However, the volume change was monitored and

found to have negligible effect on the results, and so the use of the area correction method as stated was justified.

A further point to bear in mind with multi-stage tests is the difficulty associated with changes of density at the different cell pressures. Granular material has a very low range of void ratio associated with changes from low to high relative densities, so a small change in volume can have a considerable influence on the density. But once again, it is the comparative nature of these test that is important, and the interpretation of the data will take these factors into account.

8.3.3 Method of Static Testing

The equipment used for these tests has been described at length in Part II. The overall test configurations will be described here.

The 38mm and 100mm cell configurations followed familiar lines, with the cells being placed within loading frames, controlling the rate of strain applied and the load being measured with proving rings of appropriate stiffness. Constant cell pressure was supplied from the air/water system previously described. The cell pressure was routed through a double manometer in order to

measure the amount of water movement to the cell, allowing an assessment of the change of volume of the sample. An example of the 100mm cell configuration is illustrated in plate 8.2.

The large cell was placed on the strong floor beneath a load resisting frame. A 'loose' 50 kN actuator acting against the frame provided the loading, but again under stroke control. The actuator incorporated both load and stroke monitoring facilities via the integral load cell and LVDT. The control signal for these tests was the stroke channel, so the load response was displayed on the internal digital read out. As the load was being monitored external to the cell an assessment of the frictional characteristics of the ram/bush interface was required. This is detailed in section 8.3.3.1 below. The maximum load available of 50 kN limited the range of cell pressures to a maximum of 3 bar. This arrangement is shown in plate 8.1.

The rate of strain application for these tests was selected to permit adequate time to record the readings using the area correction procedure. The British Standard recommends static testing to be carried out at strain rates between 1% and 2% per min. A strain rate of 1% per min. was thus used.

The sample preparation procedure has previously been explained in section 6.4, only items of detail will be discussed here. The 38mm and 100mm cells both used modified formers normally used to form clay samples. This resulted in a slight reduction in the 'as built' sample height. This was brought about by the need to utilise some of the external former as a bearing lip for the base pedestal and top platten. The final diameter to height ratios were 1:1.76 and 1:1.67 for the 38mm and 100mm samples respectively. These ratios still fall within acceptable limits for use with standard end platens. As previously explained the large cell utilised friction free end platens allowing a sample ratio of 1:1.33.

A variety of preparation techniques have been developed and reported in the past. These vary from simple single pour techniques with constant pour heights, to methods involving preparation of the sample in two halves and inverting them prior to placement in order to overcome any gravity effects. The aim of the tests reported here required the consistent formation of samples at a range of densities in the three cell sizes. As long as the samples were formed in a consistent manner in each cell the comparative nature of the testing would be valid, which was the primary aim of the project. This allowed simpler preparation techniques to be used. The samples

themselves were formed in layers, with each layer being tamped to achieve the required density. All tests in this series were run using dense samples, with relative densities in the range 70 - 80%.

8.3.3.1 Determination of Friction on ram/bush interface

The ram/bush interface utilised a PTFE bearing surface ensuring a running fit. In order to prevent water leakage under pressure, a U seal was also incorporated. In order to get an assessment of the friction induced as a consequence of this arrangement a series of simple dry tests (no water pressure applied to one side of the U seal) were performed. Only dry tests could be performed due to the lack of an appropriate water proof load cell. The test configuration is shown in figure 8.9

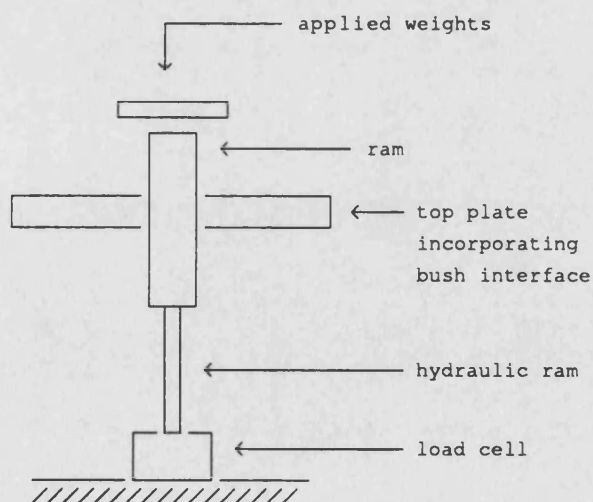


Figure 8.9. Layout for ram friction determination

Self weight of jack = 6 Kg	}	total 16 Kg
Self weight of ram = 10 Kg		

Applied weights [Kgf]	Recorded load for upward movement [Kgf]
0	32
10	42
20	52
30	62

The consistent difference between the total applied loads and that recorded, of 0.157 KN (16 Kgf) indicates the friction when pushing against the seal.

Applied weights [Kgf]	Recorded load for downward movement [Kgf]
0	10
10	20
20	30
30	40

The consistent difference of 0.059 KN (6 Kgf) between total and recorded figures indicates the friction when pushing with the seal.

These figures are illustrative values. With an internal

water pressure applied, the seal may increase the frictional forces. But for the static testing, values of this magnitude are negligible in comparison with the loads applied to the samples.

8.3.4 Summary of Results of 1st stage Calibration Tests

Three tests in each cell size were carried out for this preliminary stage. A sample plot of the multi stage readings is shown in figure 8.10a and the resulting Mohr circle plot in figure 8.10b. All test data is included in appendix 5, and summarised in table 8.2 and the Mohr circle plots illustrated in figures 8.11a,b,c.

	Friction Angle			
Sample size	Material Type			Cell pressure range
	Fine	Medium	Course	
38mm	31-33 [*]	33-35 [*]	33	1 - 5 bar
100mm	36 [*] - 38	38 - 41	38 - 39	1 - 5 bar
254mm	38	40 - 42	39 - 41	1 - 3 bar

Table 8.2 . Summary of preliminary Calibration tests.

* indicates that value associated with final pressure

Material Type: Fine = 52/100 grade, Medium = Holm sand
Coarse = 14/25 grade

PROVING ZONE D.W.

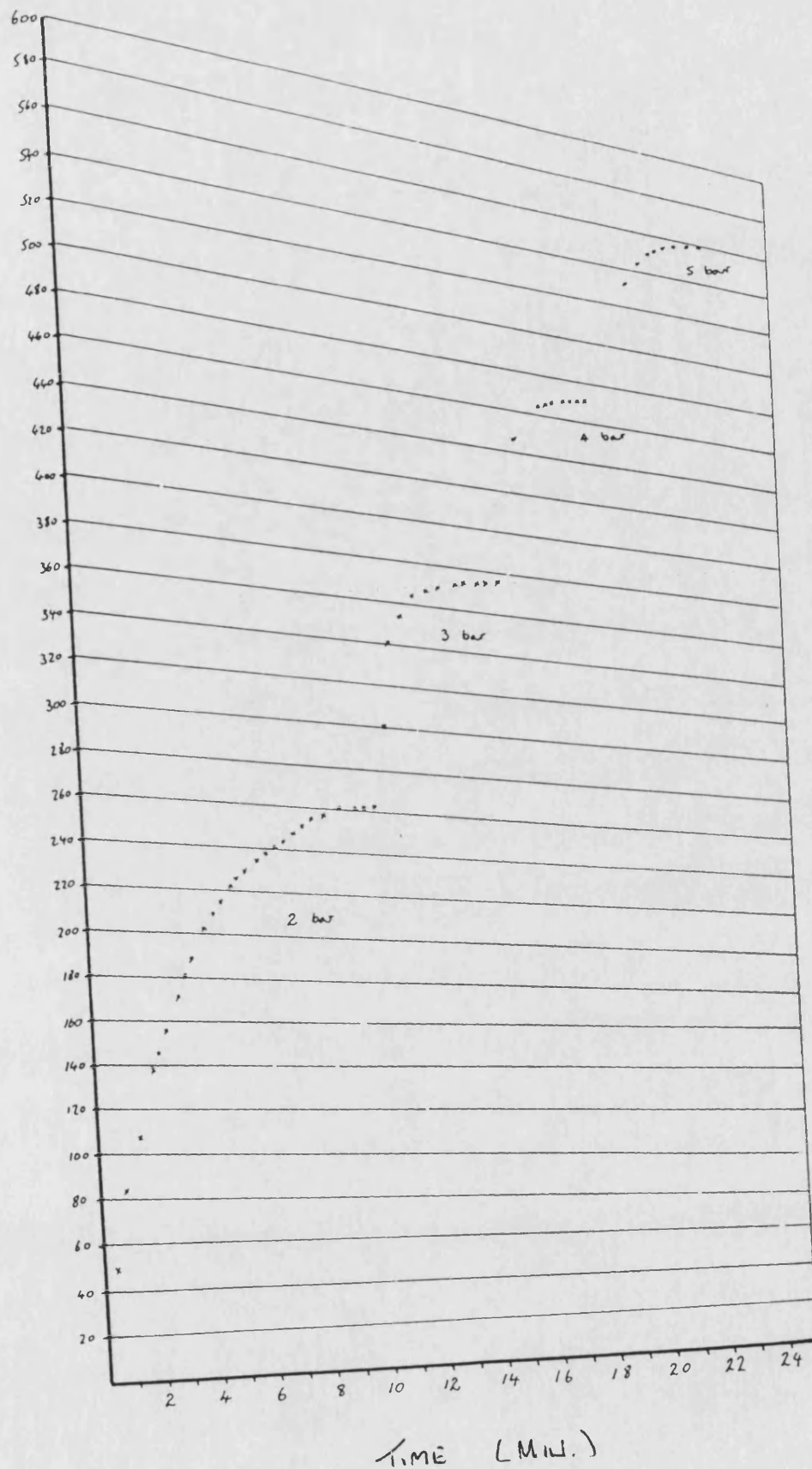
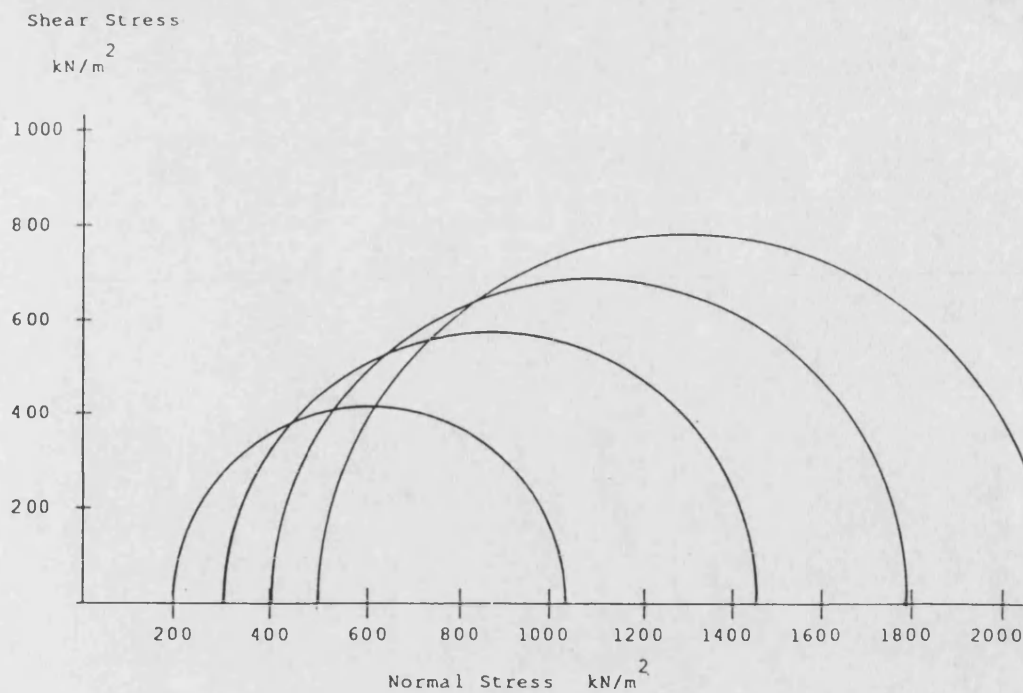
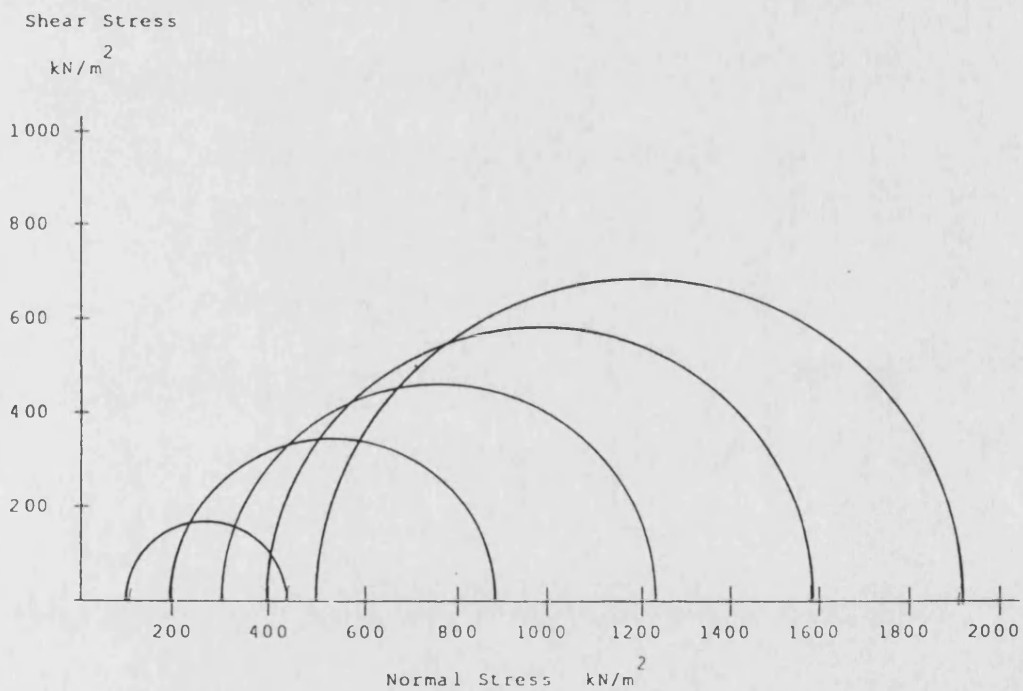


Figure 8.10a. Sample multi-stage plot on area correction paper.



Mohr's circle plot for Test No. 5



Mohr's circle plot for Test No. 6

Figure 8.10b. Mohr circle plot for multi-stage test shown in figure 8.10a.

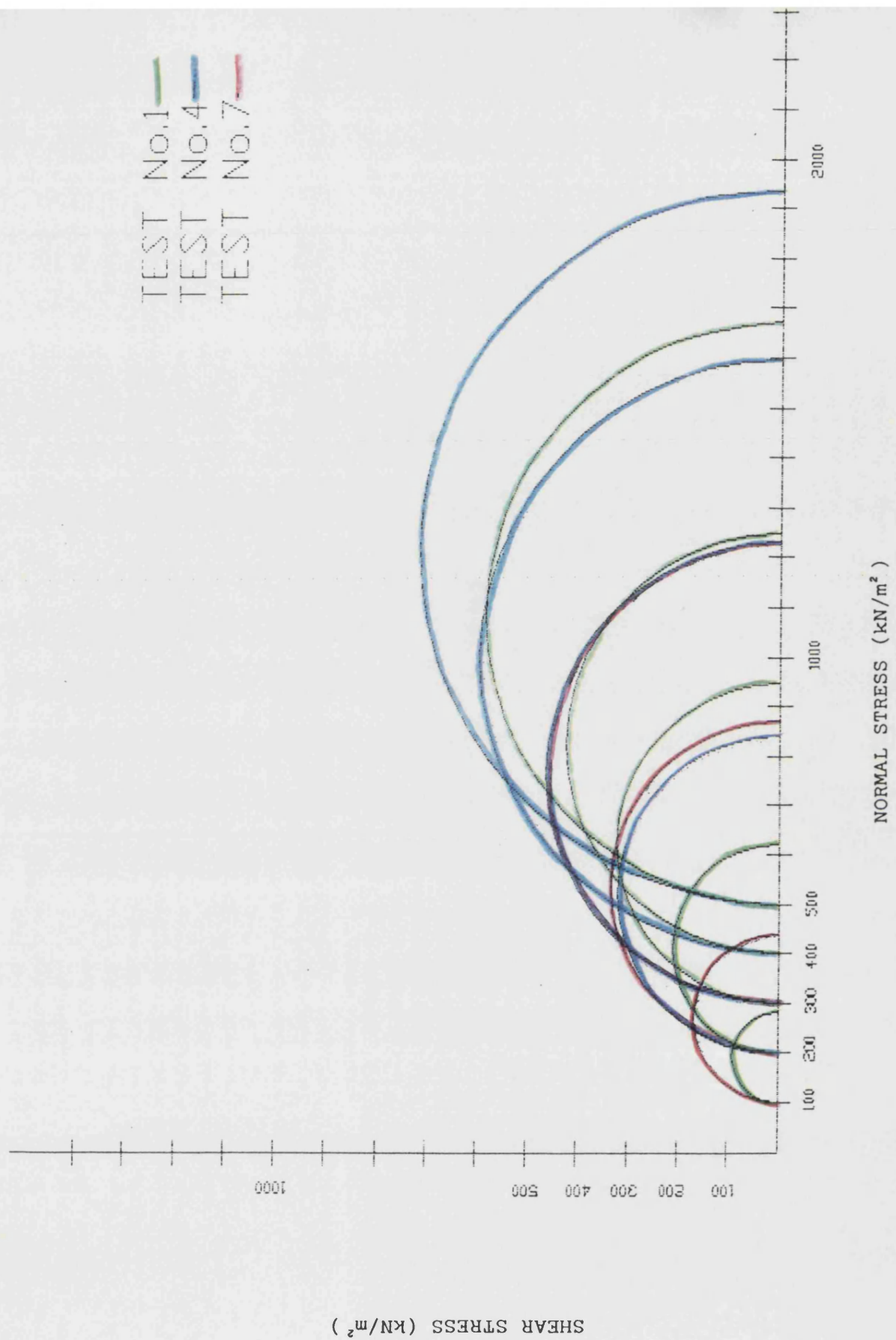


Figure 8.11a Combined Mohr circle plots for 'fine' grade samples.

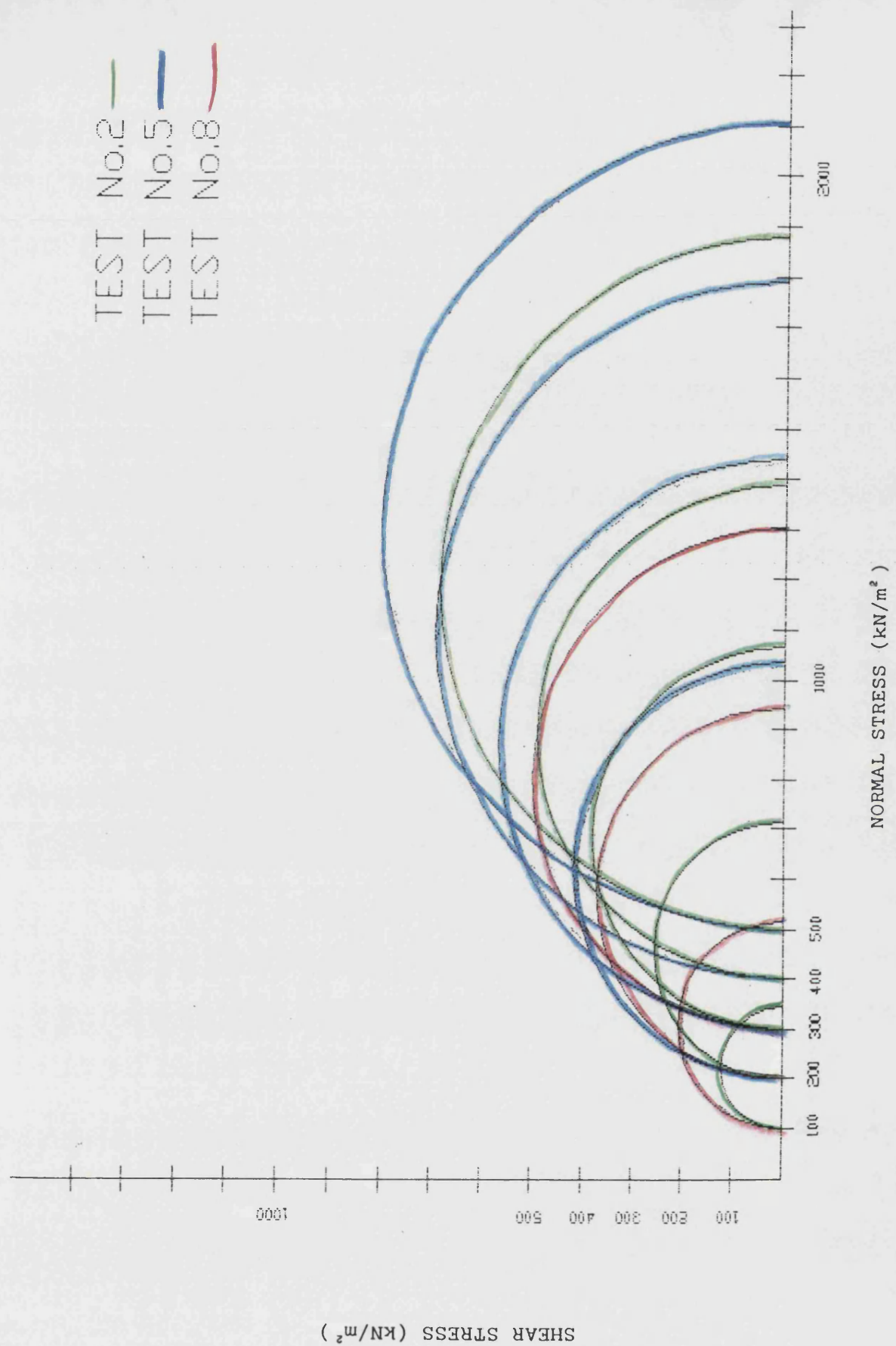


Figure 8.11b Combined Mohr circle plots for 'medium' grade samples.

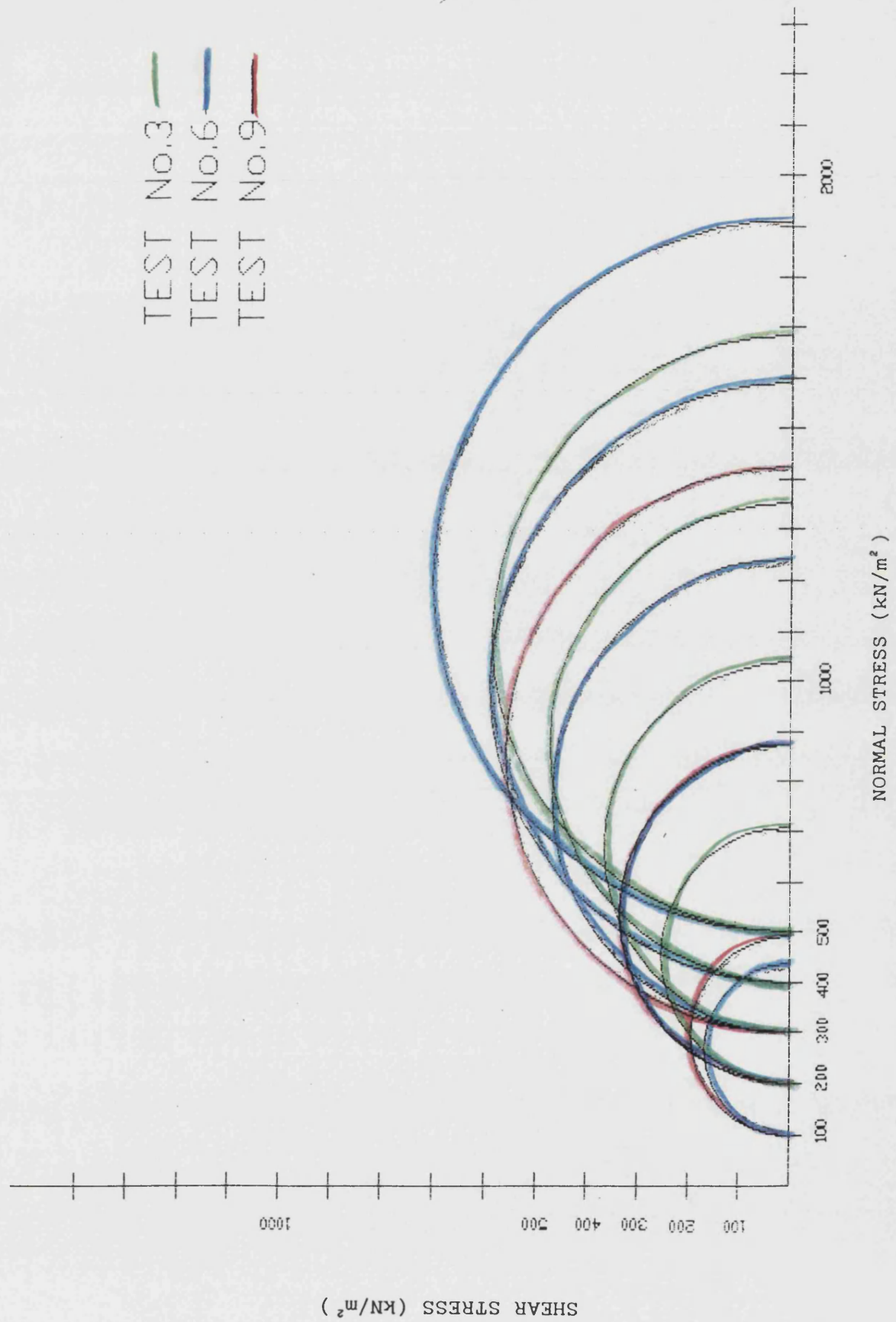


Figure 8.11c Combined Mohr circle plots for 'coarse' grade samples.

8.3.5 Discussion of Results (1st stage)

Considering first the different types of material. The medium Holm sand gives consistently higher values. The spread in values between the materials do tend to overlap, but the trend is clearly visible. This is not unexpected, since the grading curve for the Holm sand indicated a greater mix of sizes, therefore giving a sample with fewer voids and greater interparticle contact. Within each sample size the finer sand gave the lowest values. Again, not unexpected, as it is common practice to test only the fines content of mixed granular fill in order to determine a conservative value for design purposes. The course material shows a spread broadly similar to that for the medium sand.

Moving to comparisons between sample sizes. The most evident feature is the clear disparity between the 38mm results and those from the larger cells which are in close agreement. The smaller samples give consistently lower values. This is intriguing, since one would be tempted to suggest that the relative size of particle to sample size was the cause, although the upper particle size is well within the size limits quoted in section 8.2.2. An increasing number of papers are being published concerning the effects of the containment of soil samples within laboratory equipment, and the

resulting effects on the determination of material parameters, specifically related to cone penetrometer and pressuremeter tests, Schnaid, F. & Houlsby, G.T. (1991). Numerically much of this work utilises cavity expansion theory in explanation. From this and previous literature, the proximity of boundary effects would be expected to increase the apparent strength of the sample, not decrease it. As explained earlier, small changes in density can have a major impact on material strength. A simpler explanation may well be that the smaller sample size inhibits the formation of dense samples of a uniform nature, thus allowing weaker areas to be present. This would link with those papers referred to above, in which there is agreement that dense samples are most affected by the proximity of a boundary surface. If this is consistently true, then the same effect should be visible when larger material is tested in the 100mm cell compared with the 254mm samples. Further tests at lower relative density would allow further comparison.

The two larger cell sizes are in broad agreement, with almost complete matching of some of the Mohr circle plots. Of further interest, the 254mm samples gave consistent results within each multistage test, resulting in less ambiguity in the value of friction angle for each test. Given the more complicated

arrangement for the larger cell, the greater work involved in preparing samples, etc., it was feared this could lead to less consistent results. In the event, it did not. A further point to consider, and briefly touched on earlier, was the efficiency of the friction free ends in the 254mm samples. These were found to be not fully effective in that following dismantling of the cell, barrelling of some of the samples was apparent which indicates friction at the end platens. If this barrelling was significant, then with the low height:diameter ratio used within the large cell, higher apparent strengths would result. The very close agreement for the friction angles determined between the 254mm and 100mm samples suggests that there is little evidence of end effects affecting the results.

The results were encouraging and useful experience was gathered, but it was felt that further tests should be performed to clarify the aspects raised.

8.3.6 Calibration testing (2nd stage)

In order to take on board some of the issues raised from the 1st stage tests, and also to continue the comparison of the effect of particle size between different sample sizes the material selection for the 2nd stage tests was altered slightly. Two gradings were selected from those

detailed in section 8.1.

- (1) the coarse Leighton Buzzard calibration sand -
section 8.1.1
- (2) the 1 to 2mm material sieved from the limestone
grit - section 8.1.2

The testing methods and apparatus were identical to that described for the 1st stage tests, with the inclusion of some single stage testing to allow comparison. The tests were concentrated between the 38mm and 100mm cells, the 100mm already having shown good correlation with the larger cell. This decision, whilst not wholly satisfactory, greatly increased the number of tests that could be carried out, as the complexity and time required to set up the 100mm cell was much reduced in comparison with the 254mm cell.

8.3.7 Results (2nd stage)

Full result sheets are included in appendix 5, and are summarised in table 8.3. The tests have been organised by cell size, density, material type and where interesting comparisons occur between multi and single stage tests these have been separated.

Cell Size	Material	Density	Friction Angle	Comments
38 †	Cal.Sand	high	35 - 37	Both multi & single stage tests
		med.	35 - 36	
		low	35	
100	Cal.Sand	high	34 - 36	multi stage tests
		med.	34 - 35	
		low	33 - 34	
100	Cal.Sand	high	38	single stage tests
		med.	37	
		low	35	
254	Cal.Sand	med.	35	single
100	Type (2) 1 - 2mm grading	high	42 [*] - 44	multi stage tests
		med.	40 [*] - 43	
		low	40 [*] - 43	
38 †	Type (2) 1 - 2mm grading	high	40 [*] - 44	multi stage tests
		med.	38 [*] - 44	
		low	39 - 44	
38 †	Type (2) 1 - 2mm grading	high	42	single stage tests
		med.	43	

† some of these tests were run with the drainage line closed, although the vacuum had been removed.

* indicates the final stage value(s) if clearly identifiable.

Table 8.3. Summary of 2nd stage calibration tests

8.3.8 Discussion of Results (2nd stage)

Dealing first with the calibration sand material. These tests showed good consistency between both sample sizes, with the friction angle increasing with density. There was very close agreement in the values for the friction angle determined. There was some disparity between the single stage and multi stage tests on the 100mm samples. The same general trend of increasing friction angle with increasing density was still apparent. But the actual values determined from the single stage tests were consistently larger than those resulting from the multi stage tests. There is a tendency when running multi stage tests to increase the cell pressure at the first sign of a leveling off of the load response curve, which may result in the cell pressure being increased too early. This would be a possible explanation of the data, leading to the conclusion that the single stage values are more reliable, although, as stated, there was good agreement between the multi stage tests from both 100mm and 38mm samples. Fewer single stage tests were carried out on the 38mm sample which limits comparisons, but those that were done fell within the range of the multi stage tests. If the single stage results are considered to be more reliable then again the smaller cell is seen to give lower values than the 100mm.

The results from the tests on the chippings are less clear. All the tests indicate that the material yields a higher friction angle than the finer calibration sand. There is a wide scatter with the range of values within the various density bands overlapping considerably. Looking at the maximum values within the various density bands, a general rise in the value for the friction angle with increasing density is evident. The values from the single stage tests from the 38mm samples fall below or within the range of values for the 100mm cell, again indicating lower values coming from the smaller samples.

A more detailed parametric study into these effects seems to be indicated. It was decided to progress onto the testing using the large cell to continue the investigation to the larger grained material, and also to further elaborate on the operational characteristics of the larger cell.

8.4 STATIC TESTING OF LARGE GRAINED MATERIAL

These tests were run for a number of reasons. Firstly to continue the pattern of the earlier tests to see whether any clearer relationships emerged. Secondly to determine the operational characteristics of the large cell when dealing with large grained material, also the supporting control and monitoring equipment. Lastly to provide material data for the larger particulate material for later use in the graded and cyclic experiments.

The materials utilised from those detailed in section 8.1.2 were the two largest gradings available;

- (1) 10mm nominal grading
- (2) 20mm to 30mm nominal grading.

As previously noted, these had very angular particles.

As the loads for this material were expected to exceed those for the finer grades already tested, and also to allow a greater range of cell pressures to be examined, the 50 kN digitally controlled actuator was replaced with a 100 kN manual hydraulic jack. The load was monitored by two independent load cells, one giving a direct digital readout. Displacement measurement utilised a strain gauge displacement transducer bearing on the cell ram top plate. All the electrical transducers were logged by the ADU.

The controlling software for the ADU allowed real time screen plots of the logged channels against time. This facility was used to provide the load response curve for the sample as the test was running, allowing decisions to be made regarding the progress of the test. These plots were later screen dumped to the printer and formed part of the data output for each test. Load, stroke, and time channels were all logged, allowing XY plots of load versus stroke to be produced once each test was completed. Additionally, a full print out of all logged data was provided in order to calculate the stress levels required to determine friction angles.

8.4.1 Mode of Testing

The use of a manual hydraulic jack meant that a constant rate of strain could not be maintained. The tests were run by use of the real time load response curve of the sample. This gave sufficient information with which to judge the continued load application.

The general arrangement of the apparatus and the sample preparation procedure have been detailed previously.

Considerable problems were encountered during these tests with the membranes. The highly angular nature of the materials resulted in many instances of the

membranes tearing, especially at the higher confining stresses, resulting in a loss of cell pressure and premature failure of the sample. The use of thicker membranes was an obvious solution, however considerable difficulty was encountered in stretching the 254mm diameter membranes over the 304mm outer former, top and bottom platens of the cell. The use of additional membrane protection, allowing the thinner membranes to be used, was therefore investigated.

Initially a layer of fabric was placed against the inner, sample face, of the membrane, so that any indentation by the granular particles would be reduced by this layer of fabric. The fabric was split into two vertical overlapping elements in order to allow relative movement between granular particles, fabric layer, and outer latex membrane, thus reducing any additional restraining action this 'protective' layer may impose upon the sample response. Whilst the principle was successful, this application was not. It was found that as the sample deformed, the particles would displace the fabric from the sample ends at the top and bottom platens, and again pierce the membrane. To provide secure protection around the top and bottom platens a couple of membranes were cut into shorter lengths. These shorter lengths were placed on the inside face of the sample former, one at either end sandwiching the inner

layer of fabric. This arrangement effectively gave a double thickness membrane around the top and bottom plattens. Whilst this arrangement protected the membrane, it was found impossible to form a sample with perpendicular sides adjacent to the plattens, due to the inability to pull back the loose inner section of membrane evenly. The final solution to this problem was to use two additional split rings of the fabric material, bent and cut to allow relative movement, placed around the top and bottom of the sample former, thus overlapping the main protective elements, preventing any weak spots appearing. This membrane protection is illustrated in plate 8.3.

8.4.2 Results

All the test data is held in appendix 5, and summarised in table 8.4. Only those tests that ran to completion are included. Tests with membrane failures prior to sample 'failure' have been discarded.

Material	Confining Pressure	Friction Angle	Density kN/m ³ *	Test No.
10mm	2 Bar	42	14.26	6
	2.5 Bar	38	14	2
	3 Bar	38 - 41	13.85	7
	3.8 Bar	34	14.15	3
	4 Bar #	34	14.21	4
	4 Bar #	35	13.63	5
	4 Bar ‡	39	15.33	8
	1 Bar ‡	44 - 46	15.10	10
	1 Bar ‡	49	15.08	11
	1 bar †	45.5	15.2	12
20 - 30mm	1 Bar ‡	45 - 51	12.37	13
	2 Bar ‡	40 - 45	12.71	14

‡ Samples tested with membrane protection

† Cell pressure increased during test. Friction value
Calculated on basis of extending original data line.
Pressure jump clearly indicated on graphical output.

* Based on Nominal sample volume. See discussion.

Membrane failure, but 'peak' resistance apparently
attained.

Table 8.4. Summary of Large scale testing

8.4.3 Discussion

The first point to note is the low densities achieved during these tests. The samples were formed by the single pour technique from a constant height to achieve a uniform density. The higher densities for the latter tests on the 10mm gravel, coinciding with the use of membrane protection arose for the need for greater care in the placement of the material to avoid disturbance of the membrane protection. The density calculations utilised the nominal sample volume, but as previously mentioned, membrane indentation with large grained material does reduce the actual volume of the sample. The values given are therefore useful for comparison purposes only.

Interpretation of the data must take account of the inconsistent operation of the friction free ends. A noticeable stiffening of the sample, represented as a discontinuous 'jump' on the load response plot, was apparent as the axial strain exceeded $\approx 10\%$ if the friction free ends did not operate smoothly. This clearly indicates that for the large grained material the combination of a low height:diameter ratio and friction at the end platens influences the sample response. In order to extract some data from those tests affected, the form of the load response curve prior to

the stiffening was extended to see if sensible values resulted. Additionally, values for the friction angle were determined at 10% strain. This allowed comparison between tests that showed 'peaks' at higher strains, which would have been more affected by end effects. Care must be exercised during this type of data manipulation and in the use of the resulting information. There is a tendency to be selective in the use of data giving rise to false conclusions. With the limited number of tests conducted, overall trends, if apparent, are more robust.

The influence of the frictional constraints at the end plattens is more noticable at the lower confining pressures, accounting for the higher friction angles. As the confining stress increases the friction angles become more consistent.

Comparing the results between samples with and without the membrane protection, those with the protection indicate higher values for the friction angle. However, the greater care needed in placing the material when utilising the protection gave rise to higher densities. These higher densities would increase the friction value. It is the authors opinion that the membrane protection does not influence the result to any great extent. Further testing is required to fully justify this assertion, but is hindered by the sticking

'friction free' end platens. Failure of the membranes without protection hinders comparative testing at the higher cell pressures. Conducting future tests with the protection at a variety of densities would prove satisfactory and allow comparisons.

8.5 CYCLIC TESTS

These tests were necessary to validate the testing configuration for the large cell. These tests required the full set of data monitoring devices, including strain rings, LVDT's, and outputs from the control cabinets. The sample preparation procedure incorporated the location studs and specially prepared membranes. All these items have been described in sections 6 and 7.

As discussed in sections 6.1.1 and 6.2 it is advisable to measure the load within the cell chamber so that effects of friction on the ram/bush interface are removed. This is especially the case for low stress/strain applications. Ideally, for dynamic tests the load cell should be placed in a static position relative to the sample and loading ram so that inertial effects are minimised. Beneath the base of the sample or lower pedestal would be a suitable position. The present configuration of the large cell does not incorporate an internal load cell due to cost constraints. This limits

the application of the apparatus for dynamic testing to higher stress/strain ranges such that frictional effects become minimal. Kokusho (1980) demonstrated that even if the frictional effects were accounted for when using an external load cell, the results for shear strains below 10^{-4} still contained an appreciable error.

The inconsistent operation of the friction free end plattens was a factor also requiring attention. A further modification was the addition of a 1.27mm. thick layer of latex rubber - identical to that of the membranes - placed at each end of the sample. This system has to date proved unsuccessful. Limited time due to the extended program of calibration tests has constrained efforts to provide an adequate solution to this problem.

A number of cyclic loading tests have been carried out with the apparatus in its present configuration. These are useful for verification purposes, both of procedures and equipment, and are reported here for the information they provide for the development of the apparatus.

The arrangement of the sensors around the sample has been discussed previously in section 6.0 and illustrated in plates 6.2 and 6.3. The overall configuration is illustrated in figure 8.12.

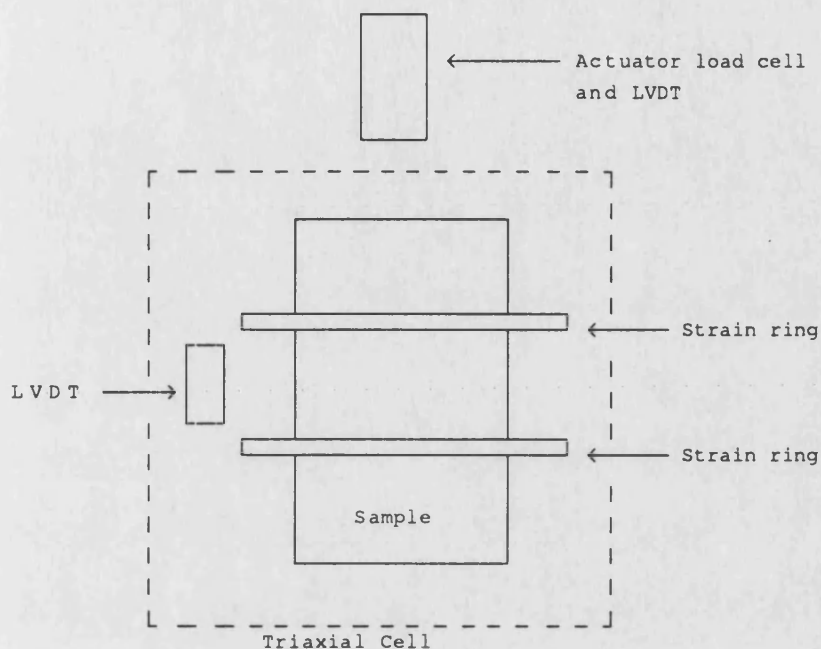


Figure 8.12. General configuration for cyclic tests

Initially it was found impossible to hold the cell pressure. This was due to leakage of pressurised water into the sample at the junction where the location studs pierced the membrane. It proved difficult to compress the 'O' ring seal a sufficient amount without tearing the membrane. Two possible solutions to this problem were considered.

(1) Use of a double membrane. This would still require holes in the second, outer membrane. Difficulties would arise preparing the sample as only one membrane can be pulled via the vacuum.

(2) Use of a flexible sealant around the junction of the location stud and membrane. Such a sealant would need to be inert relative to the latex membrane.

Option (2) was selected as the most appropriate solution. Utilising a one part silicone sealant ARBOSIL 1081, manufactured by Ratcliffe & Co. Ltd, proved successful. The sealant was placed around the location studs on the external face of the membrane, and is shown in operation in plate 8.4.

The material utilised for these tests was the nominal 10mm grading. Membrane protection was used to allow testing at high cell pressures. Great care needs to be taken during the assembly phase in order not to disturb the sensor arrangement. The remote instrumentation provides the only information regarding the sample whilst the test is in progress since it is not possible to visually examine the sample until the apparatus is dismantled.

Full copies of the relevant test data are included in Appendix 5 for reference. Sample plots are shown in figures 8.13 to 8.15 and discussed below.

LOAD v STROKE

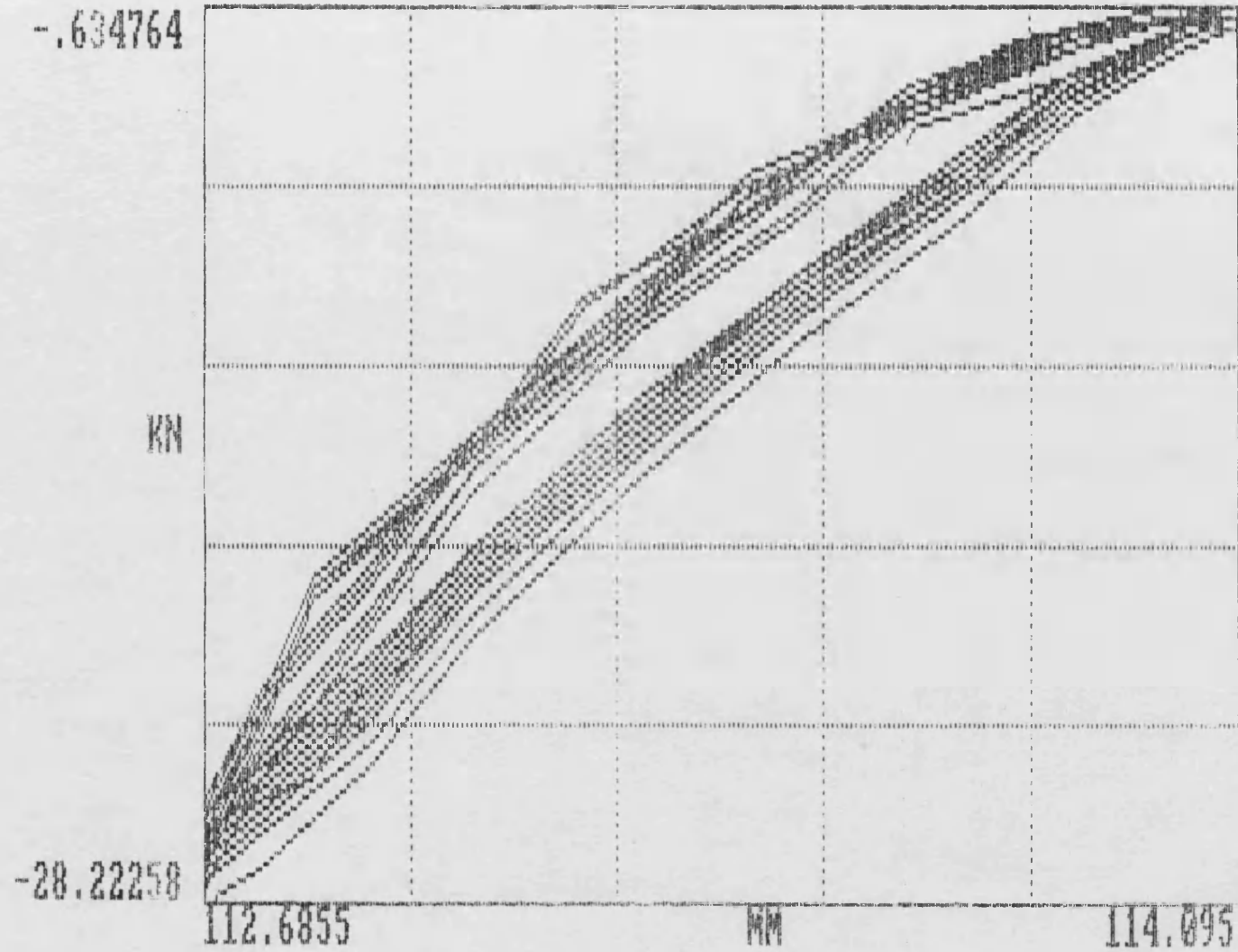


Figure 8.13. Sample load vs stroke plot for dynamic verification test 6.

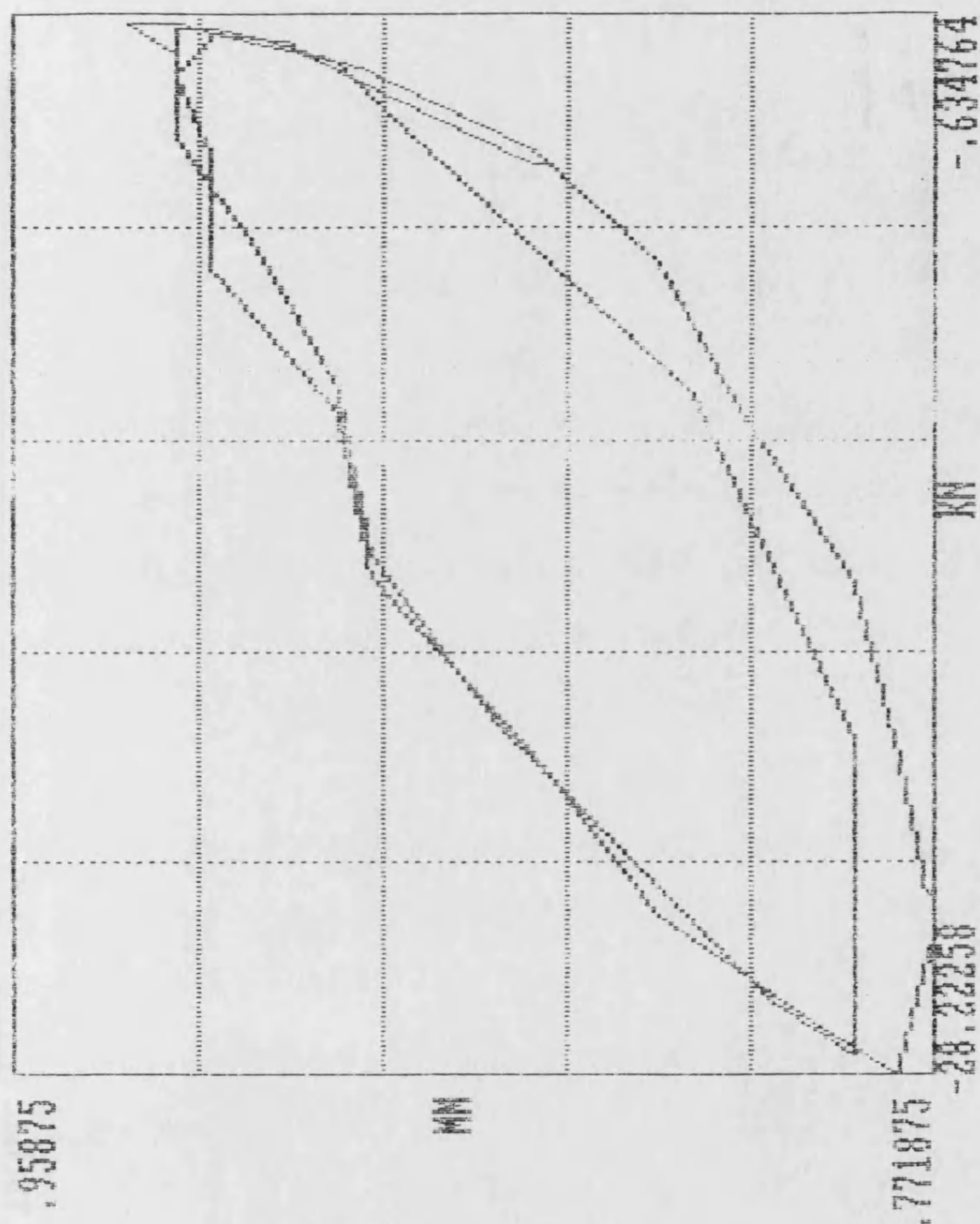


Figure 8.14. Sample plot of strain ring response vs load from dynamic verification test 6. First 2 cycles shown.



Figure 8.15. sample plot of strain ring response vs load from dynamic verification test 6. First 2 cycles shown.

The hysteretic nature of the stress - strain response is clearly evident. The response as monitored by the external sensors is shown in figure 8.13, and is matched by the output from the strain rings as indicated in figures 8.14 and 8.15.

The output from the strain rings was variable. This is attributed to a number of factors. The strain rings were originally tested placed vertically between location studs held firmly in hydraulic jaws. Within the triaxial cell they are placed horizontally relying on the embedment of the location studs for a secure fixing. The strain rings are susceptible to very slight movements and require a firm support that reflects the sample movement. The depth of embedment of the location studs required for a particular test, is dependent on the nature of the material under test, angular or rounded, and also on the particle size distribution, single sized or graded. In order to ensure a rigid support it is necessary to provide longer location studs for the larger particulate material so that the stud is gripped firmly in place. It is recommended by the author that this anchorage length needs to be at least $1.5 \times$ minimum particle size diameter. This places the current studs on the border for the 10mm material, and would explain why some became displaced.

It is important that the design of the location studs and fixings for the strain rings reflects the deformation of the sample accurately, especially at the very low strains (10^{-6}) required for the determination of G_{max} . Within the current setup the strain rings have been shown to be capable of detecting radial strains within the range 10^{-4} to 10^{-3} . From the experience gathered to date, it is the author's opinion that this is approaching the limit of accuracy for the combined components of this system when used for monitoring cyclic tests. To assess the response at lower strains non - contacting transducers are favoured. Such a system would not require mechanical fixings subjected to load/displacement reversals giving greater confidence in the repeatability and accuracy of the output.

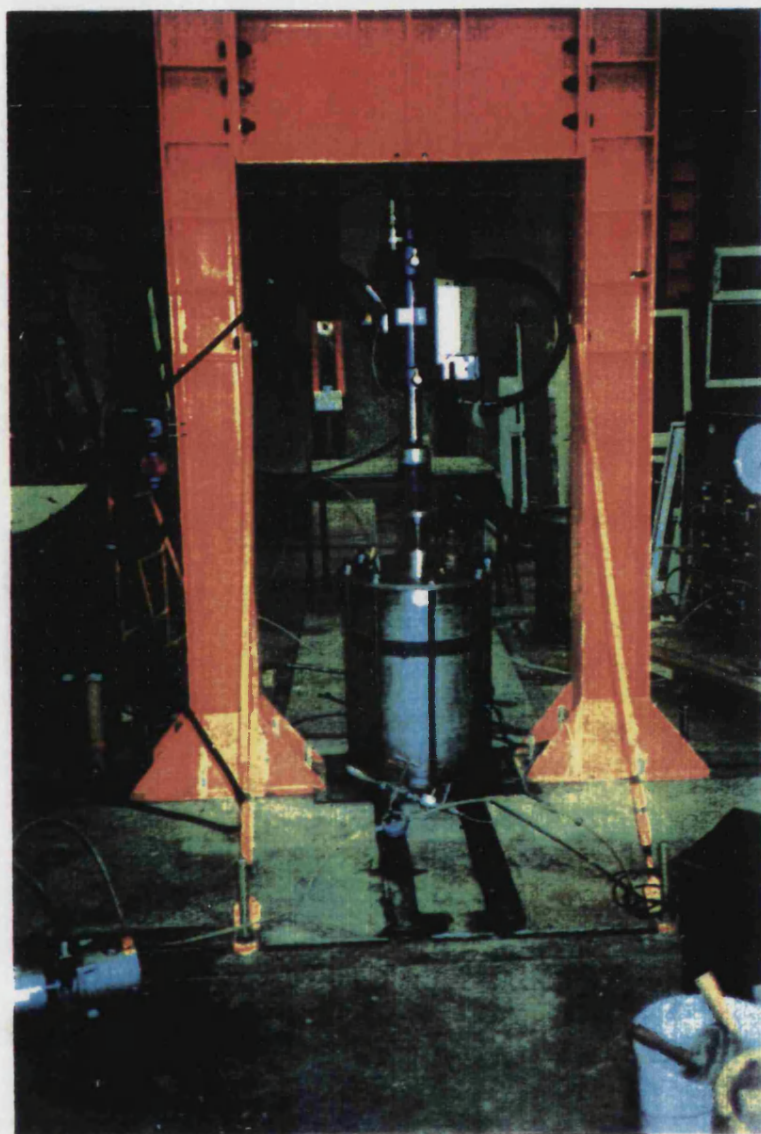


Plate 8.1 Large cell in placed within load reaction frame on the strong floor. The greased track can be seen extending away from the frame. This plate shows the 50kN fatigue actuator in position.

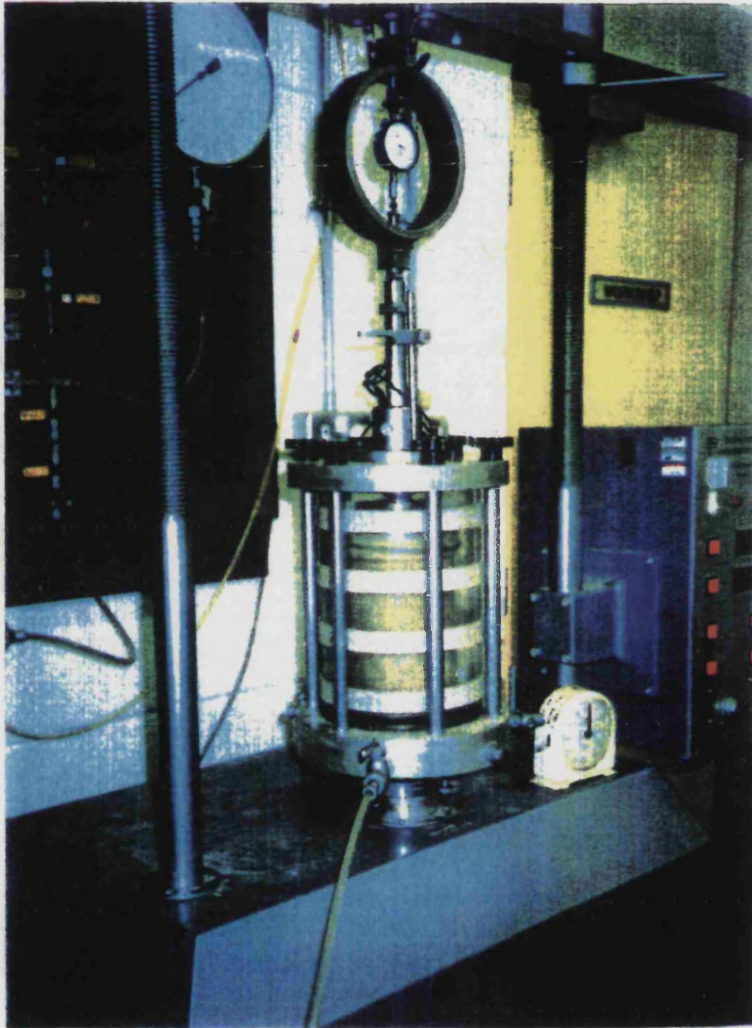


Plate 8.2 100mm cell assembled in 10 tonne loading frame showing proving ring arrangement. Cell pressure is being applied but the sample vacuum is yet to be released.

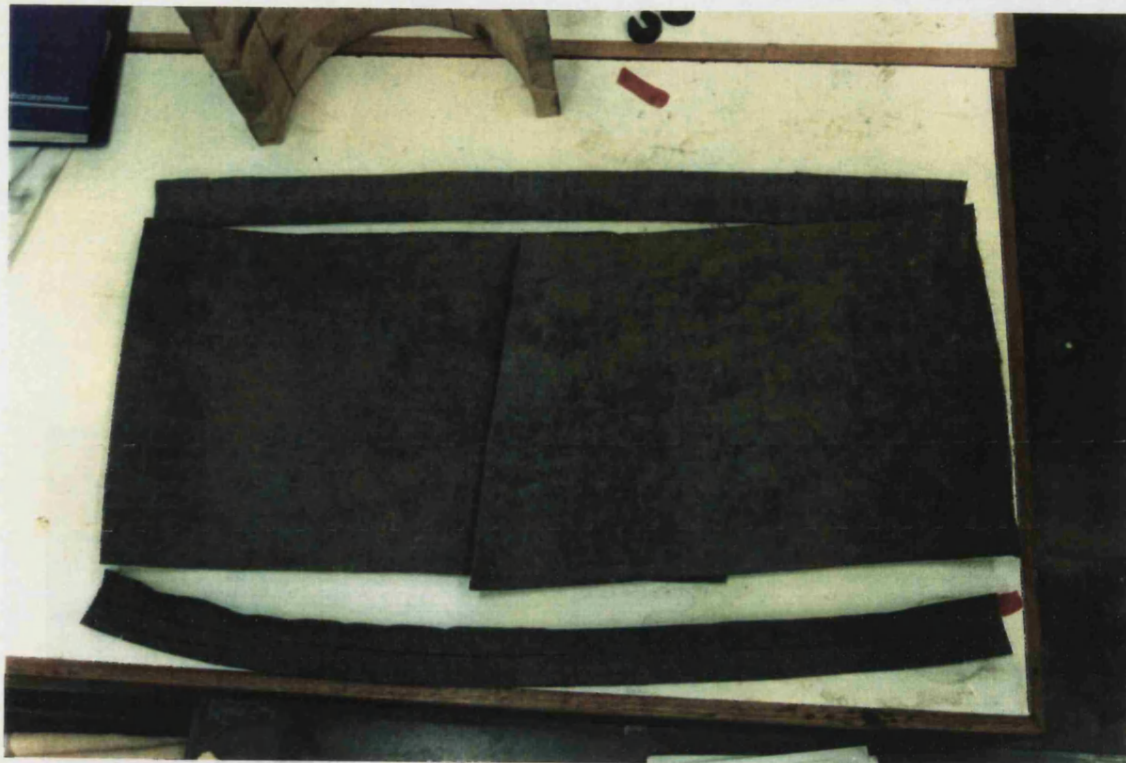


Plate 8.3a This shows the individual elements of the membrane protection utilised when testing the large grained material in the 254mm samples.

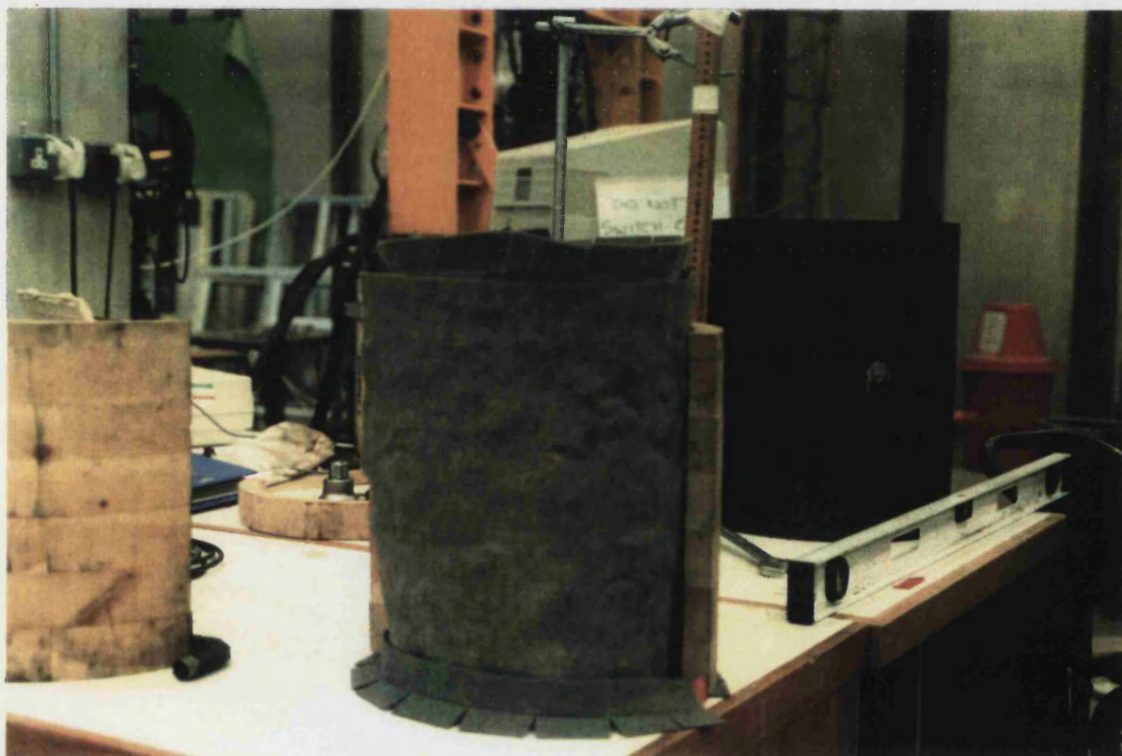


Plate 8.3b The membrane protection in the 'erected' form as placed against the inside face of the membrane.

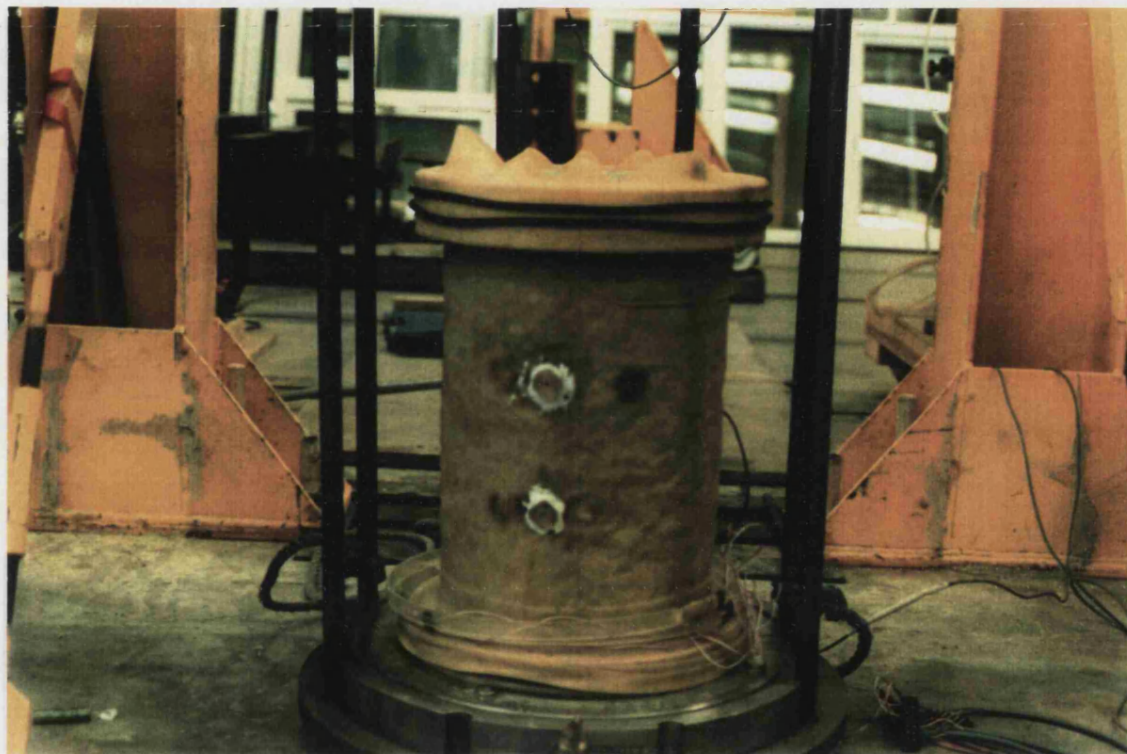


Plate 8.4. Sealant around location studs.

9.0 CONCLUSIONS AND FUTURE WORK

This chapter brings together the results of the work completed to date, and puts it into context with work planned for the future.

9.1 CONCLUSIONS

A large scale triaxial apparatus capable of holding samples up to 304mm in diameter has been brought into operation with all the associated control and monitoring devices required for a range of both static and dynamic testing regimes.

The procedures required to form large samples of particulate material on a regular basis have been perfected, and problems of membrane penetration associated with the testing of large angular particles have been solved.

The principle of utilising large 'strain rings' to measure the radial expansion of soil samples has been investigated. The interpretation of the resulting data is complicated by the inherent damping within the monitoring system and security of the location studs. Alternative non contacting displacement transducers are recommended.

The decision to limit the overall axial dimension of the apparatus, thus limiting the diameter:height ratio of the larger samples, though unavoidable and believed acceptable at the time has proved troublesome. The friction free ends utilised to date have proved to be inconsistent. Working friction free end plattens have been reported as early as the 1960's, e.g. Bishop and Green (1965), although on much smaller samples. However, Hettler and Valdoulakis (1984) have reported success on their large oversquare rig using ground glass elements. This would not seem therefore to present an insuperable problem, but was beyond the resources of this current work. However, it should be noted that the reported successful usage of friction free end plattens in the past has related to static testing.

Tests on a range of sands and chippings has shown that consistent results can be achieved from 254mm samples with a reduced height:diameter ratio when compared with 100mm samples of traditional sample dimensions.

Comparable tests between the 38mm, 100mm, and 254mm samples on a range of material from fine sands through to fine gravels, indicate that the 38mm samples give consistently lower values for the angle of friction. This is of particular interest, as the opposite would be expected due to end effects and ratio of sample

dimension to particle size. The majority of comparable testing to date has utilised high relative densities and it is suggested that this effect may be due to the small sample diameter relative to the particle size inhibiting the formation of dense samples. Further comparable testing at lower relative densities would shed further light on this aspect.

In terms of the overall objectives of the project, the further investigation into the scaling effects of different sized samples, though necessary for the long term interpretation of the dynamic tests, deflected effort from the development of the large cell and procedures for the dynamic testing itself.

9.2 FUTURE WORK

The new large scale triaxial apparatus described in this work provides a sound basis for development and the incorporation of a variety of sensors that would both simplify and increase the efficiency of the testing. The use of a load cell within the triaxial cell is essential for low stress/strain applications and dynamic testing. Utilising non contacting displacement transducer technology would significantly enhance the repeatability and accuracy of measuring the sample deformation. Increasing the overall dimensions of the

cell to allow the use of 'normal' sample diameter:height ratio's would alleviate the need for friction free end plattens. The design of the apparatus would easily accomodate this change, requiring a longer 'tube' element and new tie studs. Manufacturing considerations would need to be considered if this option was followed. Forming samples this size would be a laborious and time consuming task, requiring extreme care to ensure consistent samples.

In terms of the testing itself, additional comparisons between the 38mm and 100mm cell are required at low and medium relative densities to further examine the phenomenon of the smaller samples giving lower values for the friction angle.

The review of previous work in section 3, gave conflicting reports on the relationship between damping and relative density for large grained particulate material. Investigation would provide more information on the matter, allowing a consistent comparison between the previous work. That too, was carried out utilising triaxial cells, and thus the procedures for extracting damping values would be consistent, avoiding the use of dissimilar numerical models and eradicating a possible source for the spread in damping values previously reported.

From the experience gained it is recommended that the continuation of the dynamic testing, initially be pursued on the 100mm cell. This would significantly reduce the sample preparation time and allow development at a quicker rate. Although the associated sensor requirements will be at a much reduced scale, itself introducing a new set of parameters requiring attention.

The large triaxial rig in its present configuration is, in the short term, more suited to continuation of the static testing. This would allow investigation into the validity of producing graded samples by shifting the grading curve down the size axis, and provide information on the scaling of graded samples between the different cell sizes.

As well as aspects of gradation, the effect of angularity also requires consideration. This needs to be looked at from both graded and single sized samples. It is to be expected that a single sized sample of highly angular material would have higher stresses at inter-particle contact points than a single sized sample of rounded material due to the fewer contact points, affecting both the stiffness and damping response. Such differences would be more noticable with larger grained material. These differences might reduce with graded samples as the amount of contact points increase and

become more evenly matched. The larger cell provides the facility for these effects to be investigated.

BIBLIOGRAPHY

1. Abdel-Ghaffer, A.M. and Scott, R.F.
"Shear Moduli and Damping Factors of Earth Dam"
Journ. of Geotech. Eng. Div. ASCE 1979 pp 1405-1426
2. Abdel-Ghaffer, A.M. and Scott, R.F.
"Vibration Tests of Full-Scale Earth Dam"
Journ. of Geotech. Eng. Div. ASCE 1981 pp 241-269
3. Abdel-Ghaffer, A.M. and Scott, R.F.
"Comparative Study of Dynamic Response of Earth Dam"
Journ. of Geotech. Eng. Div. ASCE 1981 pp 271-286
4. Allen, J. and Thompson, M.
"Resilient Response of Granular Materials Subject
to Time Dependent Lateral Stresses"
Trans. Research Record No.510 1974 pp 1-14
5. Bishop, A.W. and Green, G.E.
"The influence of end restraint on the compression
strength of a cohesionless soil"
Geotechnique 1965, pp 243 - 266
6. Boyce, J.R.
"The Behaviour of a Granular Material under
Repeated Loading"
PhD. Thesis, University of Nottingham 1976
7. Braja, M.D.
Fundamentals of Soil Machanics,
Pub. Elsevier, 1983
8. Brauns, J., Kast, K. and Blinde, A.
"Compaction Effects on the Mechanical and
Saturation Behaviour of Disintegrated Rockfill"
Int. Conf. on Compaction, 1980
9. Brown, S.F.
"Significance of Variably Confined Triaxial
Testing"
Proc. of ASCE 1975 pp 832-834
10. Brown, S.F.
"Repeated Load Testing of a Granular Material"
Journ. of Geotech. Eng. Div. ASCE 1974 pp 825-841
11. Brown, S.F. and Snaith, M.S.
"The Measurement of Recoverable and Irrecoverable
Deformation in the Repeated Load Triaxial Test"
Geotechnique 1974, pp 255-259

12. Brown, S.F. and Hyde, A.F.L.
"Significance of Cyclic Confining Stress in
Repeated Load Triaxial Testing of Granular
Material"
Trans. Research Record No.573, 1975 pp 42-58
13. CIBA - GEIGY
Moulds and Release Treatments for Araldite and
Ureol resins
Instruction Manual No. M.19e
14. CIBA - GEIGY
Araldite Electronics Resins. Low-Viscosity
filled flame-retardent epoxy casting system.
Araldite CW1302GB, Hardener HY1300GB
Instruction Sheet No. C.112a
15. CIBA - GEIGY
Araldite Electronics Resins. Low-Viscosity
unfilled epoxy casting and impregnating system.
Araldite CY1301GB, Hardener HY1300GB
Instruction Sheet No. C.110a
16. CIBA - GEIGY
Safety Manual. Handling Precautions for Araldite
Epoxy Resin Materials
Safety Manual No. M.37k
17. CIBA - GEIGY
Araldite MY778 with Hardener HY951 or HY956 Epoxy
casting and impregnating resin
Instruction Sheet No. C.47c
18. Colloms, M.
Computer Controlled Testing and Instrumentation
An introduction to the IEC-625: IEEE-488 Bus
Pentech Press
19. Hardin, B.O. and Black, W.L.
"Sand Stiffness under Various Triaxial Stresses"
Journ. of Soil Mech. and Founds. Div. ASCE 1966,
pp 27-42
20. Hardin, B.O. and Drnevich, V.P.
"Shear Modulus and Damping in Soils: Design
Equations and Curves"
Journ. of Soil Mech. and Founds. Div. ASCE 1972
pp 667-691
21. Hardin, B.O. and Drnevich, V.P.
"Shear Modulus and Damping in Soils: Measurement
and Parameter Effects"
Journ. of Soil Mech. and Founds. Div. ASCE 1972
pp 603-623

22. Head, K.H.
Manual of Soil Laboratory Testing
Vols. 1,2 & 3
Pentech Press
23. Hettler, A. and Vardoulakis, I.
"Behaviour of Dry Sand Tested in a Large Triaxial Apparatus"
Geotechnique 1984 pp 183-197
24. Hicks, R.G.
"The Resilient Response of Granular Materials"
PhD. Thesis University of California 1970
25. Hicks, R.G. and Monismith, C.L.
"Factors Influencing the Resilient Response of Granular Materials"
Highway Research Board No. 345 1971 pp 15-31
26. Idriss, I.M. and Seed, H.B.
"Siesmic Response of Horizontal Soil Layers"
Journ. of Soil Mech. and Founds. Div. ASCE 1968
pp 1003-1031
27. Jacobsen, L.S.
"Steady Forced Vibration as Influenced by Damping"
Trans. ASME, 1930, pp 169-181
28. Jacobsen, L.S.
"Damping in Composite Structures"
Proc. 2nd World Conference on Earthquake Engineering, Tokyo, 1960, pp 1029-1044
29. Jardine, R.J., Symes, M.j., and Burland, J.B.
"The Measurement of Soil Stiffness in the triaxial apparatus"
Geotechnique 1984 pp 323-341
30. Jessberger, H.L. and Dorr, R.
"Behavior of Dynamically Loaded Granular Materials"
X ICSMFE *, pp 655-660.
31. KAMAN MEASURING SYSTEMS
Measurement Solutions Handbook, No. 881
32. Kast, K. and Brauns, J.
"Dynamic Compaction of Rockfill Samples"
X ICSMFE *, pp 669-671.
33. Kenjo, T
Stepping motors and their microprocessor controls
Clarendon Press

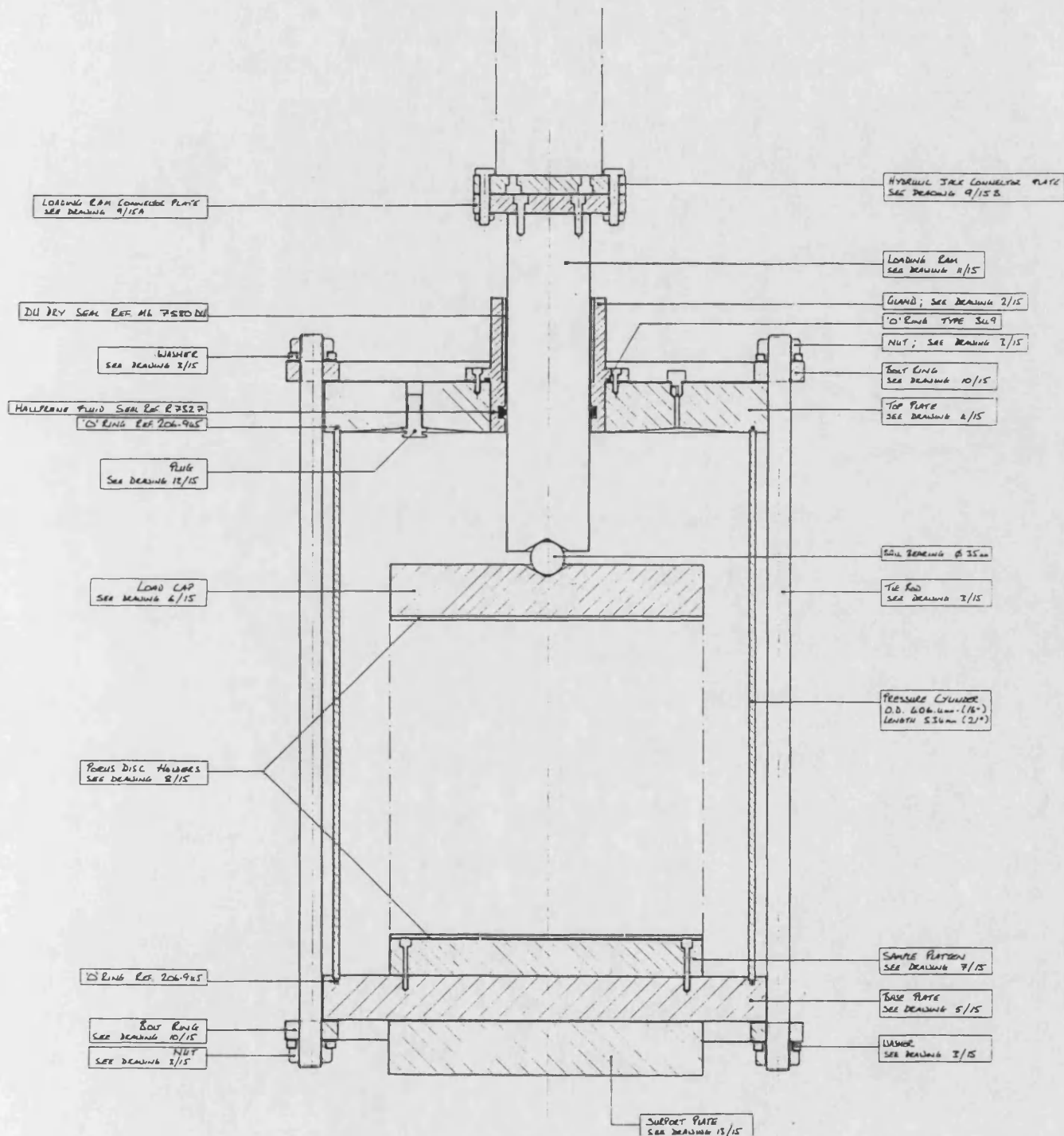
34. Kokusho, T. and Esastri, Y.
"Cyclic Triaxial Test on Sands and Course
Materails"
X ICSMFE *, pp 673-676.
35. Kokusho, T.
"Cyclic Triaxial Test of Dynamic Soil Properties
for Wide Strain Range"
Soils and Foundations. Japanese Society of Soil
Mech. and Found. Eng. 1980, Vol.20, No.2, pp 45-60
36. Mangla
Binnie & Partners
Reprint from Proc. Instn civ. Engrs, 1967, 38 pp
337-567, and 1968, 41 pp 119-203.
37. Moore, W.M., Swift, G. and Milberger, L.J.
"Deformation Mearuring System for Repetitively
Loaded, Large Diameter Specimens of Granular
Material"
Highway Research Record No. 301 1969, pp 28-39
38. Moore, W. et al
"A Laboratory Study of the Relationship of Stress
and Strain for a Crushed Limestone Material"
Research Report 99-5F Texas Transportation
Institute 1970
39. Parkin, A.K. and Adikari, G.S.N.
"Rockfill Deformation from Large Scale Tests"
X ICSMFE *,pp 727-731
40. Raju, V.S. and Sadisivan, S.K.
"Membrane Penetration in Triaxial Tests on Sands"
Proc. of ASCE 1974, pp 482-489.
41. Ramana, K.V. and Raju, V.S.
"Constant-Volume Triaxial Tests to Study the
Effects of Membrane Penetration"
Geotech. Testing Journal 1981, pp 117-122
42. Sarma, S.A. and Haeri, S.M.
"Behaviour of Earth and Rockfill Dams Subjected to
Earthquakes"
SECED Conf., Univ. Bristol 1989, paper 22
43. Schnaid, F. and Houlsby, G.T.
"An assesment of chamber size effects in the
calibration of in situ tests in sand"
Geotechnique 1991, pp 437-445

* 10th International Conference on Soil Mechanics and
Foundation Engineering, 1981

44. Seed, H.B. and Lee, K.L.
"Liquifaction of Saturated sands during Cyclic Loading"
Journ. of Soil Mech. and Founds. Div. ASCE 1966 pp 105-134
45. Seed, H.B. and Idriss, I.M.
"Soil Moduli and Damping Factors of Dynamic Response Analysis"
Report EERC 70 - 10, 1970
46. Seed, H.B., Wong, R.T., Idriss, I.M., Tokimatsu, K.
"Moduli and Damping Factors for Dynamic Analysis of Cohesionless Soils"
Journ. of Geotech. Eng. ASCE 1986, pp 1016-1032
47. Sherif, M.A., Ishibashi, I., and Gaddah, A.H.
"Damping Ratio for Dry Sands"
Journ. of Geotech. Eng. Div. ASCE 1977, pp 743-755
48. Silver, M. and Park, T.K.
"Testing Procedure Effects on Dynamic Soil Behaviour"
Journ. of Geotech. Eng. Div. ASCE 1975, pp 1061-1083
49. Silver, M. and Seed, H.B.
"The Behaviour of Sands under Siesmic Loading Conditions"
Report EERC 69 - 16, 1969
50. Weissman, G.F. and Hart, R.R.
"The Damping Capacity of Some Granular Soils"
ASTM No. 305, 1961
51. Wong, R. and Seed, H.B.
"Liquifaction of Gravelly Soils under Cyclic Loading Conditions"
Report EERC 74 - 11, 1974

APPENDIX 1 Specification of Large Triaxial Cell

REF	DESCRIPTION	MATERIAL	O/A SIZE	QTY	REF	DESCRIPTION	MATERIAL	O/A SIZE	QTY	REF	DESCRIPTION	MATERIAL	O/A SIZE	QTY
1	PRESSURE CYLINDER	STAIN ST	O.D. 606.4mm x 518mm	ONE	13	LOADING RAM CONNECTOR PLATE	MILD ST	Ø 167mm x 18mm	ONE	25	M5 SOCKET SCREW	STAIN ST	M5 x 25mm	16
2	GLAND	STAIN ST	Ø 160mm x 152mm	ONE	14	BOLT RINGS	MILD ST	Ø 8.9mm x 20mm	TWO	26	M10 BOLT	MILD ST	M10 x mm 50mm	RIGHT
3	TIE ROD	MILD ST	Ø 20mm x 715mm	RIGHT	15	LOADING RAM	STAIN ST	Ø 80mm x 324mm	ONE	27	M10 NUT	MILD ST	M10mm	RIGHT
4	M10 NUTS (RIGHT HAND THREAD)	MILD ST	AP. 32.95mm x 22mm	16	16	DU DRY SEAL M5 P580 DU			ONE	28	SEED VALVE	STAIN ST	Ø 18mm x 27mm	ONE
5					17	HALF BONE FLUID SEAL R752P			ONE	29	PLUG	STAIN ST	Ø 19mm x 32mm	NINE
6	WASHERS	MILD ST	Ø 66.5mm x 9mm	16	18	O'RING TYPE 349	CRUIS. SECTION	5.10mm	ONE	30	ROOSTY SCREW 435-009-1524-91			ONE
7	TOP PLATE	STAIN ST	Ø 466.5mm x 50mm	ONE	19	O'RING TYPE 206-965	CRUIS. SECTION	7.50mm	TWO	31	O'RING TYPE 012	CRUIS. SECTION	178	NINE
8	BASE PLATE	STAIN ST	Ø 466.5mm x 40mm	ONE	20	O'RING TYPE 115	CRUIS. SECTION	2.62mm	ONE	22	SUPPORT PLATE	MILD ST	Ø 306.8mm x 50mm	ONE
9	LOAD CAP	ALUMINUM	Ø 306.8mm x 50mm	ONE	21	ROLL BEARING	STAIN ST	Ø 15	ONE	33	M5 SOCKET CAP SCREWS	STAIN ST	M5 x 60mm	ELIGHT
10	SAMPLE PLATTEN	STAIN ST	Ø 306.8mm x 30mm	ONE	22	M5 SOCKET CAP SCREW	STAIN ST	M5 x 35mm	FIVE					
11	FORLIS DISC HOLDERS	ALUMINUM	Ø 306.8mm x 3mm	TWO	23	M5 SOCKET CAP SCREW	STAIN ST	M5 x 30mm	RIGHT					
12	HYDRAULIC JACK CONNECTOR PLATE	MILD ST	Ø 167mm x 18mm	ONE	24	M5 SOCKET CAP SCREW	STAIN ST	M5 x 20mm	SIX					



WEEK TITLE: GEOTECH. RESEARCH PROJECT

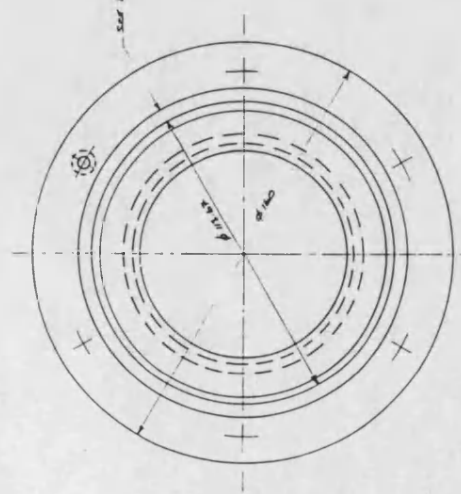
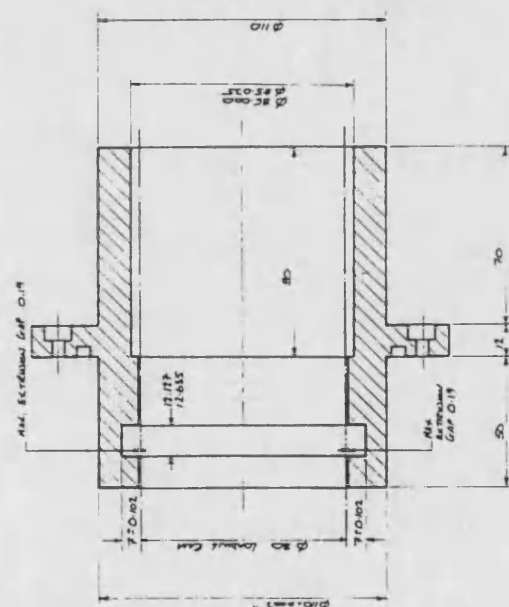
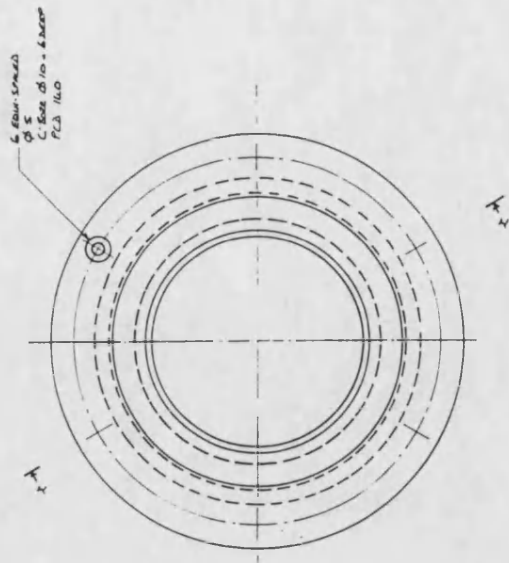
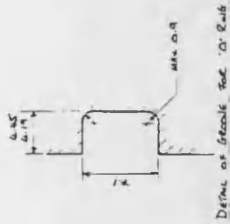
JOB TITLE: R. TAYLOR CELL

UNIVERSITY OF BATH SCH. OF ARCH. & B. ENGR.

SCALE: 1/2

DO NOT SCALE OFF DRAWING

DATE: 07/10/10 DRAWN BY: S. L. L. REVISED: 1/15



Sectional view X-X

* Revision 101 101 101

All dimensions are in millimeters unless stated

Material Type : STAINLESS STEEL
D/A Size : 10.0 - 10.0

Quantity : ONE

Design Title : GROUND BEAMS

Job Title : PROJECT MANAGER

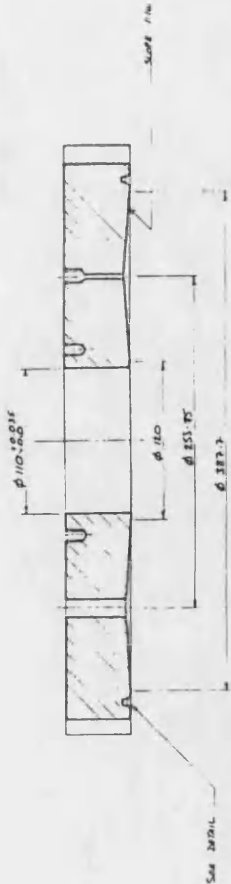
UNIVERSITY OF KENYA - KENYA

Scale : 1:1

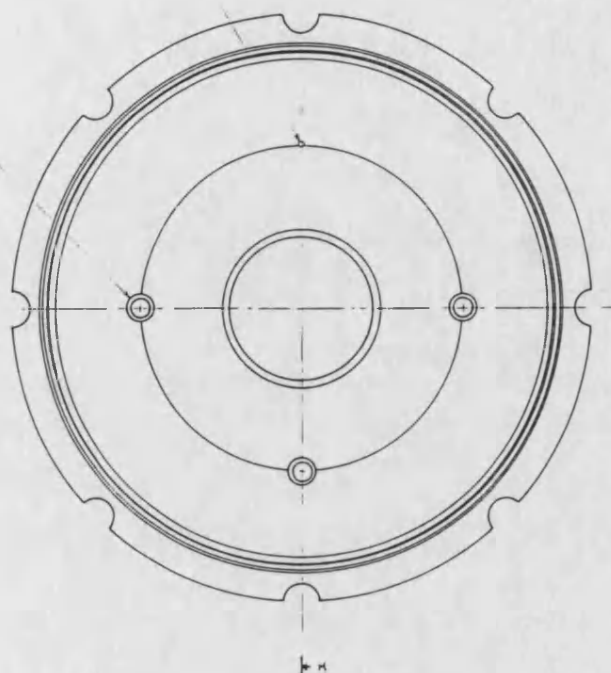
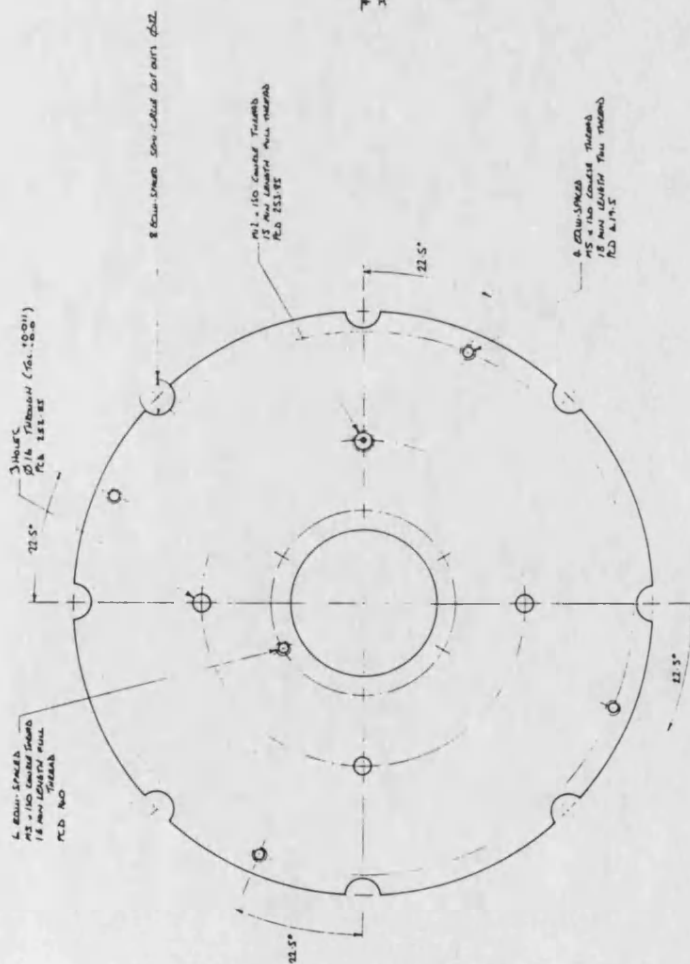
Date : 25/11/2010

Drawn : S. G. E.

2/15



Dorian of Persius (Maiden & O' End Elements)

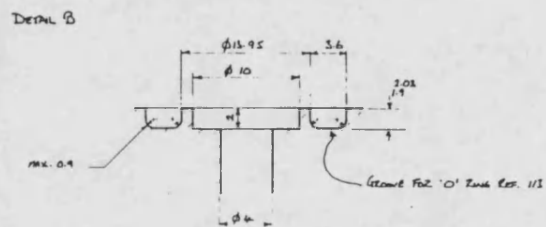
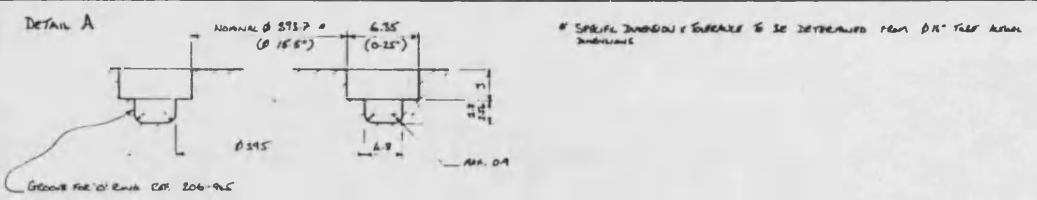
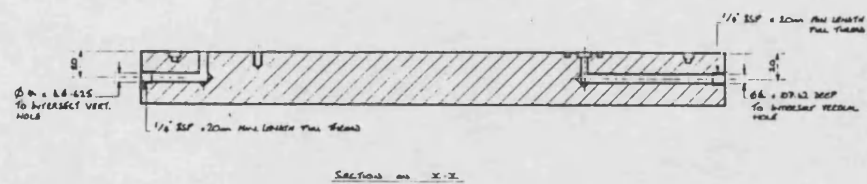
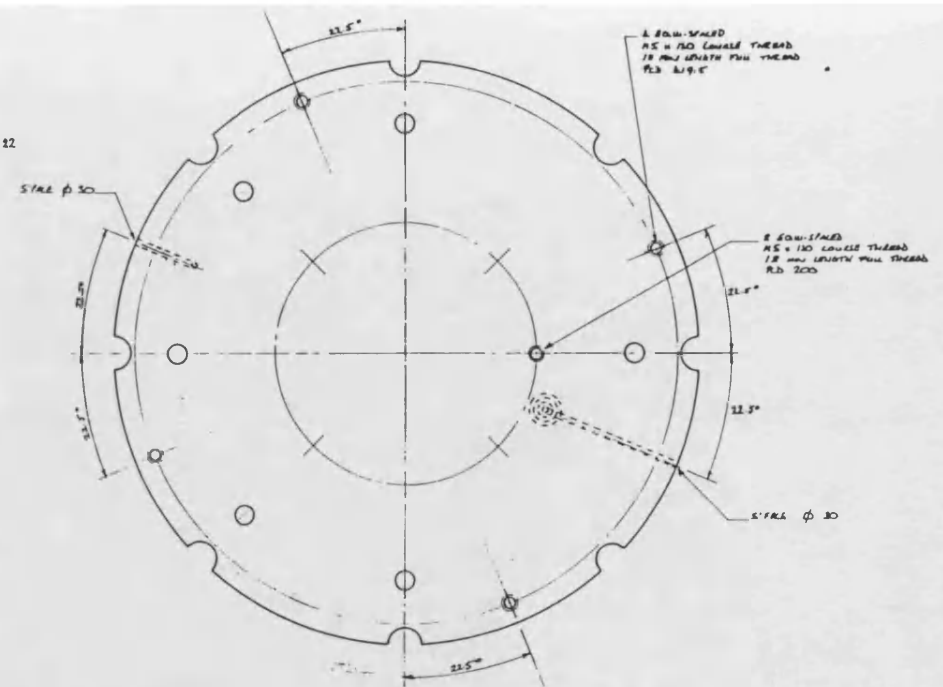
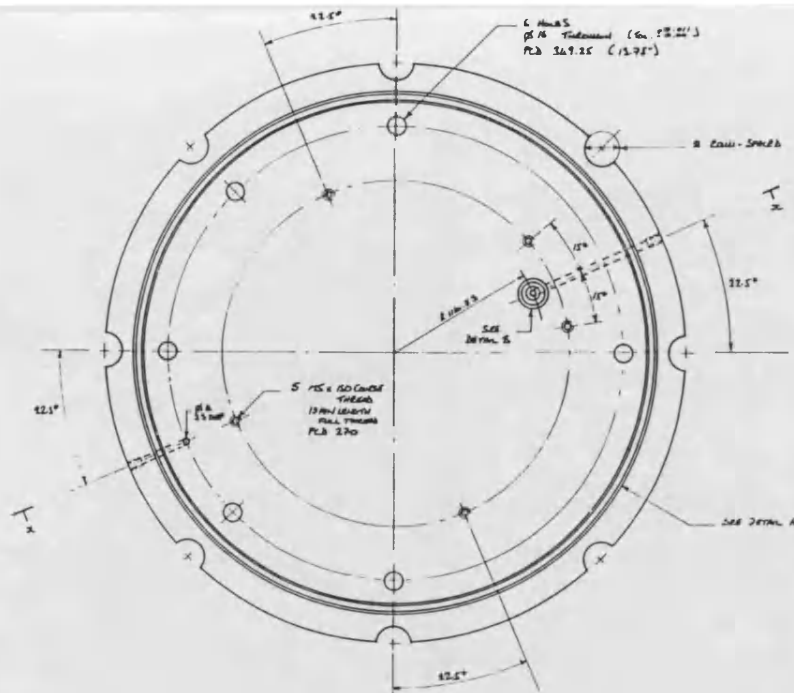


MATERIAL TYPE : STAINLESS STEEL
 S/A SIZE : $\phi 44.5 \times 50$ (INCH) $\phi 1.75 \times 1.98$ (INCH)

Δωάντη : ONE

ALL 3 PARTS SHOULD BE RUN UNLESS OTHERWISE SPECIFIED

DECU. TIME: 700 HOURS
SOL. TIME: 11° TERNAL CELL
UNIVERSITY OF CALIF. SOL OF MECH. & ENGRG
SCALE 1:2
DATE 25/12/88
RECEIVED 5.01. 20/15



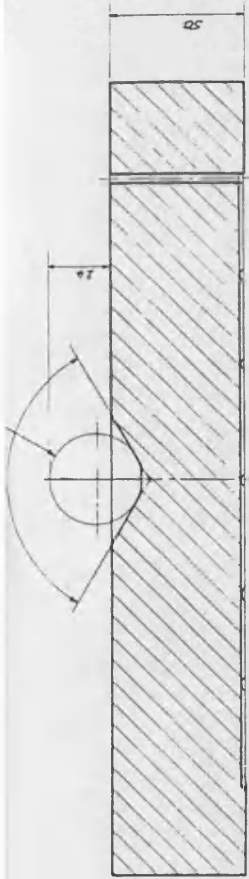
MATERIALS: STAINLESS STEEL

O/A SIZE: ϕ 42.5" = 40" (ϕ 17.5" = 15.75")

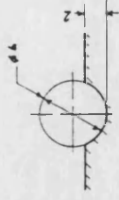
QUANTITY: ONE

DRW. TITLE	BASE PLATE
JOB TITLE	12" TENSILE CELL
UNIVERSITY OF	SEN. OF AGEN. & I. ENG.
SCALE	1:2
DATE	12/18
DESIGNER	S. L. R.
REVISION	5/15

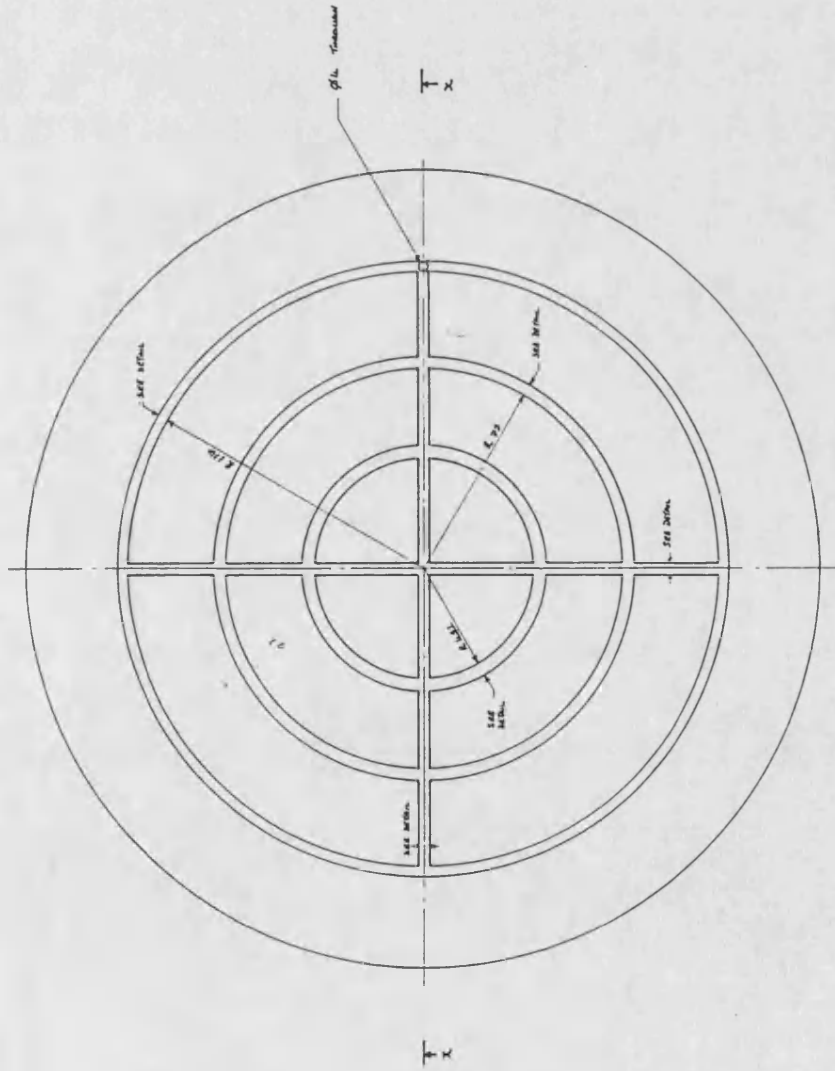
DO NOT SCALE DIMENSIONS



Section at X-X



Detail of Holes

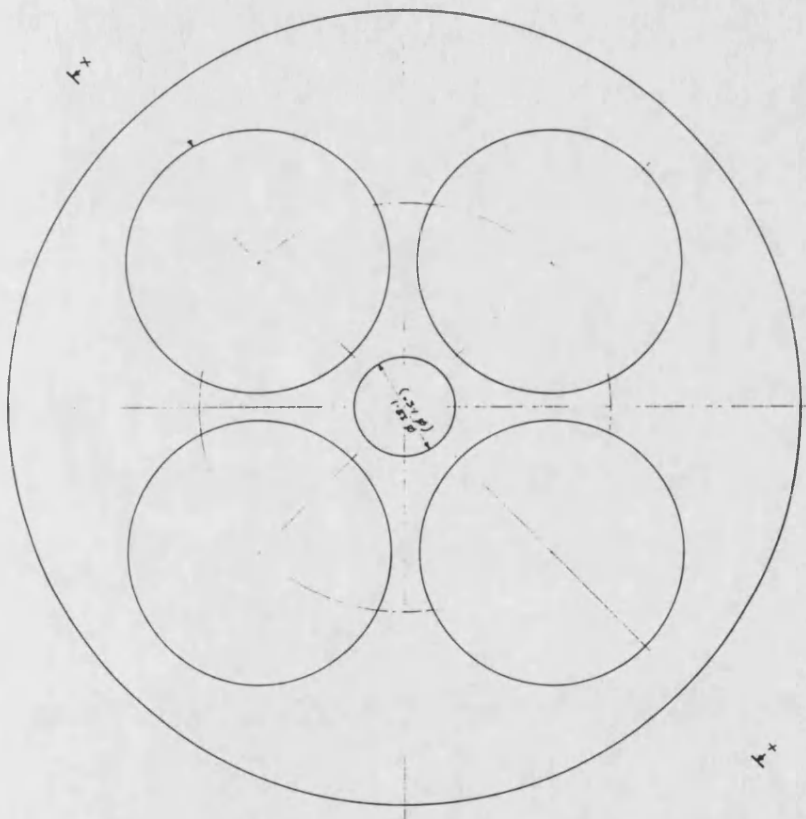


All dimensions in mm unless otherwise stated

Material: ALUMINUM
 S/A Size: 100mm x 50mm (ø 12" x 1/2")

Quantity: ONE

Rev. Title: LOW CP BENCH	Rev. Title: 12-12-12
Rev. Title: 12-12-12	Rev. Title: 12-12-12
University of Bath	Rev. Title: 12-12-12
Scale: 1:1	Rev. Title: 12-12-12
Date: 12/12/12	Rev. Title: 12-12-12



1. Holes Equi-Spaced
 12.01/12.0
 12.01/12.0



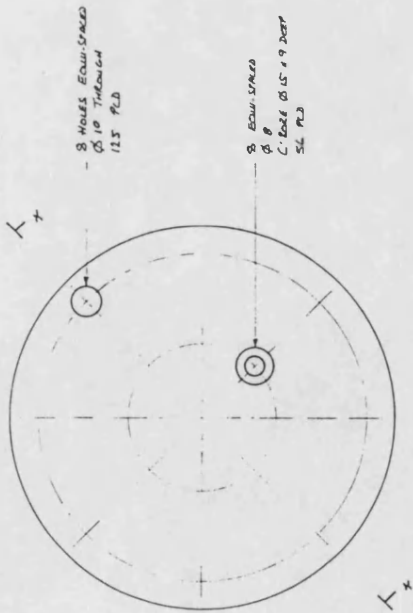
SECTION ON X-X

ALL DIMENSIONS ARE IN UNITS OF INCHES UNLESS OTHERWISE STATED

MATERIAL TYPE: ALUMINUM
 O/A SIZE: 1/2" x 12" x 12" (1/2" x 12" x 12")
 QUANTITY: TWO

DESIGN TITLE: PUMP DIS HOUSING
 JOB TITLE: JR. MECHANICAL CELL
 UNIVERSITY OF BATH SCI. OF MECH. & ELEC.
 SCALE: 1/1
 DATE: 15/11/88 DESIGNED BY: S. J. P. DRAWN BY: S. J. P.

DO NOT SCALE OFF DRAWING



SECTION ON X-X

MATERIAL: MILD STEEL

O/A SIZE: Ø 12.5" x 18"

QUANTITY: ONE

DATE: 8/17/77

SCALE: 1/1

DESIGNED BY: S.C.L.

DATE: 9/15/77

ALL DIMENSIONS ARE IN INCHES UNLESS OTHERWISE STATED

MATERIAL: MILD STEEL

O/A SIZE: Ø 12.5" x 18"

QUANTITY: ONE

DATE: 8/17/77

SCALE: 1/1

DESIGNED BY: S.C.L.

DATE: 9/15/77

ALL DIMENSIONS ARE IN INCHES UNLESS OTHERWISE STATED

MATERIAL: MILD STEEL

O/A SIZE: Ø 12.5" x 18"

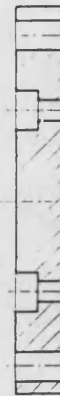
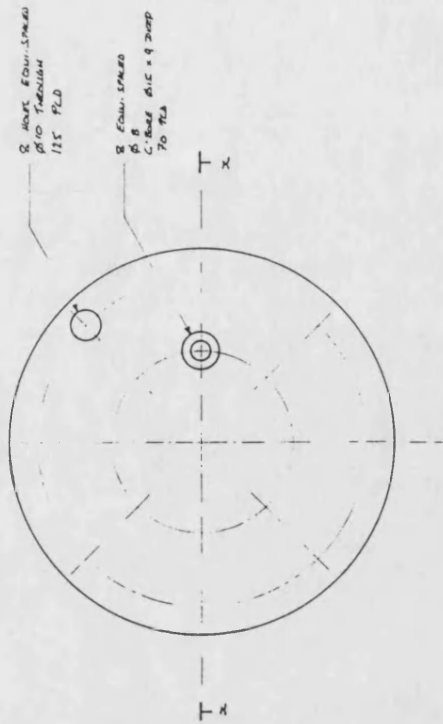
QUANTITY: ONE

DATE: 8/17/77

SCALE: 1/1

DESIGNED BY: S.C.L.

DATE: 9/15/77



SECTION ON X-X

MATERIAL: MILD STEEL

O/A SIZE: Ø 12.5" x 18"

QUANTITY: ONE

DATE: 8/17/77

SCALE: 1/1

DESIGNED BY: S.C.L.

DATE: 9/15/77

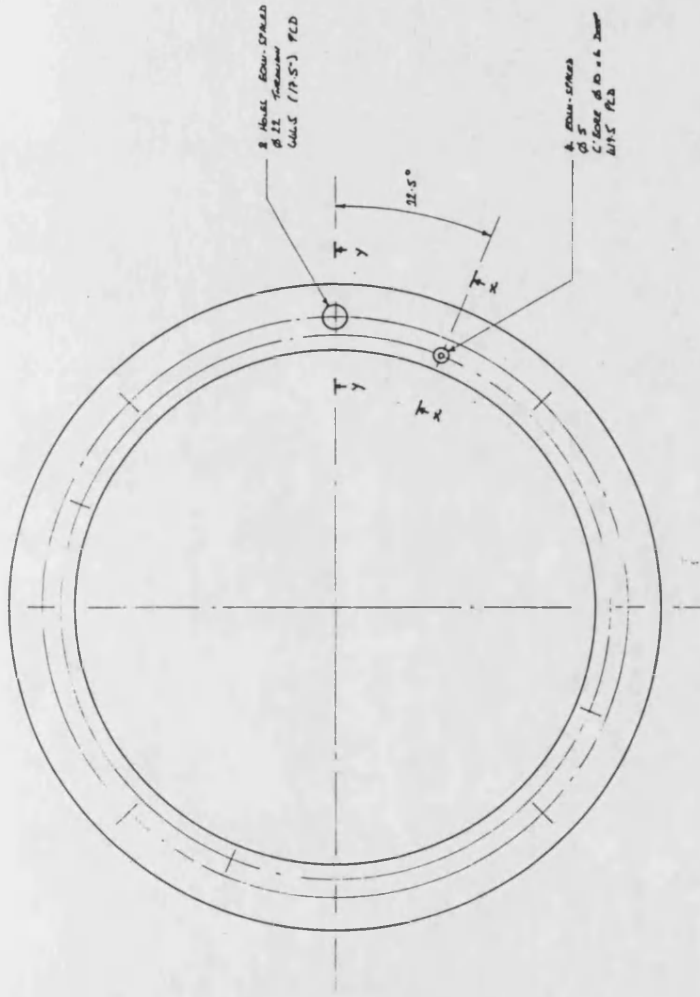
ALL DIMENSIONS ARE IN INCHES UNLESS OTHERWISE STATED



SECTION at X-X

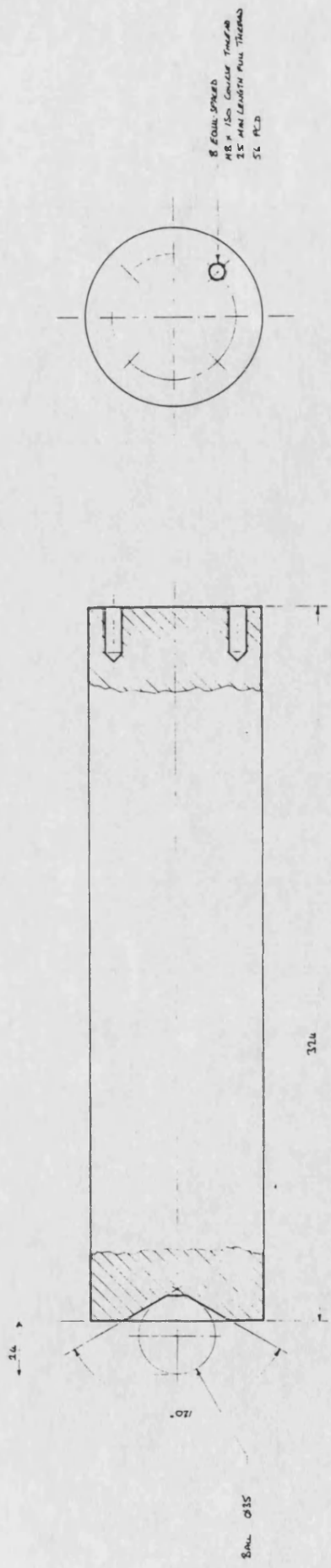


SECTION at Y-Y



ALL DIMENSIONS IN mm UNLESS OTHERWISE STATED

MATERIAL: MILD STEEL		REVISION: FOUR TIMES	
D/A SIZE: OUTSIDE DIA 476.5mm (INNER DIA 476.5mm) x 20mm		DATE: 1/2/19	
QUANTITY: TWO		DRAWN BY: P. L. S.	
		CHECKED BY: P. L. S.	
		DATE: 10/15	



8 EQUIDISTANT
H8 & H9 CONICAL TAPER
15 MAX LENGTH PULL THROUG
56 P.D

ALL DIMENSIONS IN INCHES UNLESS OTHERWISE SPECIFIED

MATERIAL: STAINLESS STEEL		DRAWN: LONNIE ODA	
D/A SIZE: ϕ 80mm x 324mm		JOB TITLE: 12-TRAILER CUL	
QUANTITY: ONE		UNIVERSITY OF BETH SEM OF MECH & E DII	
		SCALE: 1:1	
		DATE: 11/1/11	
		DRAWN BY: S.C.D	
		CHECKED BY: 11/15	



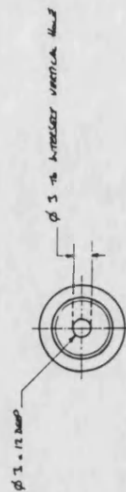
1.515

1.67

0.8

0.8

D' RING REX 012

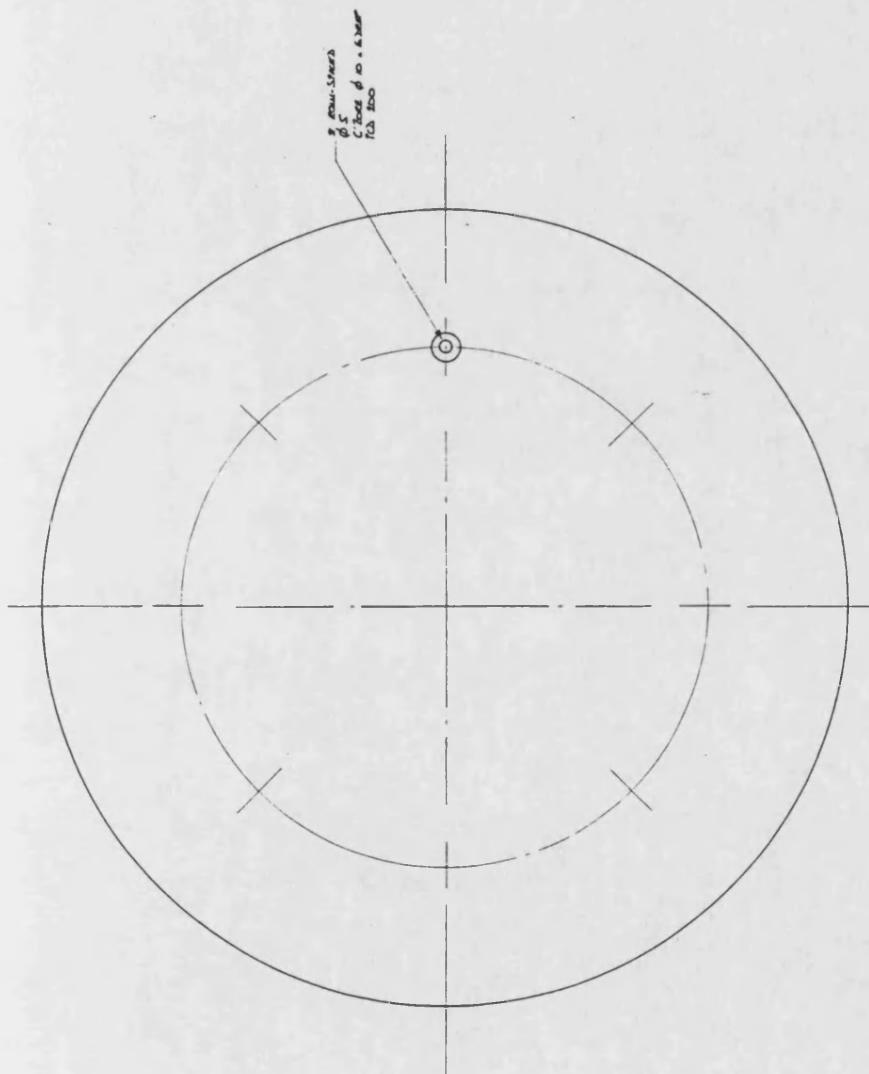


Blood Value

Plunk

All dimensions in mm unless stated

ART	15/12/1987	NEW	5.5.82	REMOVED 12/15
SCALE	2:1			
UNIVERSITY OF BATH	SEM ON ROAD + 8.5m			
JOBS TITLE	17-18/12/1987			
DRILL TITLE	17-18/12/1987			



As dimensioned in the relevant construction drawing

Material Type : Mild Steel
D/A Size : $\phi 304.8 \text{ mm} \times 50 \text{ mm}$ ($\phi 12 \times 1968$)

Quantity : ONE

Revised Title : SHEET 001

Job Title : 12" TRAVEL CELL

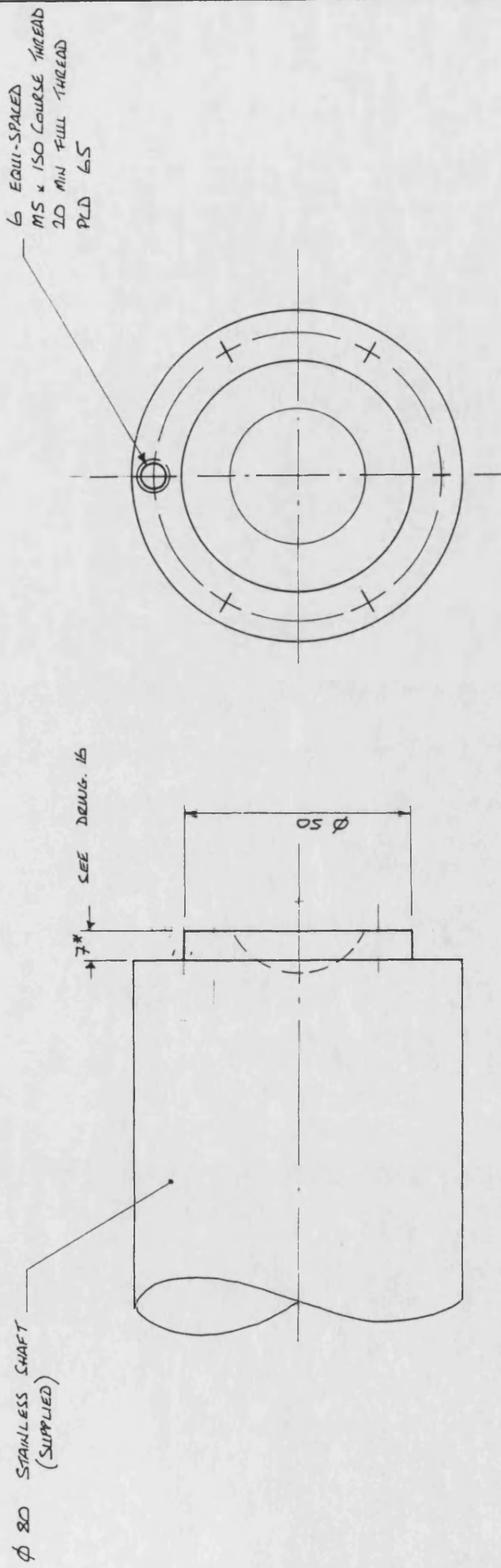
University of Bath SW. of Bath + E. of Bath

Scale : 1:1

Date : 15/1/08

Drawn : S. G. R.

Revised : 15/1/15



ALL DIMENSIONS IN MM

DO NOT SCALE OFF DRAWING

MATERIAL: STAINLESS STEEL

DIA SIZE: MODIFICATION TO EXISTING SHAFT AS SHOWN

NO. OFF :

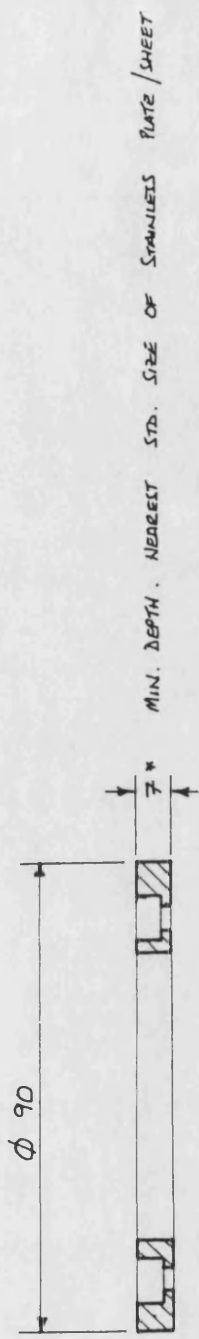
SCHE. TITLE: 12" TRIAXIAL CELL

DWG. TITLE: LOADING RIM MODIFICATION

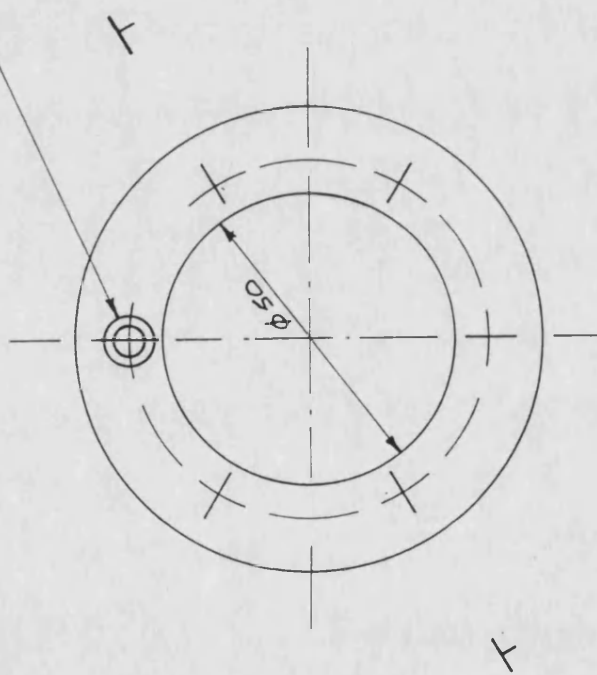
DWG. BY: S.G.R.

DATE: 19/11/84

DWG. NO. 17



6 EQUI-SPACED
 Ø 5
 C'BORE Ø 9.5 x 5 DEEP
 PCD 65



ALL DIMENSIONS IN MM

MATERIAL: STAINLESS STEEL O/A SIZE: Ø 90 x 7* No. OFF: ONE	JOB. TITLE 12" TRIAXIAL CELL	
	DWG. TITLE RAM PRESSURE PLATE	
	DWG. BY. S.G.R.	DATE 19/1/89
		DWG. NO. 16

APPENDIX 2 Modification to air/water pressure system

Details of Stepper Motor modification to air/water pressure system.

As mentioned in section 5.2 there was a need to provide a means of applying cyclic loading to the existing 100mm triaxial cells. A selection of these utilised the constant air/water pressure system explained in section 5.2. By adapting this system to include remote control of the regulator valve, the air pressure could be varied throughout the test.

The approach selected was to use a stepper motor mechanically linked to the regulator and controlled by a computer. Because a stepper motor moves in distinct uniform 'steps' its position is defined simply by counting the number of steps from a reference point. This removes the need to use additional position indicators and to incorporate them in a closed feedback loop with the associated timing and interfacing problems. Thus, as long as the stepper motor can be remotely controlled to a suitable tolerance a simpler open control system may be adopted.

A common example of a stepper motor utilising an open control system is in computer printers, whereby the printer head is moved by a linear stepper motor without any position indicators. The printer head is precisely

positioned by accurate control of the number of pulses sent to the stepper motor.

There are possible errors in the system, but for any given stepper motor these are not cumulative. They are not a function of the number of pulses sent, but are a feature of the mechanical design of the internal elements of the motor. It is not intended to give a description of stepper motor design here. The reader is referred to reference 33 in the bibliography if this is of interest.

The nature of the task considered here was to utilise a rotary stepper motor, connected via a mechanical fixing to an air regulator valve, in order to provide a variable air pressure. Two basic requirements were;

1. Accurate calibration of the air regulator, such that the pressure change due to any rotation throughout its range is defined.
2. Control system to send the appropriate number of pulses to the stepper motor in response to setup data.

It must be noted that an open control loop control system within this application is only satisfactory for load control. In order to control the system so that a

defined rate of displacement is applied, a position sensor to monitor the movement due to any pressure change would be required. This would need to be fed back within the control loop to determine the next signal to the stepper motor, thus forming a closed feedback system. The use of a computer controlled system maintained the flexibility, if required, to monitor remote sensors and compare expected response to actual measurements. To incorporate these measurements into the feedback loop would be possible if phasing and timing considerations were dealt with.

Standard stepper motor control cards are on the market, but were unavailable to this project. An advantage of designing and manufacturing a specific control card is that it can be tailored to the specific application. The control card had to perform a number of functions;

1. Receive a signal from the controlling computer. It was decided to use a standard ASC II serial signal for the command 'string', specifying the direction and amount of movement required from the stepper motor.
2. Convert the analogue command signal to digital form to enable processing.
3. Processing of the command signal.

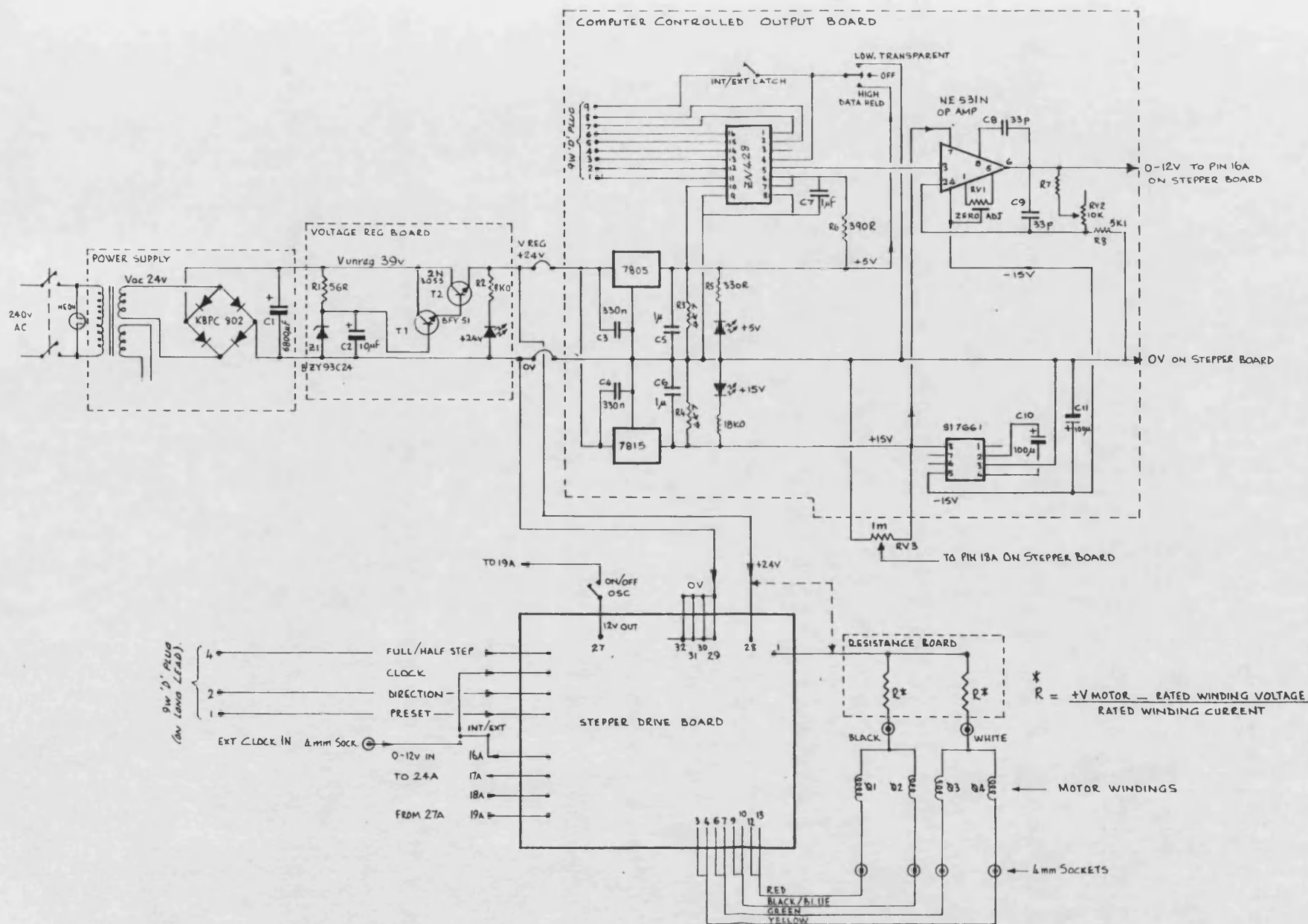
4. Stepper motor pulse generator.

The developed wiring details are included with reference material at the end of this section.

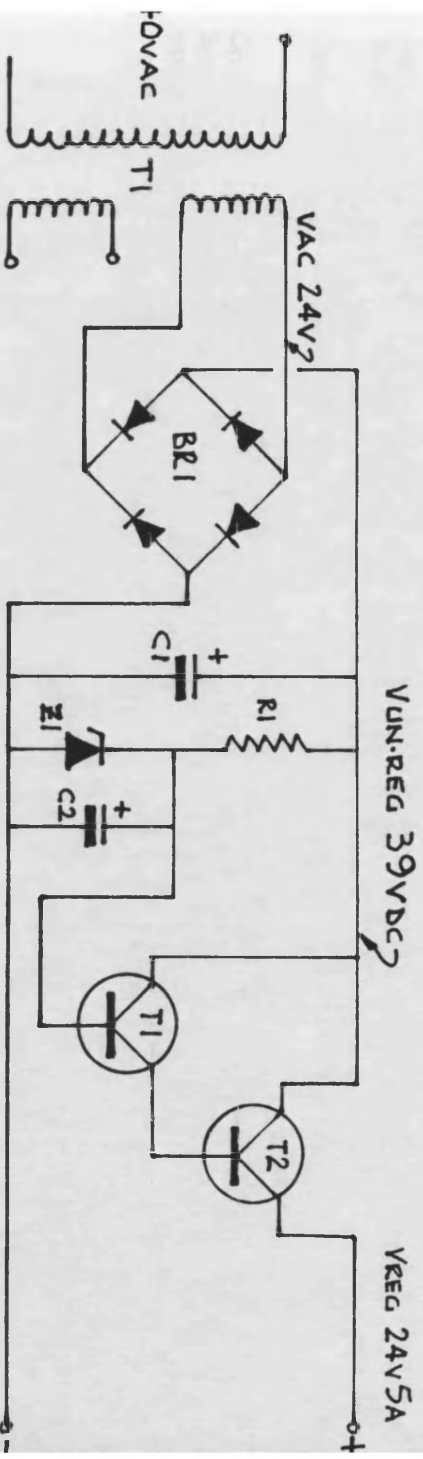
A BBC micro computer was chosen as the controller, due to the ability to directly access the I/O ports. Following development of the system, any processor capable of sending the required ASC II 'string' command via an RS 232 interface could be used.

Whilst the control system operated satisfactorily, the air/water pressurising system could not react to the changes at the air regulator. Pulsing occurred within the pipework, and compression in the air system modified the pressure at the air/water interface. To overcome these problems the pressure side could be converted to a full hydraulic system. There might still be a need to utilise a fully closed feedback control loop which negates the principal advantage of using a stepper motor. Closed feedback systems are more appropriate to AC motors.

As stated in the main text, the commissioning of a new structures laboratory removed the need for further development.



Power Supply for Stepper Drive



C1 = 6800 μ F 40V

C2 = 10 F μ 63V

R1 = 56R 7W

Z1 = ZENER 24V 6A

T1 = BFY 51

T2 = 2N3055E

BR1 = BRIDGE RECTIFIER 200V 8A

T1 = TRANSFORMER 200VA 0-24V 0-24V

SKINNER.

Fig. 2

Connection to RS stepper motors

When the windings of the RS Stepper Motors are assigned ($\phi 1 - \phi 4$) as shown in Fig. 3, they can be connected to the board according to Fig. 1.

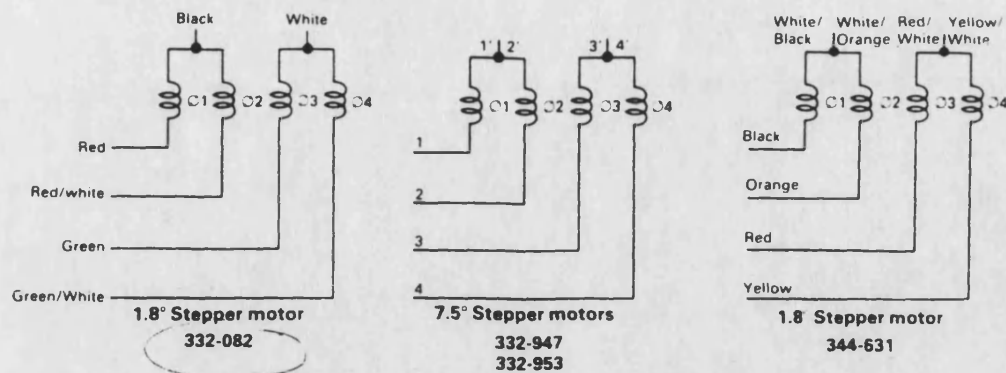


Fig. 3

If the supply voltage is set to 24V d.c. then R values for use with the RS motors are given in table 1 below:

motor	rated current (A)	rated winding voltage (V)	R (Ω)	power dissipation through R (W)
332-947	0.1	12	120	1.2
332-953	0.24	12	47	3
332-082	1	5	19	19
344-631	1.7	3	12.3	36

Table 1

For other design details and motors performance refer to RS data sheet 5550 on stepper motors.

On-board oscillator assembly

If external clock source is not available, an on-board oscillator can be assembled simply by soldering into place the required RS Components listed below.

Note: the oscillator clock output must be externally wired to the clock, input-pin 24a).

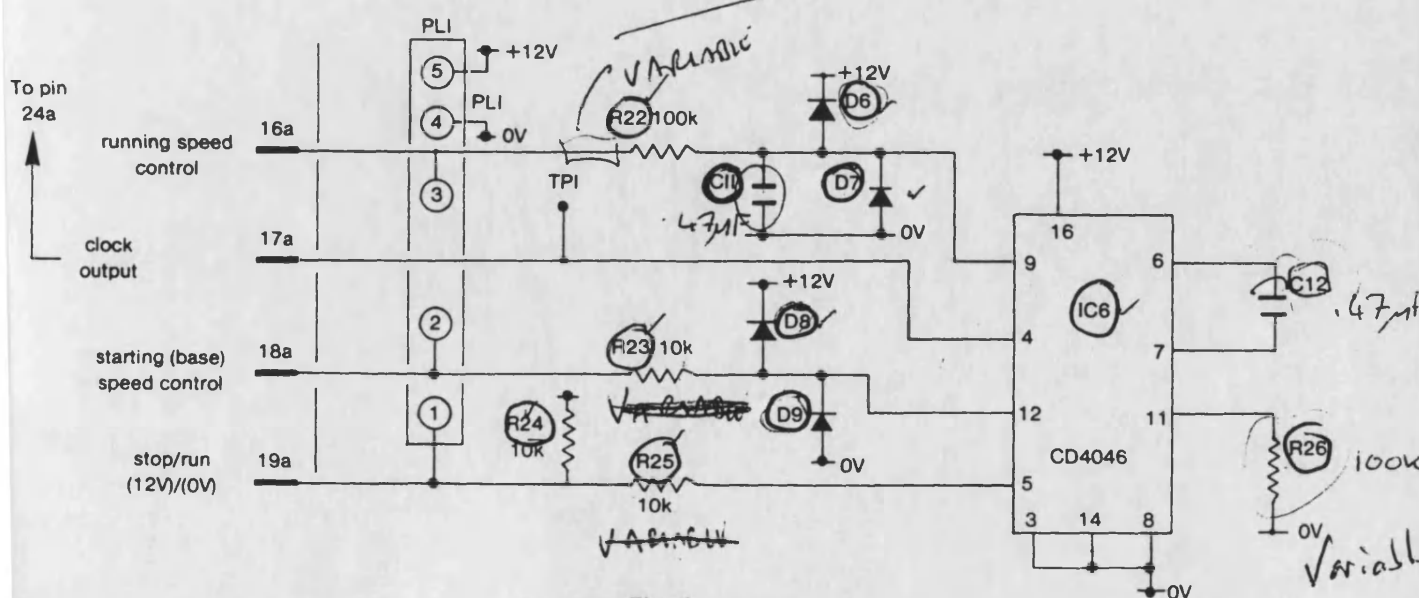


Fig. 4

R22	100K Ω resistor	RS 131-491	1 off	10 Ω
R23, 24, 25	10K Ω resistor	RS 131-378	3 off	10 Ω
D6, 7, 8, 9	signal diode	RS 271-606	4 off	10 Ω
IC6	CMOS I.C.	RS 306-645	1 off	1
R26, C11 & C12	(value depends on application)		1 off each	

If oscillator remote controls are required (e.g. front panel controls) then plug PLI (5-way RS inter-p.c.b. 467-576) can be added together with mating cable shell RS 467-627 and crimp terminals RS 467-598.

Starting (base) and running speed control

The on-board oscillator can be arranged to start at a fixed frequency (thus a fixed motor speed) and then ramp up to a final value (the running motor speed). This facility is available to start the motor within its pull-in performance region and then accelerate the motor through so that it can operate within the pull-out mode. On switch-off the motor decelerates automatically.

Three parameters need to be determined for any application:

- The starting speed: this should be below the pull-in speed for the motor (with any additional load).
- The running (final) speed: this should be within the pull-out capability of the motor (with any additional load).
- The acceleration and deceleration rate between starting and running speeds: this is limited by the motor capability to accelerate through its own (plus any load) inertia.

Oscillator controls (external)

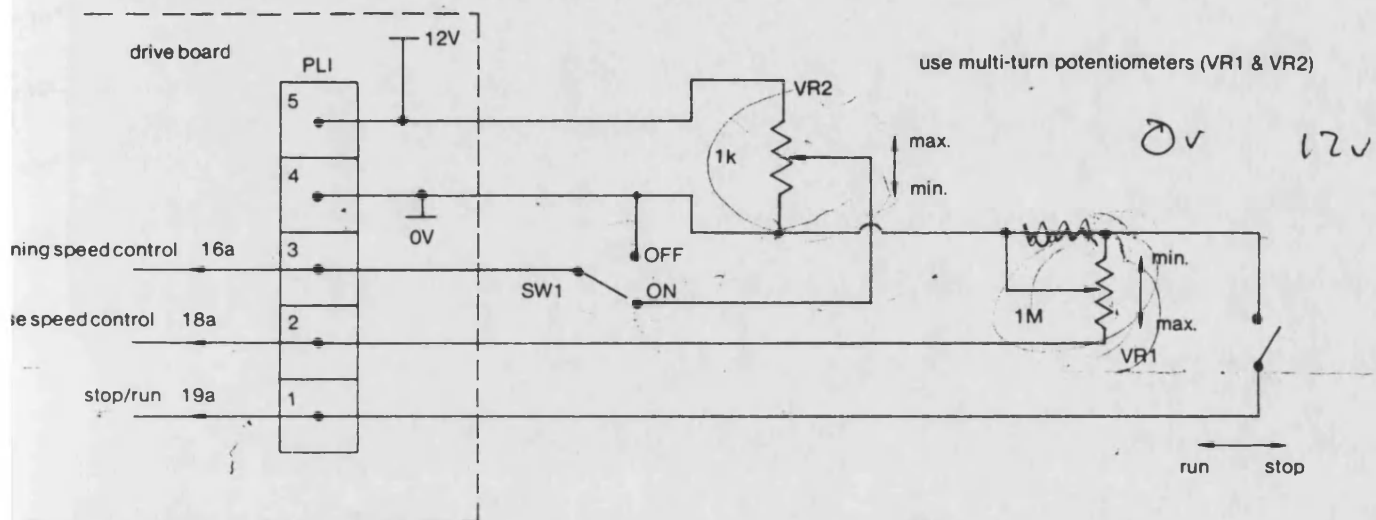


Fig. 5

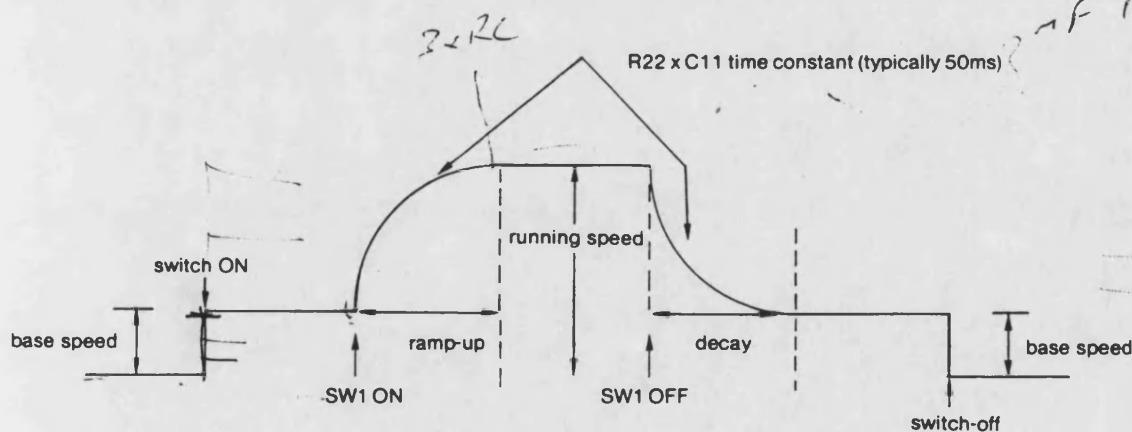


Fig. 6 Motor speed-ramping characteristic

Note: Oscillator frequency corresponds directly to motor speed in steps/s or half steps/s depending on motor drive mode.

For A 1.8° stepper motor:

$$\text{speed in revs/min} = \frac{60}{200} \times \text{speed in steps/s}$$

or
$$\frac{60}{400} \times \text{speed in half steps/s}$$

For A 7.5° stepper motor:

$$\text{speed in revs/min} = \frac{60}{48} \times \text{speed in steps/s}$$

or
$$\frac{60}{96} \times \text{speed in half steps/s}$$

Oscillator frequency setting

Recommended component values

VR1 0 – 1M Ω

VR2 1K Ω

R26 10K Ω – 1M Ω

C12 greater than 100pf

Determine the base frequency and maximum running frequency. Using Fig. 7 and the base frequency value choose a value for C12 and VR1. Calculate the ratio max. running frequency/base frequency to determine the ratio of $\frac{VR1 + R23 \text{ (fixed at } 10k\Omega \text{)}}{R26}$

R26

and thus using Fig. 8 establish the required value for R26.

base frequency (R26 = ∞ VR2 = min.)

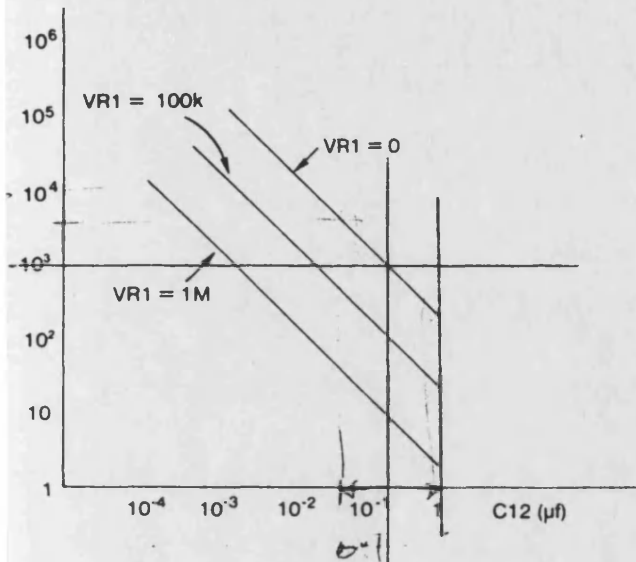


Fig. 7

max. running frequency/base frequency

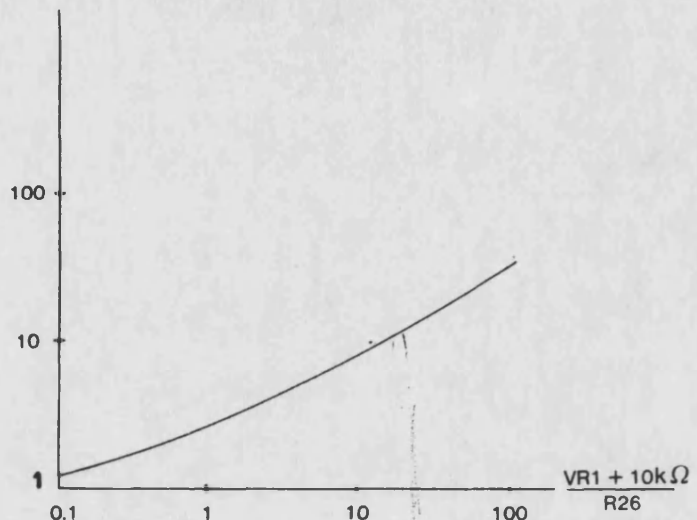


Fig. 8

Once all component values are established and assembled the oscillator frequency range is as shown in Fig. 9. If SW1 is OFF the oscillator runs at base frequency. When SW1 is ON the oscillator builds up (at a rate depending on R22 x C11 time constant) to a frequency determined by VR2 setting.

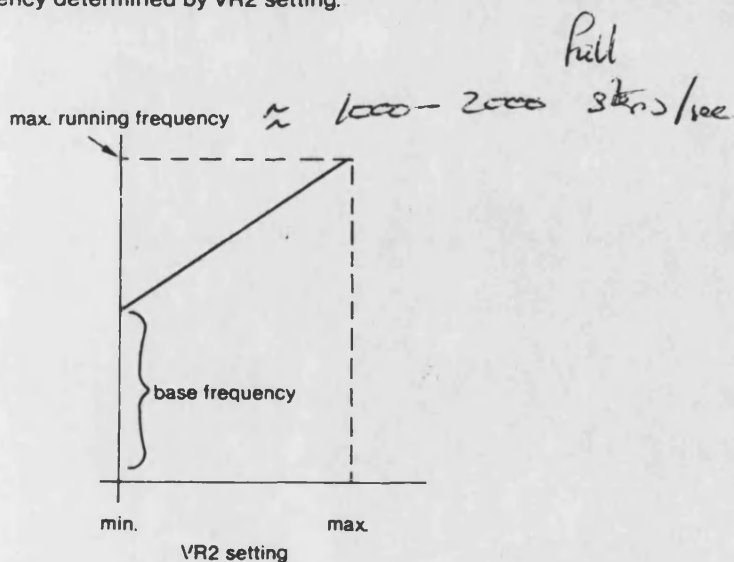


Fig. 9

0.5×10^{-6} sec

? pull in rate for
type 23 step motor


R. S. COMPONENTS LIMITED

13-17 Epworth Street

London EC2P 2HA

Telephone 01-253 1222

DATA SHEET

8 Bit D to A/A to D Converter I.C.

DESCRIPTION

The ZN425E is an 8 bit dual mode analogue to digital/digital to analogue converter. It contains an 8 bit D to A converter using an advance design of R-2R ladder network and an array of precision bipolar switches plus an 8 bit binary counter and a 2.5 volt precision voltage reference all-on a single monolithic chip.

The special design of ladder network results in full 8 bit accuracy using normal diffused resistors.

The use of the on-chip reference voltage is pin optional to retain flexibility. An external fixed or varying reference may therefore be substituted.

By including on the chip an 8 bit binary counter, analogue to digital conversion can be obtained simply by adding an external comparator (531) and clock inhibit gating (7400).

By simply clocking the counter the ZN425E can be used as a self-contained precision staircase ramp generator.

A logic input select switch is incorporated which determines whether the precision switches accept the outputs from the binary counter or external digital inputs depending upon whether the control signal is respectively high or low.

FEATURES

- Dual mode, Digital to Analogue/Analogue to Digital
- On chip precision voltage reference
- Includes 8 bit binary counter
- Will function as precision staircase ramp generator
- TTL and CMOS compatible
- Direct voltage output
- 16 pin D.I.L. encapsulation

PIN CONNECTIONS (TOP VIEW)

Ground	1		16	V_{REF} Output
Logic Select	2		15	V_{REF} Input
Counter Reset	3		14	Analogue Output
Clock	4		13	Bit 1 (M.S.B.)
Bit 8	5		12	Bit 2
Bit 7	6		11	Bit 3
Bit 6	7		10	Bit 4
$+V_{CC}$	8		9	Bit 5

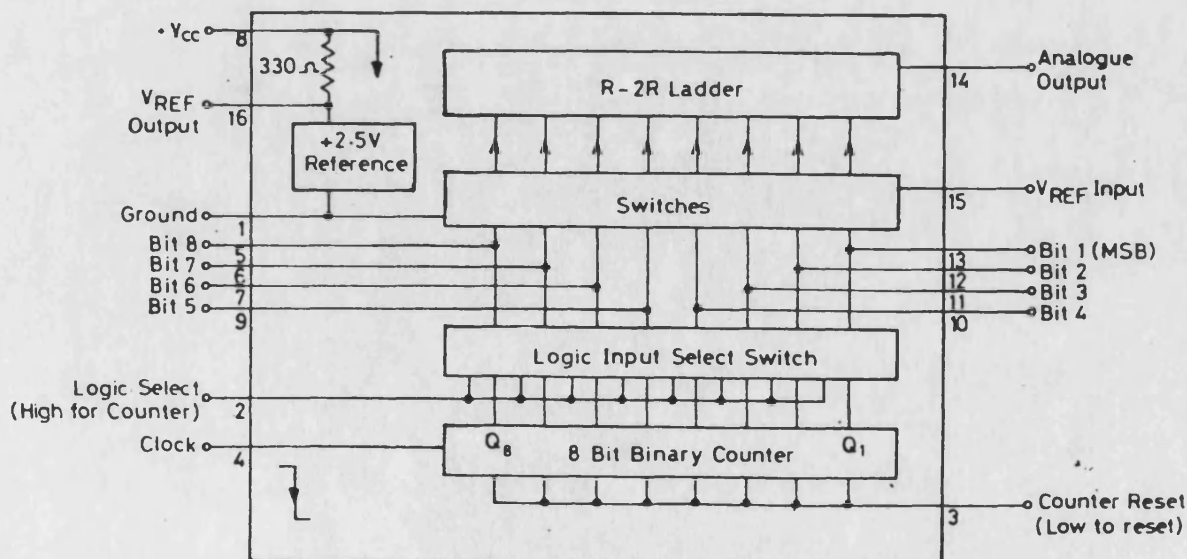


Fig. 1 Block Diagram

ABSOLUTE MAXIMUM RATINGS

Supply voltage V_{CC}	+7.0 volts
Max. voltage, logic and V_{REF} inputs	+5.5 volts
Operating temperature range	0 to +70 °C
Storage temperature range	-55 to +125 °C

CHARACTERISTICS (at $T_{amb} = 25$ °C and $V_{CC} = +5$ volts unless otherwise specified).

Parameter	Symbol	Min.	Typ.	Max.	Units	Conditions
Supply voltage	V_{CC}	4.5	—	5.5	volts	
Supply current	I_s	—	30	40	mA	
High level input voltage	V_{IH}	2.0	—	—	volts	See notes 1 and 2
Low level input voltage	V_{IL}	—	—	0.7	volts	
High level input current	I_{IH}	—	—	10	μA	$V_{CC} = \text{max.}, V_I = 2.4V$
		—	—	100	μA	$V_{CC} = \text{max.}, V_I = 5.5V$
Low level input current	I_{IL}	—	—	-0.68	mA	$V_{CC} = \text{max.}, V_I = 0.3V$
High level output current	I_{OH}	—	—	-40	μA	
Low level output current	I_{OL}	—	—	1.6	mA	
High level output voltage	V_{OH}	2.4	—	—	volts	$V_{CC} = \text{min.}, Q = 1, I_{load} = -40\mu A$
Low level output voltage	V_{OL}	—	—	0.4	volts	$V_{CC} = \text{min.}, Q = 0, I_{load} = 1.6 \text{ mA}$

4-bit D to A Converter

Parameter	Symbol	Min.	Typ.	Max.	Units	Conditions
Linearity error		—	—	± 0.5	L.S.B.	
Settling time		—	1.0	—	μs	1 L.S.B. step
Settling time to 0.5 L.S.B.		—	2.0	—	μs	All bits ON to OFF or OFF to ON
Offset voltage	V_{OS}	—	3.0	—	mV	All bits OFF
OS Temperature coefficient		—	5	—	$\mu V/^{\circ}C$	
S.R. Temperature coefficient		—	3	—	ppm/ $^{\circ}C$	Ext. $V_{REF} = 2.5V$
Linearity Error Temperature coefficient		—	7.5	—	ppm/ $^{\circ}C$	Relative to F.S.R.
Analogue output resistance	R_o	—	10	—	k ohm	
Internal reference voltage		0	—	3.0	volts	

Internal voltage reference

Parameter	Symbol	Min.	Typ.	Max.	Units	Conditions
Output voltage	V_{REF}	—	2.55	—	volts	$I = 7.5 \text{ mA (internal)}$
Output resistance	R_s	—	2	—	ohms	$I = 7.5 \text{ mA (internal)}$
V_{REF} Temperature coefficient		—	40	—	ppm/ $^{\circ}C$	$I = 7.5 \text{ mA (internal)}$

NOTES:

1. The Logic Select pin (2) must be held low when the bit pins (5, 6, 7, 9, 10, 11, 12 and 13) are driven externally.
2. To obtain counter outputs on bit pins the Logic Select pin (2) should be taken to $+V_{CC}$ via a $1 \text{ k}\Omega$ resistor.
3. The internal reference requires a $0.22 \mu F$ stabilising capacitor between pins 1 and 16.

APPLICATIONS

8 bit D to A Converter

The ZN425E gives an analogue voltage output directly from pin 14 therefore the usual current to voltage converting amplifier is not required. The output voltage drift, due to the temperature coefficient of the Analogue Output Resistance R will be less than 0.004% per °C (or 1 L.S.B./100°C) if R is chosen to be $\geq 650k\Omega$.

In order to remove the offset voltage and to calibrate the converter a buffer amplifier is necessary. Fig 2 shows a typical scheme using the internal reference voltage. To minimise temperature drift in this and similar applications the source resistance to the inverting input of the operational amplifier should be approximately $6k\Omega$. The calibration procedure is as follows:

- Set all bits to OFF (low) and adjust R_2 until $V_{out} = 0.000V$.
- Set all bits to ON (high) and adjust R_1 until $V_{out} = \text{Nominal full scale reading} - 1 \text{ L.S.B.}$
- Repeat i. and ii.

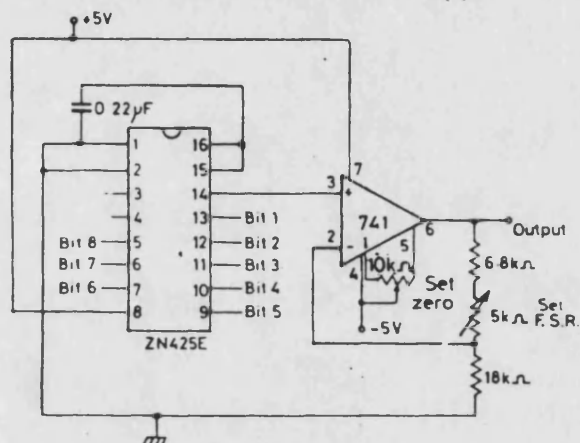
e.g. Set F.S.R. to +3.840 volts -1 L.S.B.

$$= 3.825 \text{ volts}$$

$$\frac{3.84}{256}$$

$$(1 \text{ L.S.B.} = \frac{3.84}{256} = 15.0 \text{ millivolts.})$$

Fig 2. 8 Bit Digital to Analogue Converter



8 bit Analogue to Digital Converter

A counter type analogue to digital converter can be constructed by adding a voltage comparator as in Fig 3. On the negative edge of the CONVERT COMMAND Pulse the counter is set to zero and the STATUS output to logical 1. On the positive edge the counter starts to count up from zero. The analogue output ramps until it equals the analogue voltage applied to the other input of the comparator. At this point, any further clock pulses are inhibited and STATUS goes low to indicate that the output data is valid.

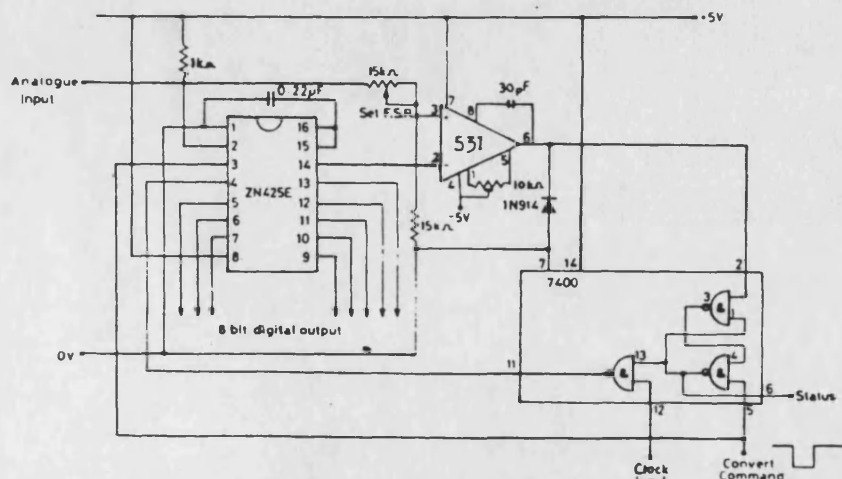
The conversion time depends upon the value of the analogue input and for full scale reading is given by the clock frequency divided into the maximum number of counts.

For example if $F_{clock} = 256K$

$$\text{conversion (for F.S.R.)} = \frac{2^8 \text{ seconds}}{256,000}$$

$$= 1 \text{ millisecond}$$

Fig 3. 8 Bit Analogue to Digital Converter



Precision Staircase Ramp Generator

The inclusion of an 8 bit binary counter on the chip gives the ZN425E a useful staircase ramp generator function. The circuit, Fig 4 uses the same buffer stage as the D to A converter. The calibration procedure is also the same. Holding pin 2 low will set all bits to ON and if RESET is taken low with pin 2 high all the bits are turned OFF. If the end voltages of the ramp are not required to be set accurately then the buffer stage could be omitted and the voltage ramp will appear directly at pin 14. (N.B. Resetting of the output may take place at any point of the waveform by taking pin 3 [reset] low.)

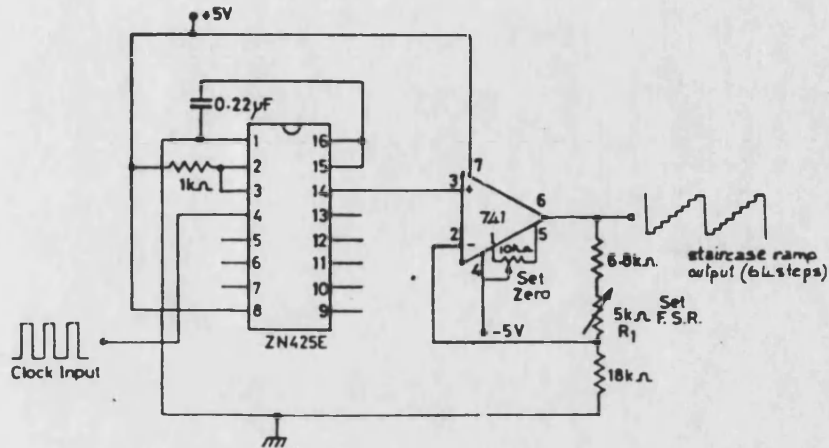


Fig 4. Precision Staircase Ramp Generator.

**APPENDIX 3 Non contacting displacement transducer
 details**

NON CONTACTING DISPLACEMENT TRANSDUCERS

These transducers utilise the inductive technique to measure the relative distance between a 'sender' unit and a 'target', and are unaffected by water. These systems allow a resolution as fine as 0.2 nanometers, although 0.12 to 0.25 micrometers is more common.

The inductive principle is as follows. An AC current flowing in a coil causes the field of one winding to add to the field of the next winding. The fields pulsate, in turn generating an AC electromagnetic field surrounding the coil. Placing the coil a nominal distance from a target induces a current flow (the 'eddy current') on the surface and within the target. The induced current produces a second magnetic field that opposes and thus reduces the intensity of the original field with resultant changes in the impedance of the exciting coil. These changes are analysed by signal conditioning electronics to give information about the target.

In the transducers considered here, produced by Kaman Measuring Systems, the sensing coil is a leg of a balanced wheatstone bridge network. As the target moves toward the coil, the current flow increases in the target and losses within the bridge network increase, and visa versa as the target moves away. The bridge

network responds to these unbalanced conditions, and the output of the bridge is amplified, demodulated and presented as a linear analogue signal directly proportional to the targets position, which in turn can be displayed on a digital readout.

The measurement range of the system is a direct function of the surface diameter of the sensor. As the sensor diameter increases so does the measurement range, but, the resolution decreases. Thus, the selection of an appropriate sensor configuration is dependent on space considerations, and a compromise between range and resolution.

For the application of measuring the radial expansion/contraction of soil samples within the large triaxial cell during the cyclic testing, four sensors were considered a minimum requirement in order to assess and average the sample movement. These would be arranged with two sensors placed diametrically opposite one another on the 1/3rd points of the sample. A four channel system with sensors with a measurement range of 15mm and a resolution of 0.0015 mm was proposed. However as stated within the main text, cost constraints ruled out this option.

**APPENDIX 4 Calibration Data for Strain Rings and
Calibration Certificates for Proprietary Transducers**

Calibration data for the Strain Rings.

Includes;

1. Stiffness test results.
2. XY plots of RESPONSE vs INDUCED DISPLACEMENT for each 1/2 bridge for each strain ring, utilising DIALOG software and screen dump.
3. Yt plot of RESPONSE vs TIME for each 1/2 bridge for each strain ring. Utilising DIALOG software and screen dump.
4. Calculation sheet for the conversion factors.

The raw numerical data is not presented as the graphical output represents the better format indicating the response in a more appropriate manner, as discussed in section 8.1 in the main text.

STRAIN RING STIFFNESS TEST RESULTS

These tests consisted of hanging small weights from the strain rings and measuring the resulting deformation. Measurements were recorded whilst the rings were being loaded and unloaded in order to ensure the response was similar for extension and contraction.

Weight [N]	Absolute Deformation reading on scale rule. [mm]	
	Loading	Unloading
hanger only (50g)	405	404
+ 0.5	402	401
+ 0.5	399	399
+ 0.5	396	398
+ 0.5	396	

self weight		410
+ hanger (50g)	408	408
+ 0.5	406	406
+ 0.5	404	406
+ 0.5	402	402
+ 0.5	400	

self weight	410	410
+ hanger (50g)	409	408
+ 0.5	407	406/7
+ 0.5	405	404/5
+ 0.5	403	402
+ 0.5	400/1	

Average stiffness value = 2 mm/N, 0.5 N/mm

Strain Ring Conversion Factor Calculation Sheet

As mentioned in section 8.1, each 1/2 arrangement on each strain ring is identified by the Ring no. and the monitoring channel no.

Data taken from XY plots and numerical listings as appropriate.

RING No.1

Channel no. 11

Induced Displacement [mm] = 4.98164

Response [ADU units] = 778

Conversion Factor = 6.403×10^{-3} mm/unit

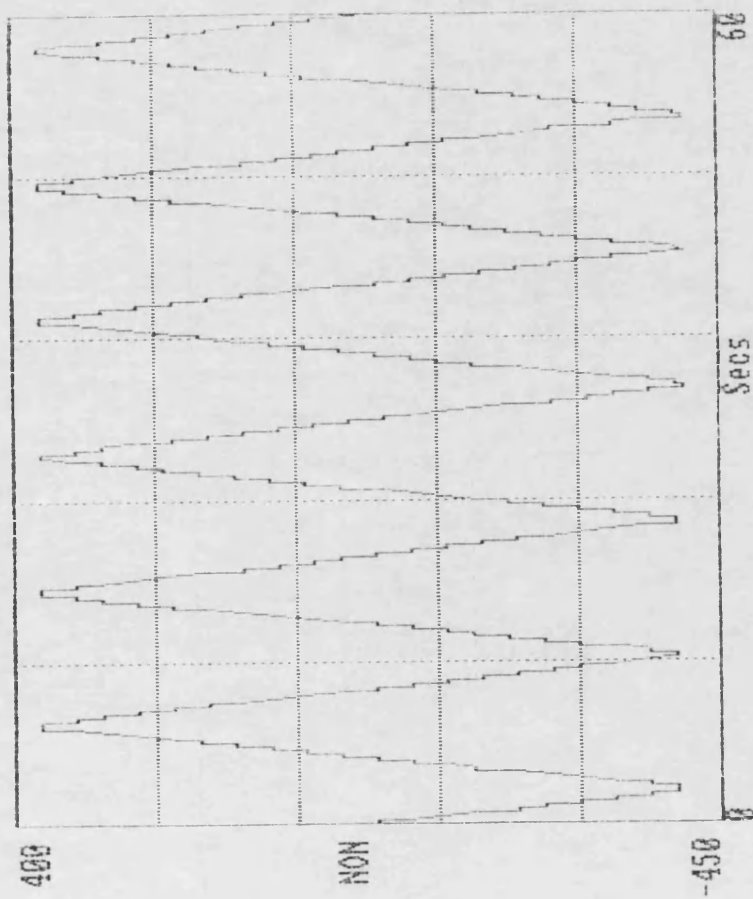
Channel no.15

Induced Displacement [mm] = 4.98164

Response [ADU units] = 662

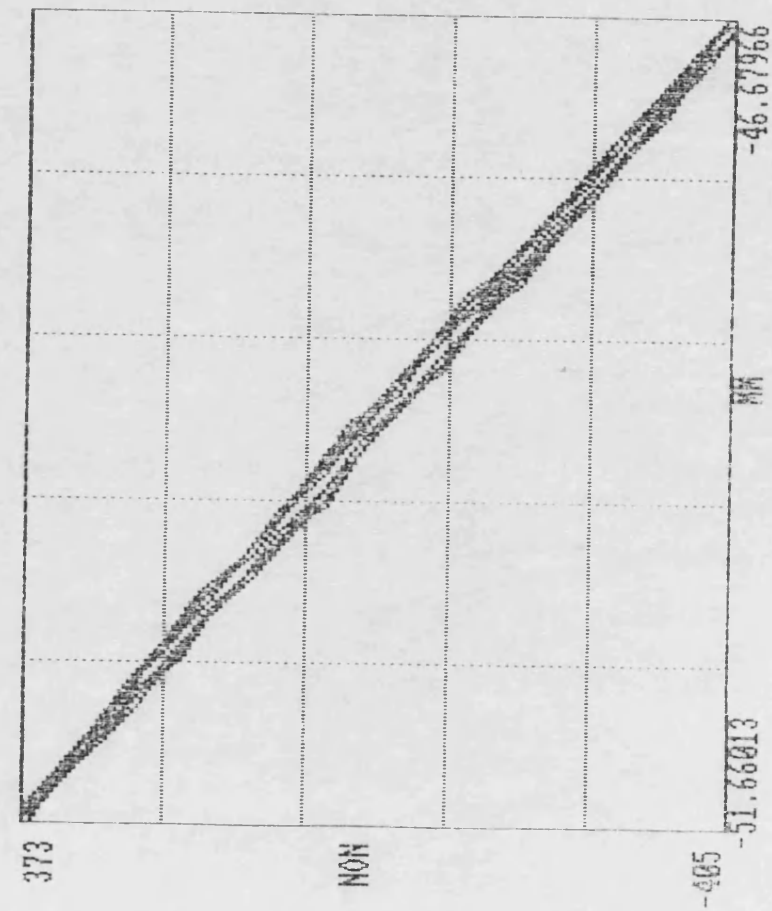
Conversion Factor = 7.525×10^{-3} mm/unit

STRING v Time



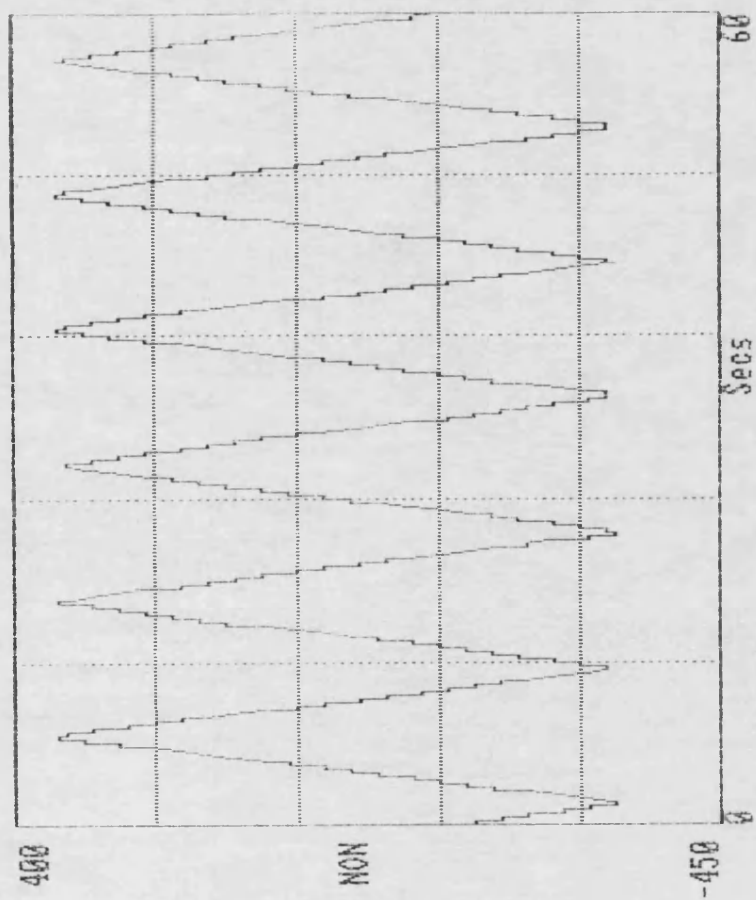
RING 1/11

STRING v DISP



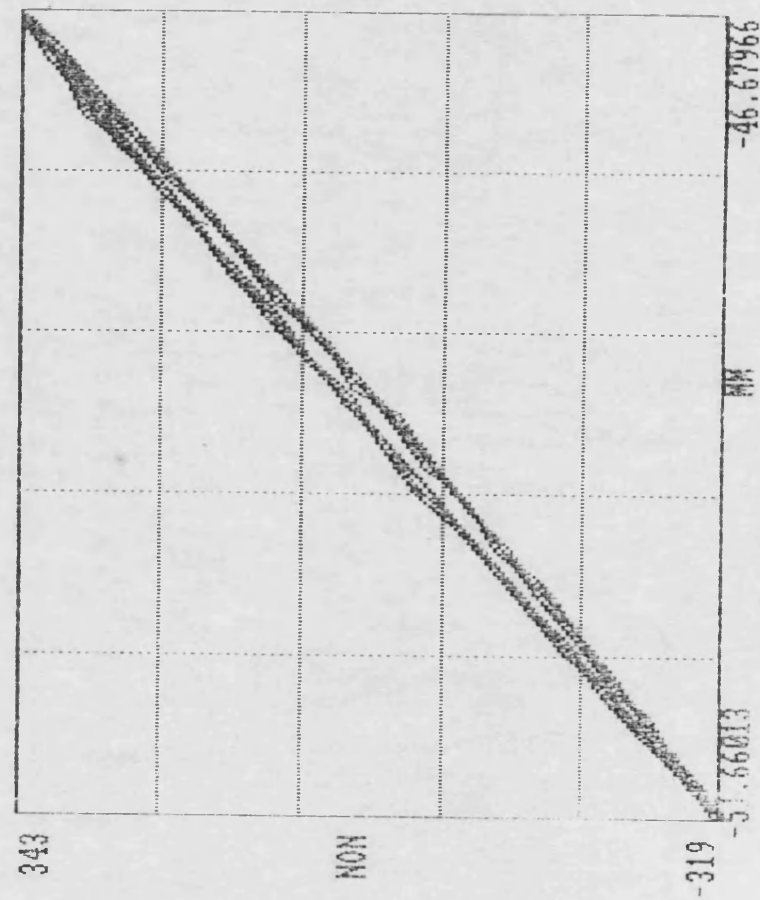
RING 1/11

STRING v Time



RING 1/15

STRING v DISP



RING 1/15

Strain Ring Conversion Factor Calculation Sheet

As mentioned in section 8.1, each 1/2 arrangement on each strain ring is identified by the Ring no. and the monitoring channel no.

Data taken from XY plots and numerical listings as appropriate.

RING No.2

Channel no.14

Induced Displacement [mm] = 4.98047

Response [ADU units] = 350

Conversion Factor = 0.0142299 mm/unit

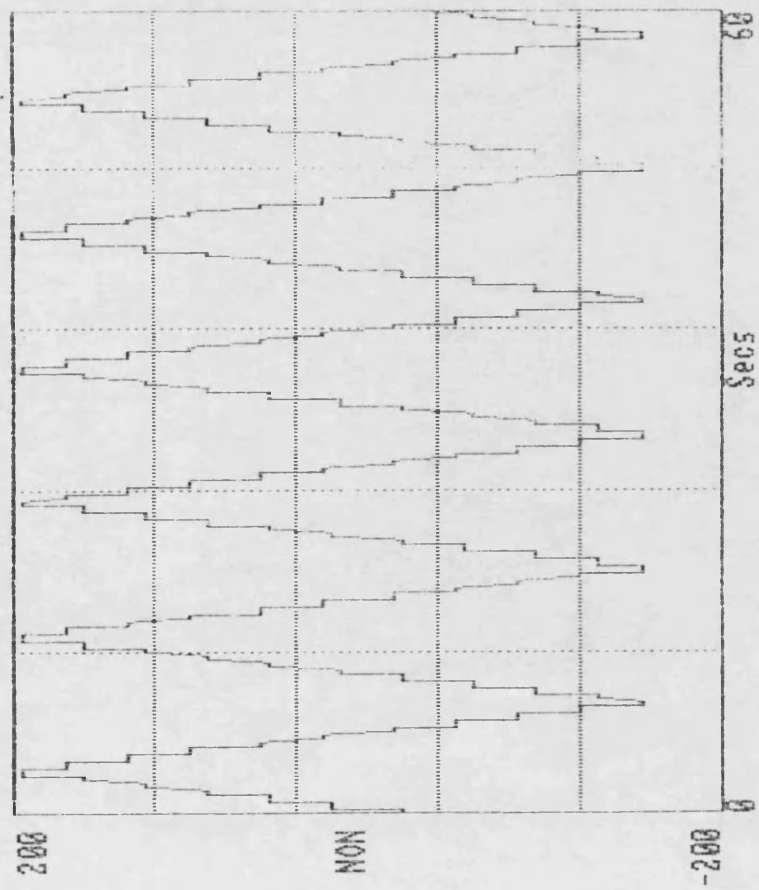
Channel no.13

Induced Displacement [mm] = 4.98047

Response [ADU units] = 330

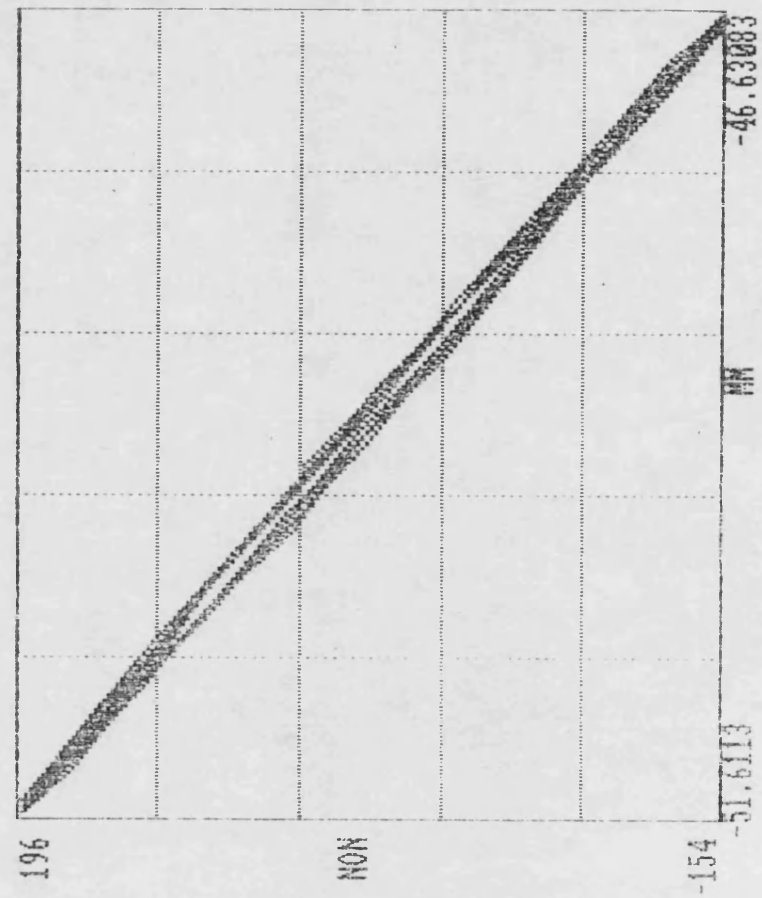
Conversion Factor = 0.01509 mm/unit

STRING v Time



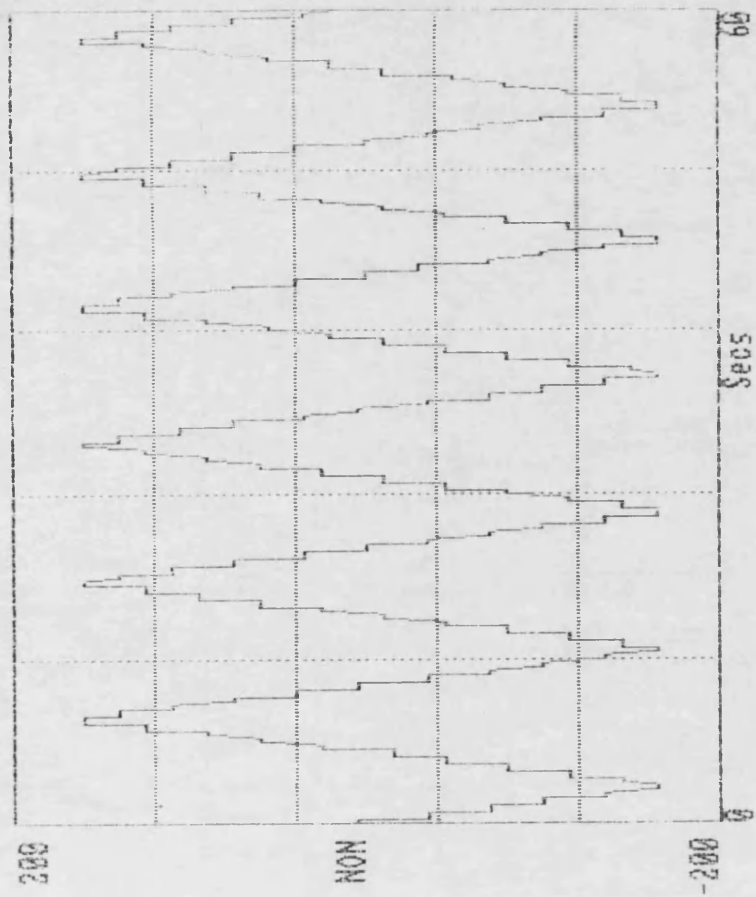
RING 2/14

STRING v DISP



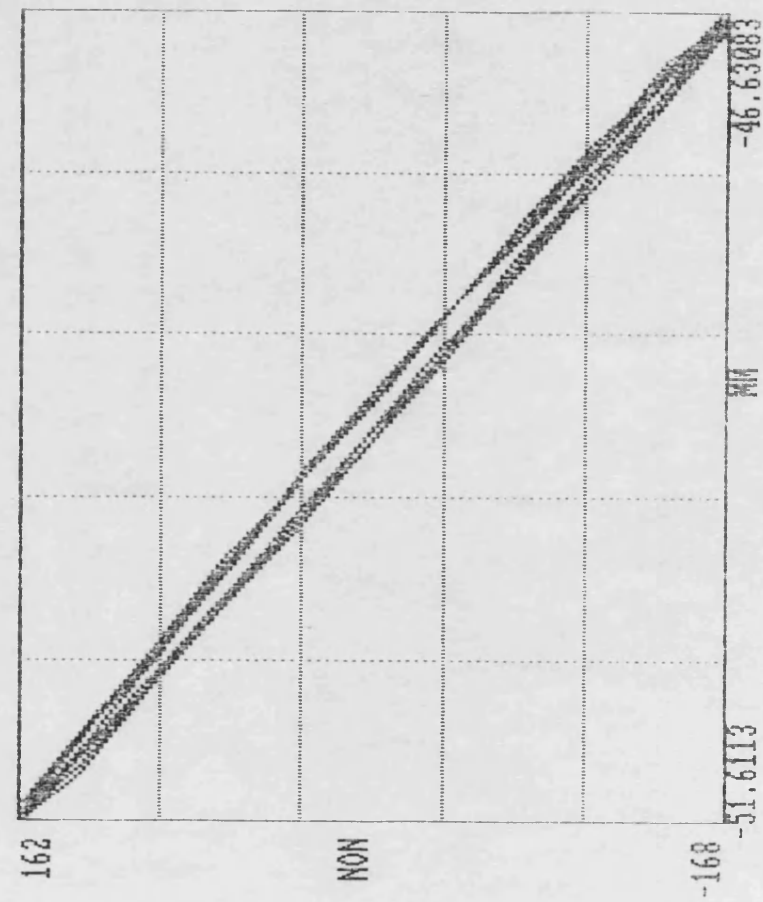
RING 2/14

STRING v Time



RING 2/13

STRING v DISP



RING 2/13

Strain Ring Conversion Factor Calculation Sheet

As mentioned in section 8.1, each 1/2 arrangement on each strain ring is identified by the Ring no. and the monitoring channel no.

Data taken from XY plots and numerical listings as appropriate.

RING No.3

Channel no.16

Induced Displacement [mm] = 5.27344

Response [ADU units] = 649

Conversion Factor = 8.125×10^{-3} mm/unit

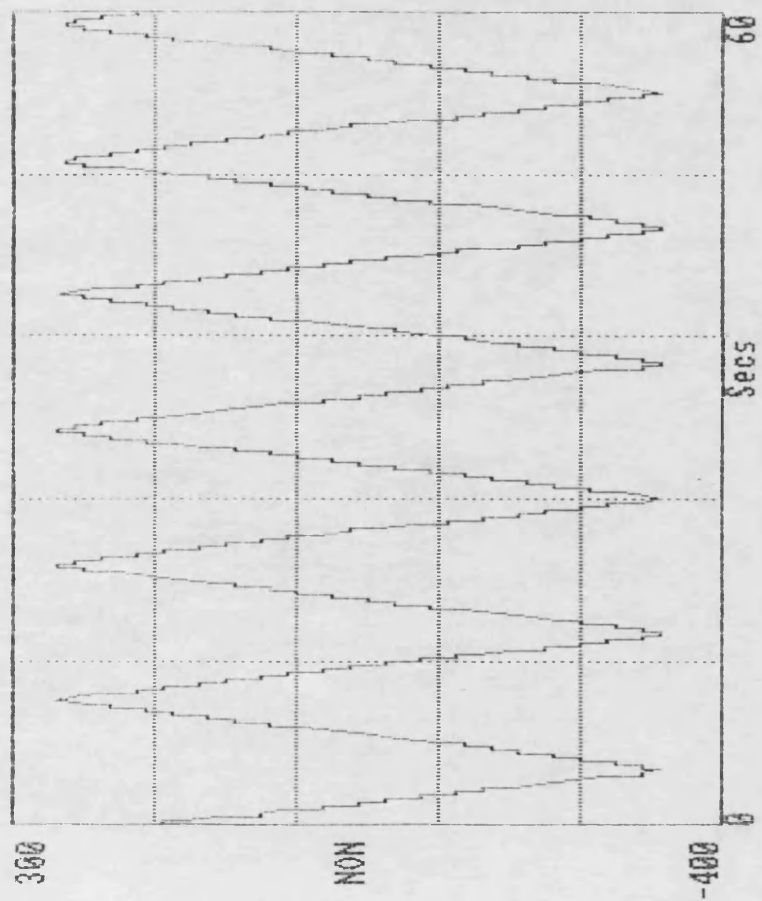
Channel no.12

Induced Displacement [mm] = 5.27344

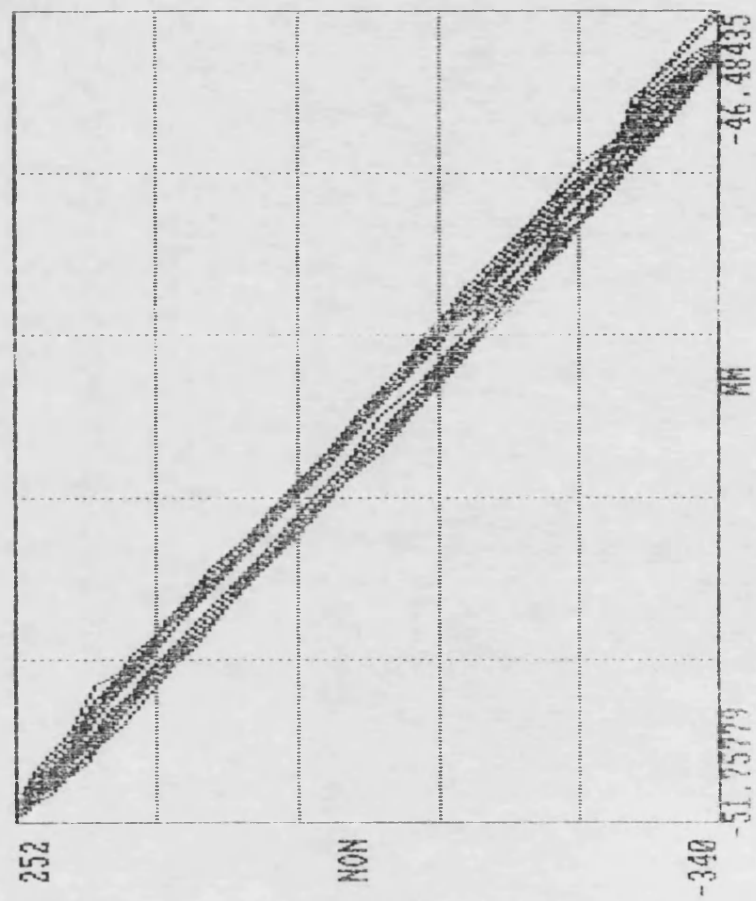
Response [ADU units] = 592

Conversion Factor = 8.9078×10^{-3} mm/unit

STRING v Time



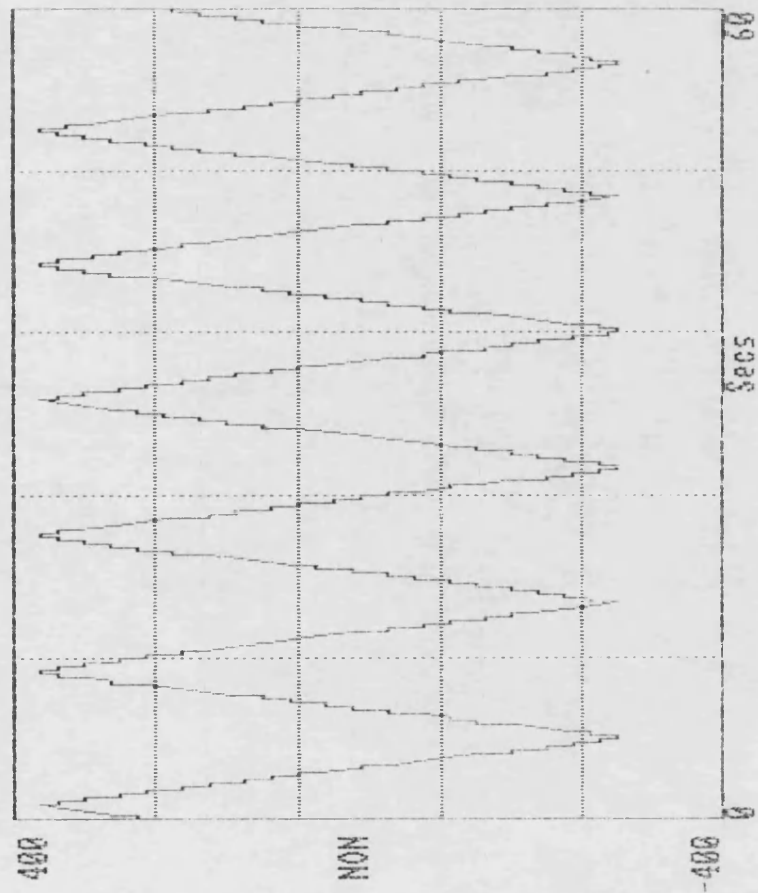
STRING v DISP



RING 3/12

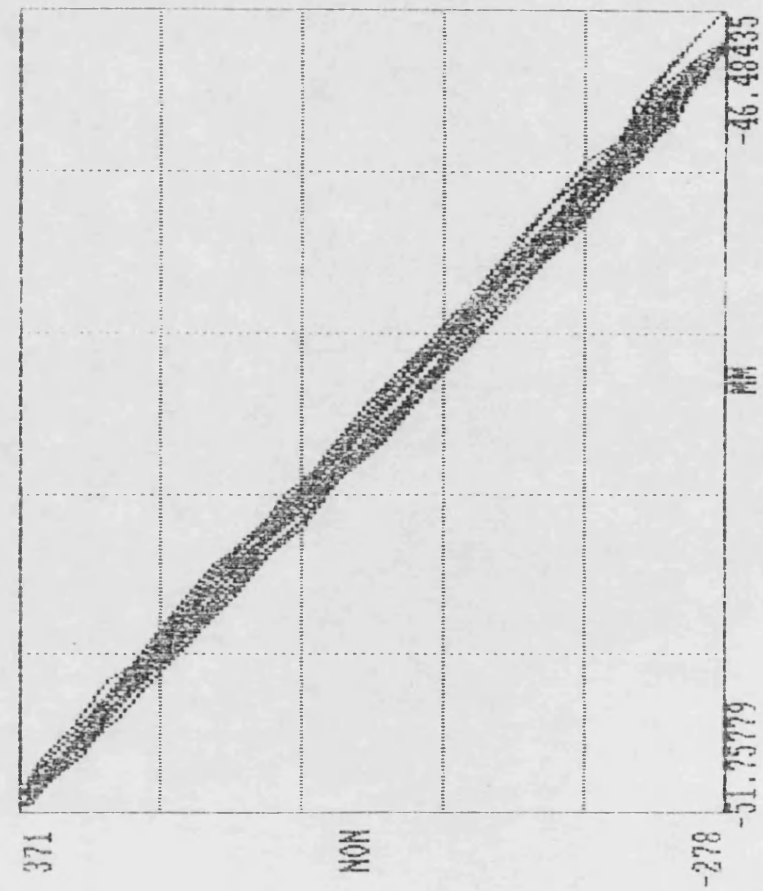
RING 3/12

STRING v Time



RING 3/16

STRING v DISP



RING 3/16

Strain Ring Conversion Factor Calculation Sheet

As mentioned in section 8.1, each 1/2 arrangement on each strain ring is identified by the Ring no. and the monitoring channel no.

Data taken from XY plots and numerical listings as appropriate.

RING No.4

Channel no.10

Induced Displacement [mm] = 4.98047

Response [ADU units] = 160

Conversion Factor = 0.03113 mm/unit

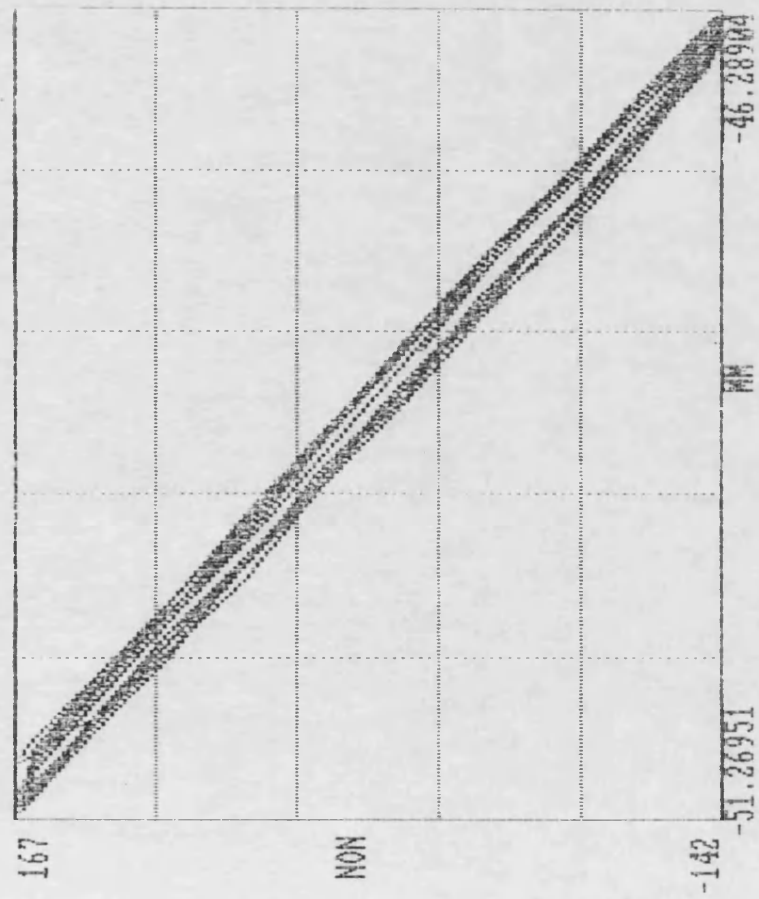
Channel no.9

Induced Displacement [mm] = 4.98047

Response [ADU units] = 309

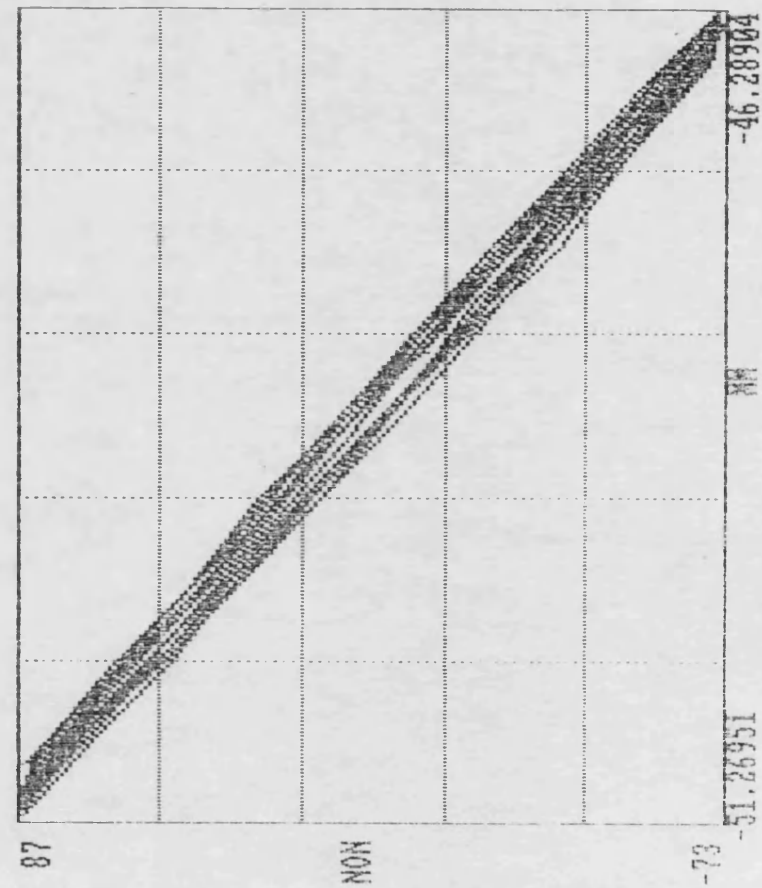
Conversion Factor = 0.016118 mm/unit

STRING v DISP



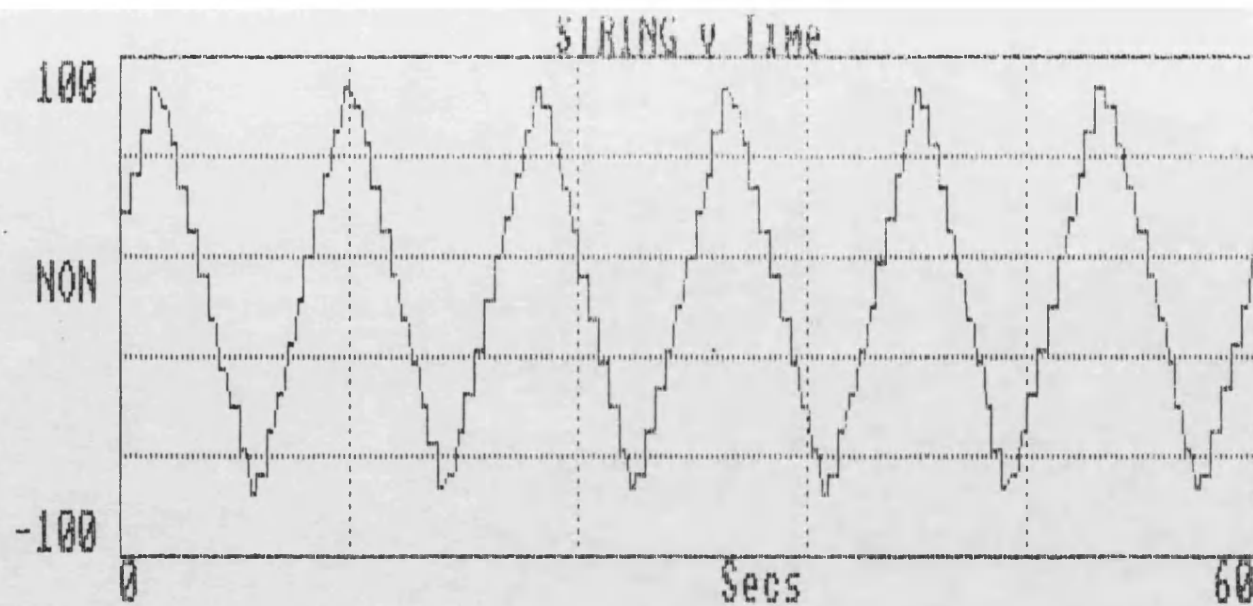
RING 4/9

STRING v DISP

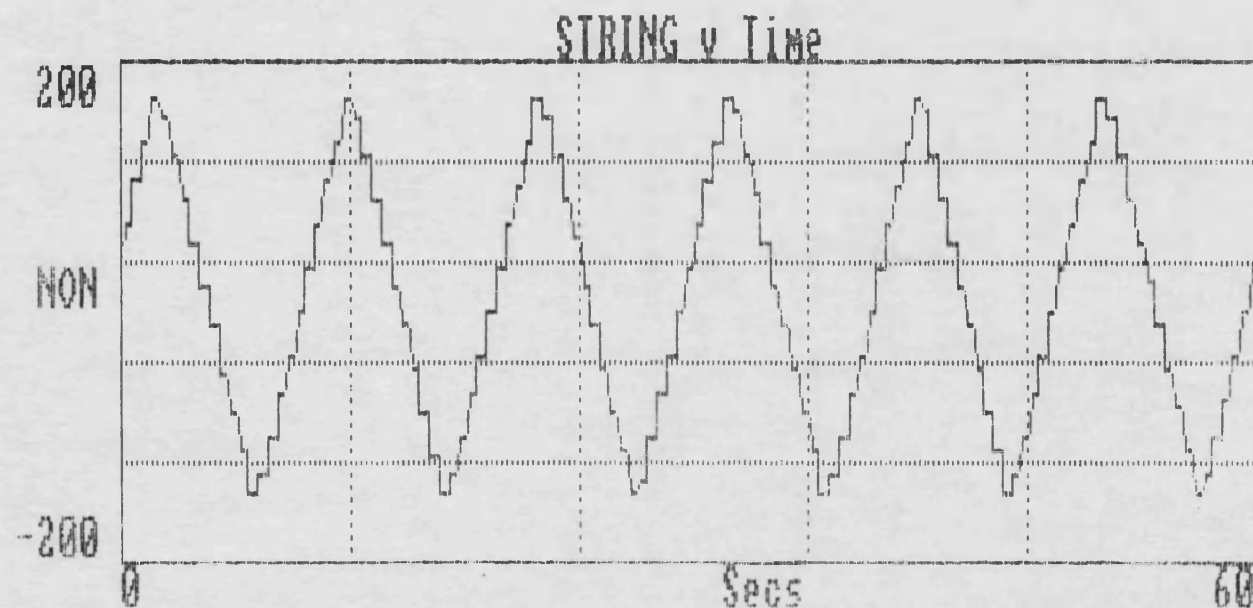


RING 4/10

RING 4/10



RING 4/9



Strain Ring Conversion Factor Calculation Sheet

As mentioned in section 8.1, each 1/2 arrangement on each strain ring is identified by the Ring no. and the monitoring channel no.

Data taken from XY plots and numerical listings as appropriate.

RING No.5

Channel no.16

Induced Displacement [mm] = 5.07812

Response [ADU units] = 727

Conversion Factor = 6.985×10^{-3} mm/unit

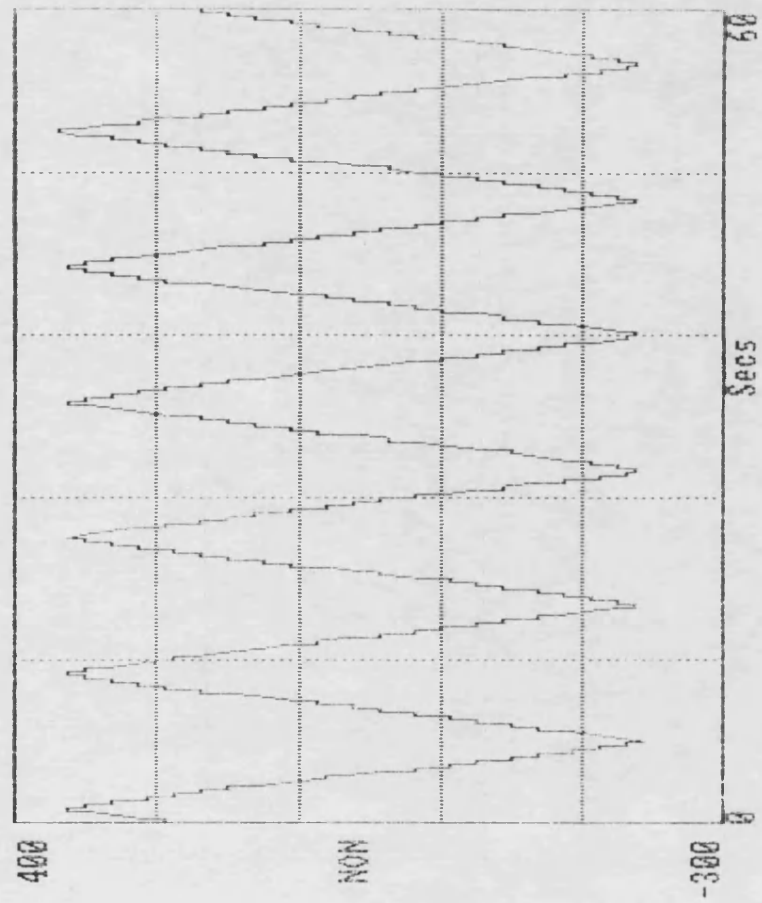
Channel no.12

Induced Displacement [mm] = 5.07812

Response [ADU units] = 571

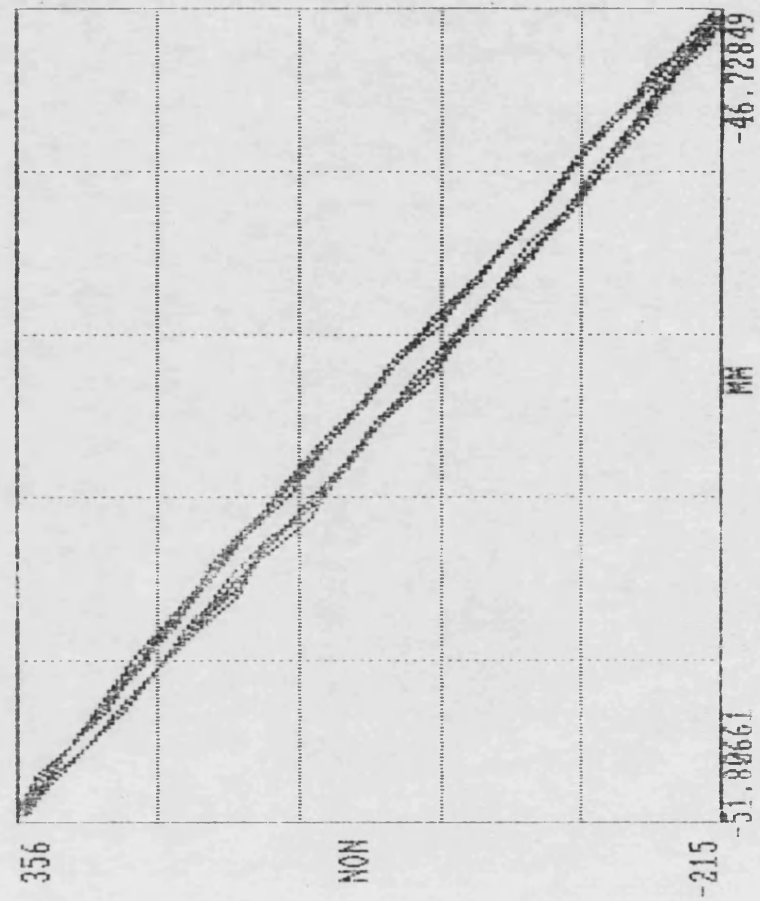
Conversion Factor = 8.893×10^{-3} mm/unit

STRING v Time



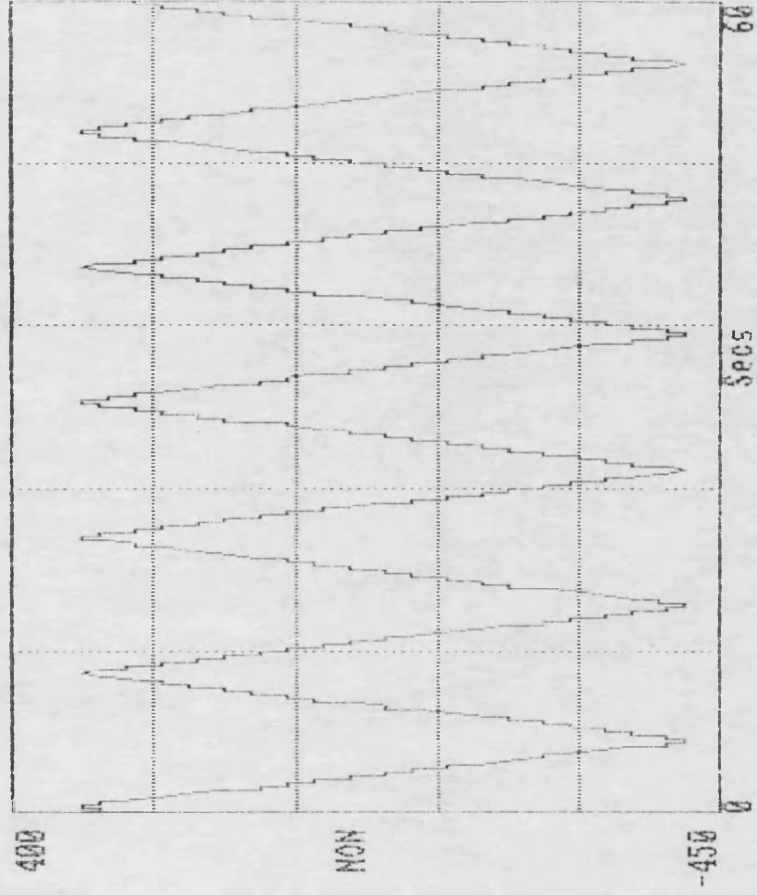
RING 5/12

STRING v DISP



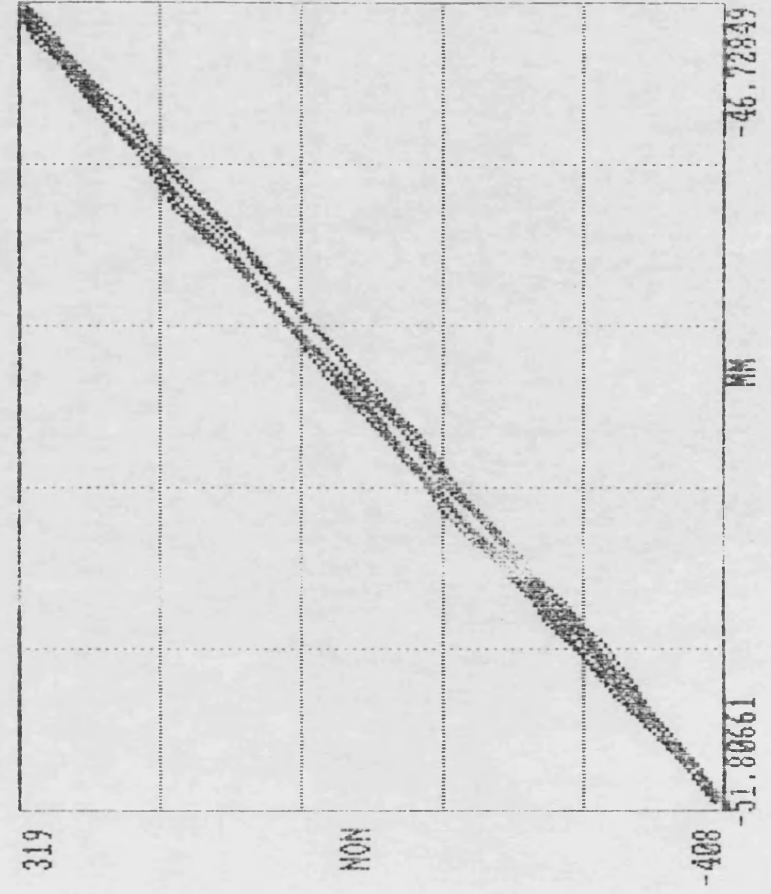
RING 5/12

STRING v Time



RING 5/16

STRING v DISP



RING 5/16

Proprietary Transducer Calibration Certificates

1. Strain gauge displacement transducers.

2. Non indicating load cell.

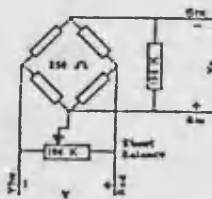
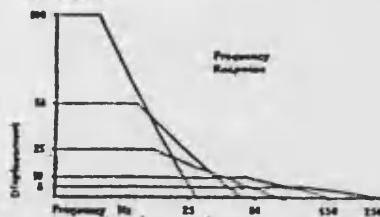
3. DARTEC actuator load cells.

WSM DISPLACEMENT TRANSDUCER

CALIBRATION REPORT

DATE 12.12.88

TRANSDUCER TYPE HLS 50
SERIAL NO. 4020
INPUT RED/YELLOW
OUTPUT BLUE/GREEN



EXCITATION VOLTS: AC OR DC
1, 5 - 10 CONSTANT
ES max: pulsed
BRIDGE 350 ohms NOMINAL
SHUNT 100k ohms NOMINAL

CALIBRATION DATA

GAUGE FACTOR SET 2.0 TEST VOLTS 5.21 VDC TEMPERATURE 20 °C
F S SPINDLE DISPLACEMENT 51.55 mm NON LINEARITY 0.05 % FS
STRAIN SENSITIVITY 143 x 10⁻⁶ /mm VOLTS SENSITIVITY 3.7 mV/V

OPERATING NOTES

CLEAN OPERATING SPINDLE WITH RESIDUE-FREE SOLVENT/INHIBITOR TYPE CLEANER 111-TRICHLOROETHANE OR SIMILAR.

FOR REPETITIVE CYCLIC OPERATION LIGHTLY LUBRICATE ONLY WITH GENERAL PURPOSE SYNTHETIC INSTRUMENT OIL WINDSOR L EN51 (MIL-L-6983A) OR EQUIVALENT, DO NOT OVER LUBRICATE.

USE SOFT JAW CLAMPS TO GRIP SPINDLE WHEN CHANGING ANVIL.

CLAMP TRANSDUCER ON BODY DIAMETER ONLY.

Welwyn Strain Measurement

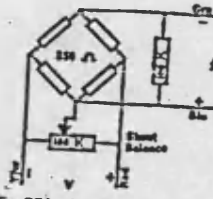
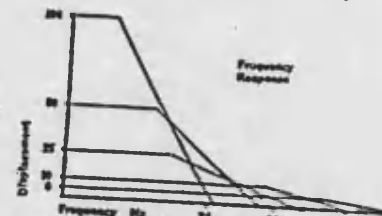
Armstrong Road, Basingstoke, Hants. RG24 0QA
Telephone: Basingstoke (0256) 462131 Telex: 858520
Fax: (0256) 471441 A Crystalate Company

WSM DISPLACEMENT TRANSDUCER

CALIBRATION REPORT

DATE 23.8.88

TRANSDUCER TYPE HS 100
SERIAL NO. 5020
INPUT RED/YELLOW
OUTPUT BLUE/GREEN



EXCITATION VOLTS: AC OR DC
1, 5 - 10 CONSTANT
ES max: pulsed
BRIDGE 350 ohms NOMINAL
SHUNT 100k ohms NOMINAL

CALIBRATION DATA

GAUGE FACTOR SET 2.0 TEST VOLTS 5.21 VDC TEMPERATURE 20 °C
F S SPINDLE DISPLACEMENT 102.1 mm NON LINEARITY 0.2 % FS
STRAIN SENSITIVITY 105 x 10⁻⁶ /mm VOLTS SENSITIVITY 4 mV/V

OPERATING NOTES

CLEAN OPERATING SPINDLE WITH RESIDUE-FREE SOLVENT/INHIBITOR TYPE CLEANER 111-TRICHLOROETHANE OR SIMILAR.

FOR REPETITIVE CYCLIC OPERATION LIGHTLY LUBRICATE ONLY WITH GENERAL PURPOSE SYNTHETIC INSTRUMENT OIL WINDSOR L EN51 (MIL-L-6983A) OR EQUIVALENT, DO NOT OVER LUBRICATE.

USE SOFT JAW CLAMPS TO GRIP SPINDLE WHEN CHANGING ANVIL.

CLAMP TRANSDUCER ON BODY DIAMETER ONLY.

Welwyn Strain Measurement

Armstrong Road, Basingstoke, Hants. RG24 0QA
Telephone: Basingstoke (0256) 462131 Telex: 858520
Fax: (0256) 471441 A Crystalate Company

CHASE MEASUREMENTS



CHASE MEASUREMENTS LTD

THEATRE STREET
WARWICK CV34 4DP

Telephone: (0926) 490088

FAX 0926 410167

TRANSDUCER & SYSTEMS ENGINEERS

CALIBRATION CERTIFICATE

CHASE MEASUREMENTS LTD
THEATRE STREET
WARWICK
CV34 4DP

Telephone: (0926) 490088
Fax: (0926) 410167

Your O.N.

362578

Our O.N.

CM284

Date

28/10/88

Model

CCA 10 Tonne

Accuracy

0.2%

Serial No

CCA 110 134

Cable Colour Coding

Output 2.00 mV/V

Red + Excitation

Blue - Excitation

Green - Output

Yellow + Output

This load cell is made to the specifications laid down in our technical information.

CERTIFICATE OF CALIBRATION

ISSUED BY THE CALIBRATION FACILITIES OF DARTEC LIMITED
DATE OF ISSUE 19th May, 1992 SERIAL NUMBER 01345/A



CALIBRATION
No. 0135

DARTEC

DARTEC LIMITED

BALDS LANE · LYE · STOURBRIDGE
WEST MIDLANDS · ENGLAND · DY9 8SH

Tel (0384) 892363
Tlx 336902 DARTEC
Fax (0384) 892772

PAGE 1 OF 1 PAGES

APPROVED SIGNATORY

Head of Laboratory:- R. E. M. Ball

Issued to:- UNIVERSITY OF BATH

Location:- Department of Architecture and Building Engineering, Bath

Description:- A force measuring system which, when used, with an actuator, within a test rig, can be regarded as a range of a testing machine. The load cell has a maximum capacity of 50kN and the associated indicator displays 0 - 50kN.

Identification:- Load Cell - 87167, Indicator - 87167/E2

Date of Calibration:- 28th April, 1992

The above force measuring system has been verified to the British Standard 1610:Part 1:1985, using the equipment listed below, which complied with Clause 3.3 of the standard. To facilitate the verification, the load cell and verification equipment were mounted in series on the actuator piston and the assembly positioned within a rig. The indicated force method was used to effect the verification and the maximum reading facility was not used.

20kN:- Load Cell, No:- 46148, Indicator No:- 46143, Manufactured by Interface Inc., NPL Reference No:- 08C011/91085/LL105/17, dated 4th April, 1991

100kN:- Load Cell No:- K011171. Indicator No:- K011171, Manufactured by Baldwin Lima Hamilton Corporation, NPL Reference No:- 08C011/90279/LL125/3, dated 26th October, 1990.

The temperature at the time of verification was 20°C
The machine indicator resolution was 0.001kN

The force measuring system complied with the requirements of the British Standard for the following grading:-

Grade 1.0 Tension 50 kN down to 1kN
Grade 1.0 Compression 50 kN down to 1kN

The uncertainties are for a confidence probability of not less than 95%

This certificate is issued in accordance with the conditions of accreditation granted by the National Measurement Accreditation Service, which has assessed the measurement capability of the laboratory and its traceability to recognised national standards and to the units of measurement realised at the corresponding national standards laboratory. Copyright of this certificate is owned jointly by the Crown and the issuing laboratory and may not be reproduced other than in full except with the prior written approval of the Head of NAMAS and the issuing laboratory.

CERTIFICATE OF CALIBRATION

ISSUED BY THE CALIBRATION FACILITIES OF DARTEC LIMITED
DATE OF ISSUE 19th May, 1992 SERIAL NUMBER 01344



CALIBRATION
No. 0135

DARTEC

DARTEC LIMITED

BALDS LANE · LYE · STOURBRIDGE
WEST MIDLANDS · ENGLAND · DY9 8SH

Tel (0384) 892363
Tlx 336902 DARTEC
Fax (0384) 892772

PAGE 1 OF 1 PAGES

APPROVED SIGNATORY

Head of Laboratory:- R. E. M. Ball

Issued to:- UNIVERSITY OF BATH

Location:- School of Architecture and Building Engineering, Bath

Description and Identification:- A universal, materials, testing machine of 100kN maximum capacity

Machine Type:- DARTEC M1000/RK

Serial Number:- 88343/B

Date of Calibration:- 28th April, 1992

The above testing machine has been verified to the British Standard 1610:1985, using the equipment listed below, which complied with Clause 3.3 of the standard.

The indicated force method was used to effect the verification and the maximum reading facility was not used.

20kN:- Load Cell, No:- 46148, Indicator No:- 46143, Manufactured by Interface Inc., NPL Reference No:- 08C011/91085/LL105/17, dated 4th April, 1991

100kN:- Load Cell No:- K011171. Indicator No:- K011171, Manufactured by Baldwin Lima Hamilton Corporation, NPL Reference No:- 08C011/90279/LL125/3, dated 26th October, 1990.

The temperature at the time of verification was 20°C
The machine indicator resolution was 0.001kN

The machine complied with the requirements of the British Standard for the following grading:-

Grade 1.0 Tension 100 kN down to 1 kN

Grade 1.0 Compression 100 kN down to 1 kN

The uncertainties are for a confidence probability of not less than 95%

This certificate is issued in accordance with the conditions of accreditation granted by the National Measurement Accreditation Service, which has assessed the measurement capability of the laboratory and its traceability to recognised national standards and to the units of measurement realised at the corresponding national standards laboratory. Copyright of this certificate is owned jointly by the Crown and the issuing laboratory and may not be reproduced other than in full except with the prior written approval of the Head of NAMAS and the issuing laboratory.

APPENDIX 5 Test Data

Contents:

1. 1st stage 'calibration' tests.
2. 2nd stage 'calibration' tests.
3. Large granular material tests in 254mm cell.
4. Preliminary dynamic data. Verification purposes only.

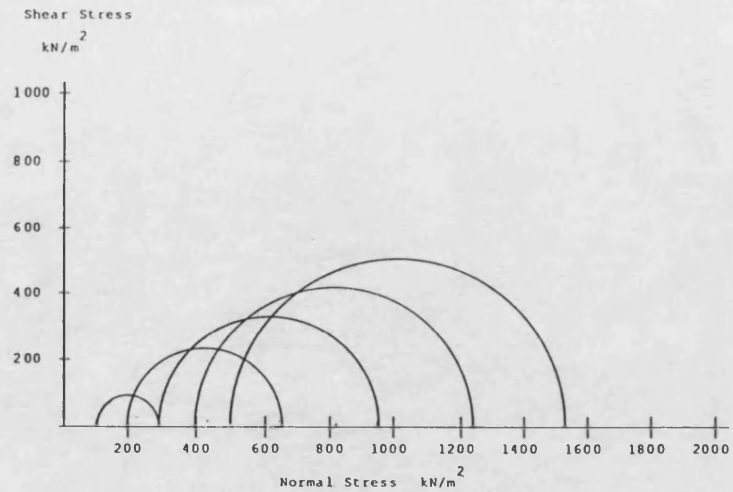
Test Results for the 1st stage static calibration tests,
as detailed in section 8.3.4 and 8.3.5. Sample sizes -
38mm, 100mm, 254mm.

Includes;

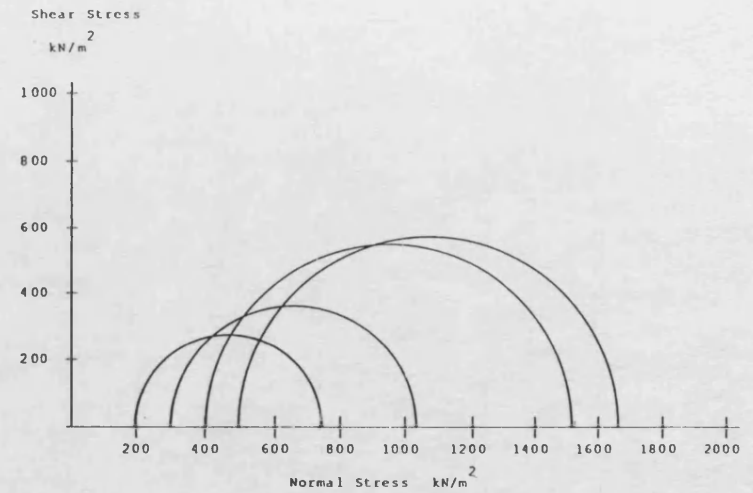
1. Summary table.
2. Multi stage plots of raw data using area correction
method.
3. Derived Mohr circle plots.

SUMMARY TABLE FOR INITIAL CALIBRATION TESTS

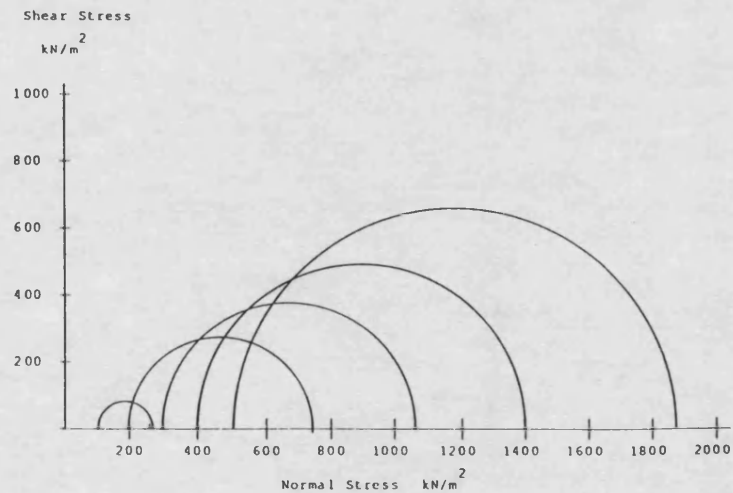
Test No.	Sample Dia. mm	material	mass kg	σ_3 kN/m ²	σ_1 kN/m ²	ϕ_{calc} (°)	ϕ_{plot} (°)
1	38	'fine' 52/100 grade	0.112	100 200 300 400 500	292 641 941 1241 1528	29 32 34 31 30	31
2	38	'medium' Holm sand	0.123	100 200 300 400 500	360 740 1074 1397 1695	34.4 35 34.3 33.7 33	34
3	38	'course' 14/25 grade	0.120	200 300 400 500	725 1058 1340 1674	34.6 33.9 32.7 32.7	33
4	100	'fine' 52/100 grade	2.193	200 300 400 500	841 1225 1590 1918	38 37.3 36.7 36	37
5	100	'medium' Holm sand	2.318	200 300 400 500	1026 1443 1788 2093	42.3 41 39.4 37.9	38
6	100	'course' 14/25 grade	2.248	100 200 300 400 500	437 881 1219 1583 1895	35 39 37.2 36.6 35.6	37
7	254	'fine' 52/100 grade	27.497	100 200 300	445 891 1267	39.3 39.3 38.1	38
8	254	'medium' Holm sand	29.962	100 200 300	534 1000 1385	43.2 41.8 40.1	40
9	254	'course' 14/25 grade	28.90	100 200 300	494 891 1454	41.6 39.3 41.1	40



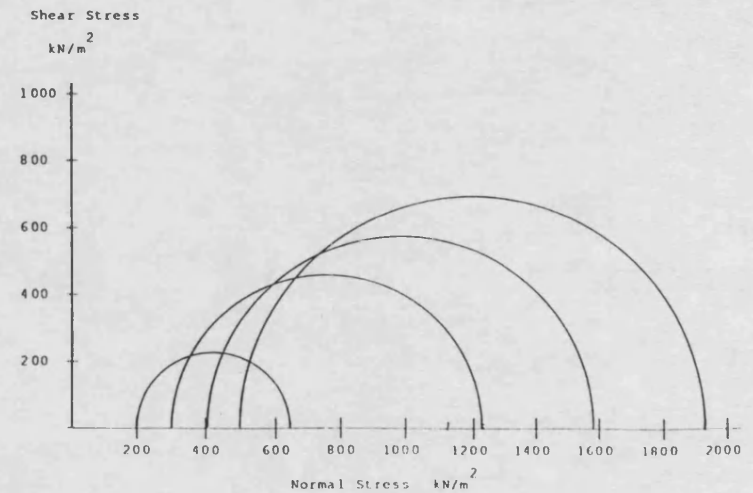
Mohrs circle plot for Test No. 1



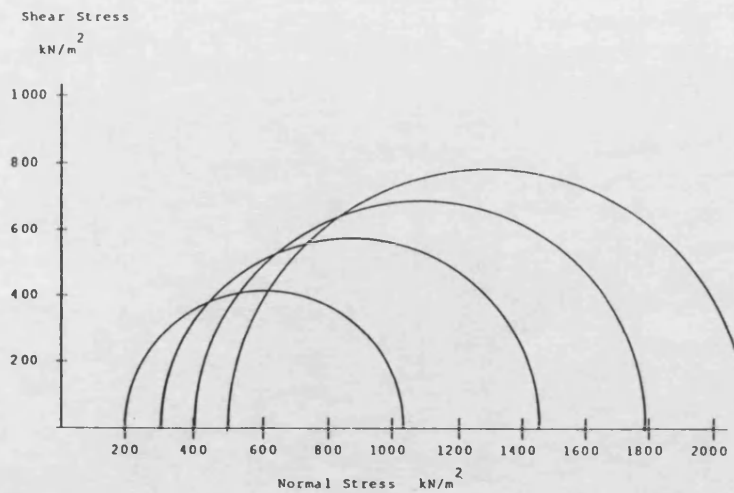
Mohrs circle plot for Test No. 3



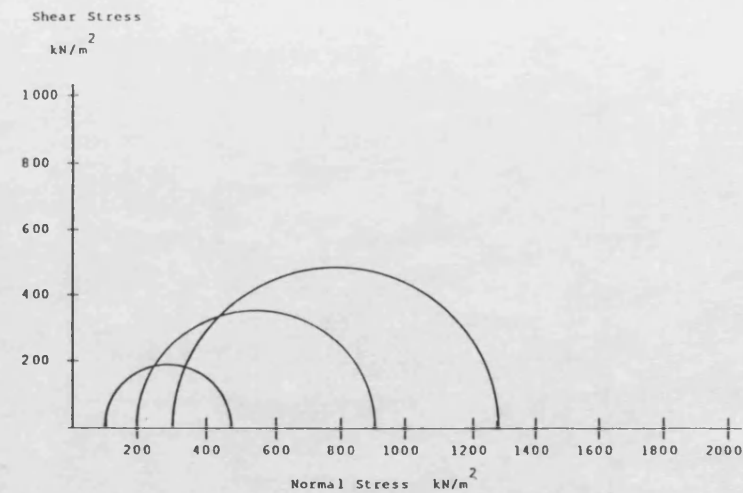
Mohrs circle plot for Test No. 2



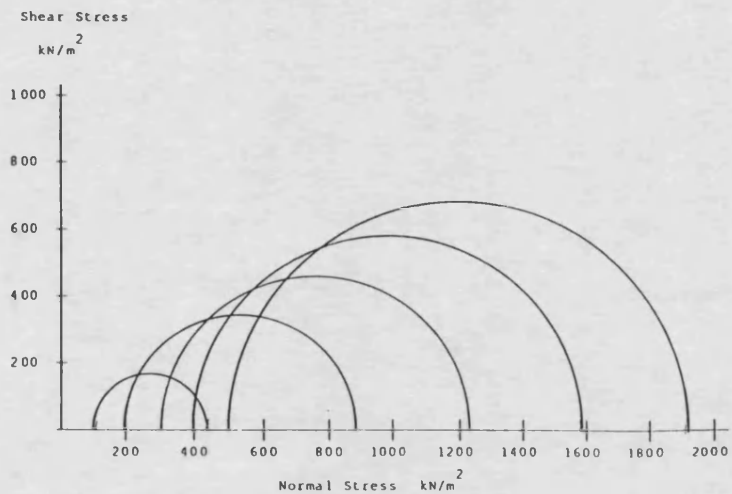
Mohrs circle plot for Test No. 4



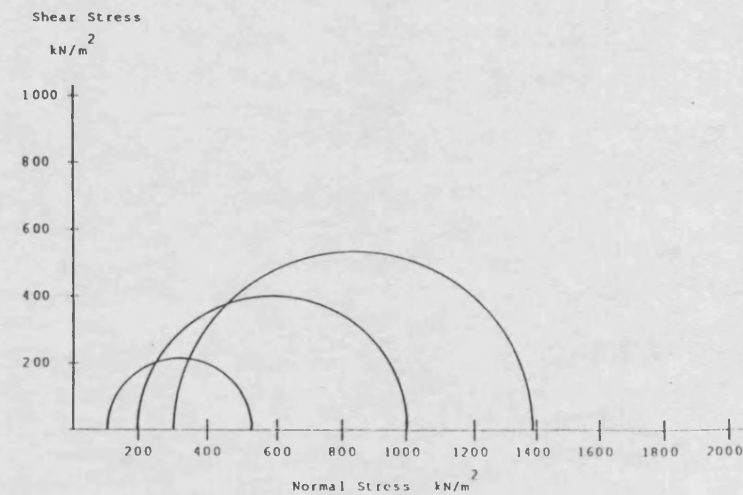
Mohrs circle plot for Test No. **5**



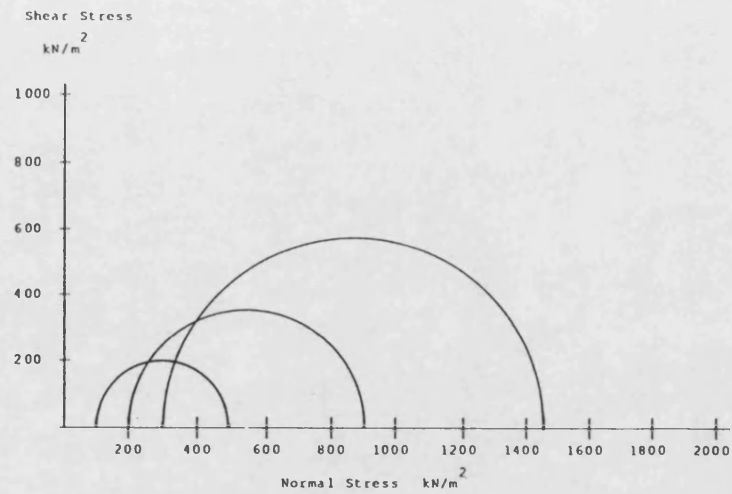
Mohrs circle plot for Test No. **7**



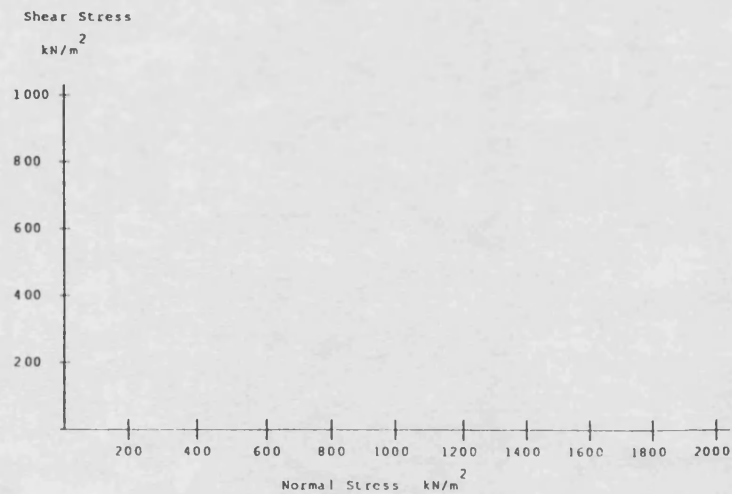
Mohrs circle plot for Test No. **6**



Mohrs circle plot for Test No. **8**



Mohrs circle plot for Test No. 9



Mohrs circle plot for Test No.

CONVERSION FACTORS FOR AREA CORRECTION METHOD

The area correction procedure, for plotting the readings during load application, provides an indication of the **stress** response of the sample, thus yielding information allowing decisions on when to increase the cell pressure in the multi-stage test method.

Assuming constant volume, knowledge of the rate of axial compression, initial sample dimensions, and load recording device, area correction paper can be produced taking account of the increase in area throughout the test. This can be understood by appreciating that as the test proceeds, the sample compresses and therefore the cross sectional area increases. As a consequence, as the test continues an increased load is necessary to maintain the same stress accross the sample.

For the data in this report, the stress is read horizontally accross each of the plots. The sloping lines indicate the load response. In order to convert the vertical axis to stress the following conversion factors are utilised.

38mm samples : initial sample area = $1.134 \times 10^{-3} \text{ m}^2$
proving ring conversion = 5.890 N/div
(2000 lb ring, ref. no. 3561)

Conversion factor = 5.194 kN/m^2

100mm samples: initial sample area = $7.854 \times 10^{-3} \text{ m}^2$
proving ring conversion = 25.96 N/div
(6000 lb ring, graphical conversion)

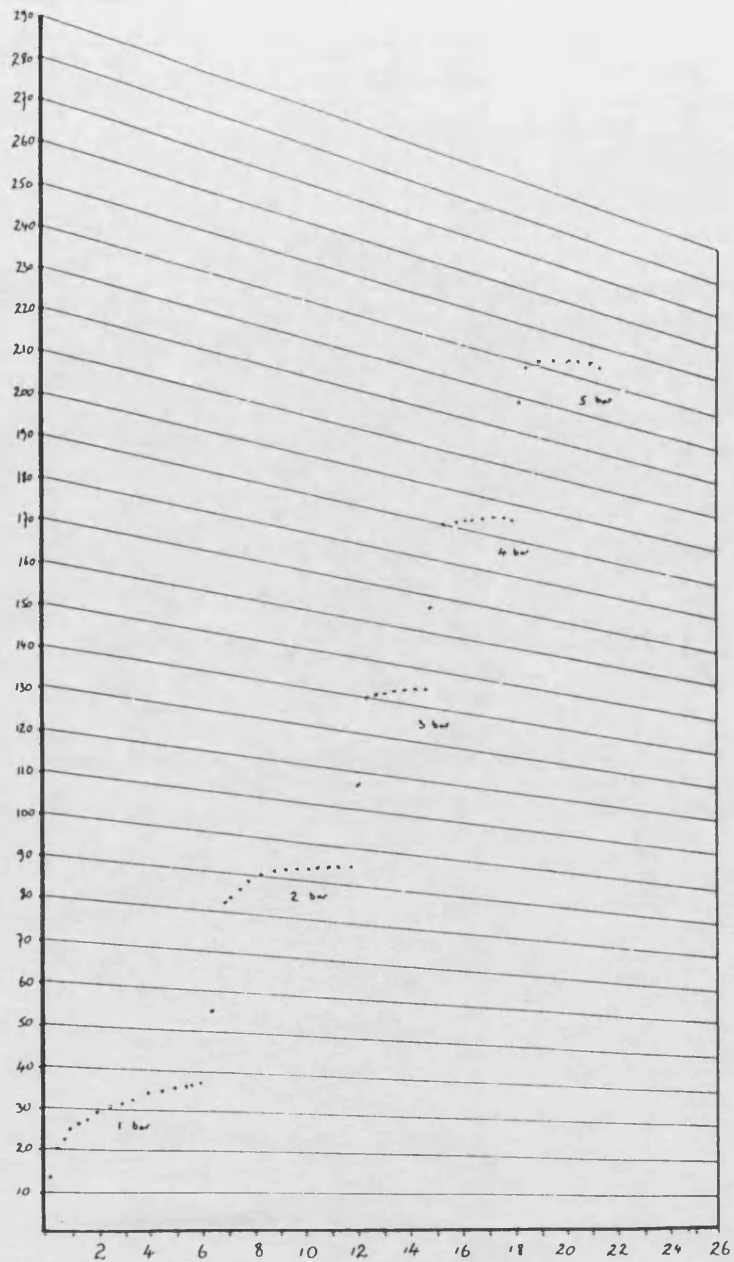
conversion factor = 3.305 kN/m^2

254mm samples: initial sample area = 0.05067 m^2
load recorded directly in kN.

conversion factor = 19.735 kN/m^2

PROVING RING DIV.

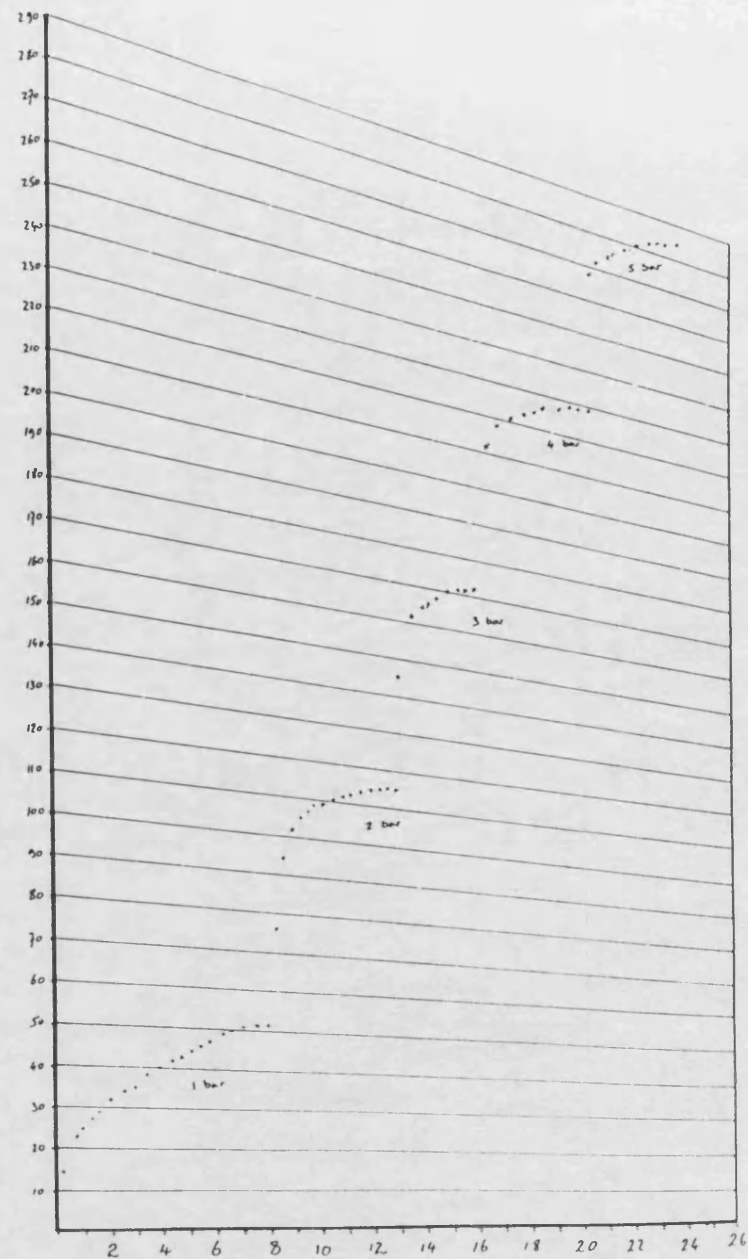
TEST 1



TIME (MIN.)

PROVING RING DIV.

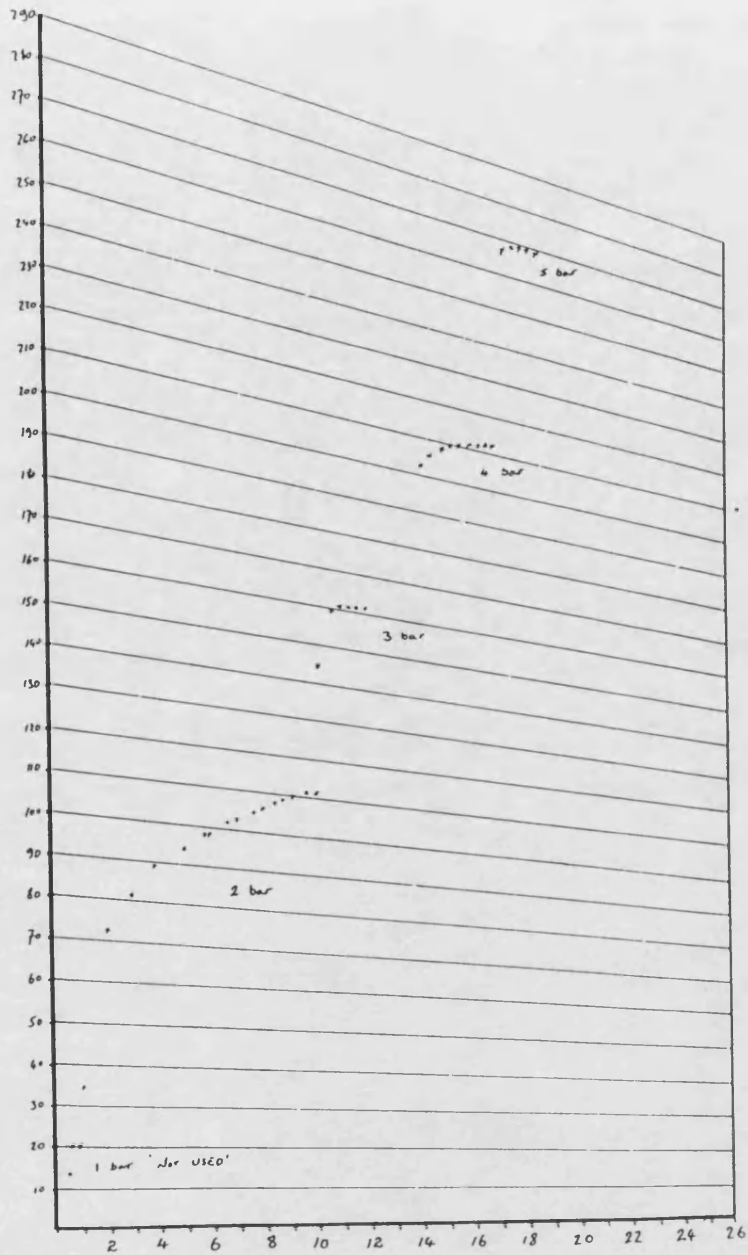
TEST 2



TIME (MIN.)

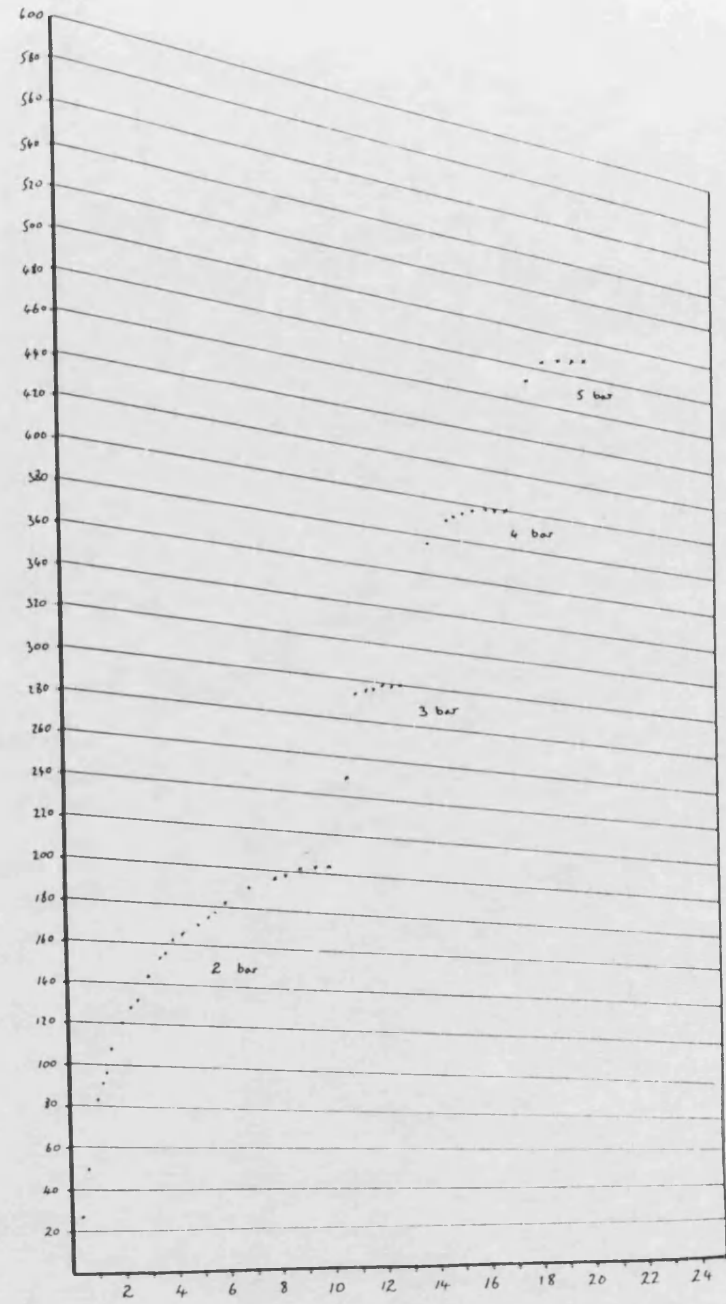
TEST 3

PROVING LINE DIV.

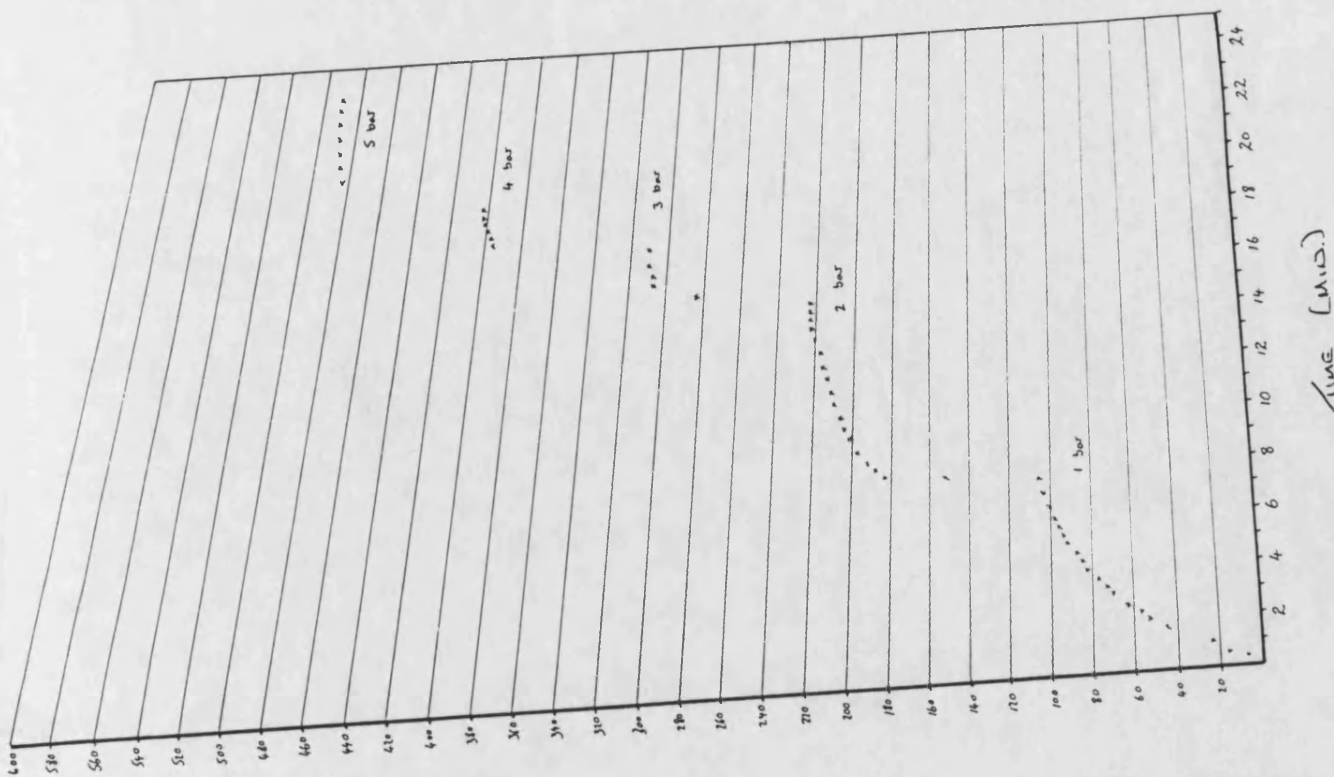


TIME (MIN.)

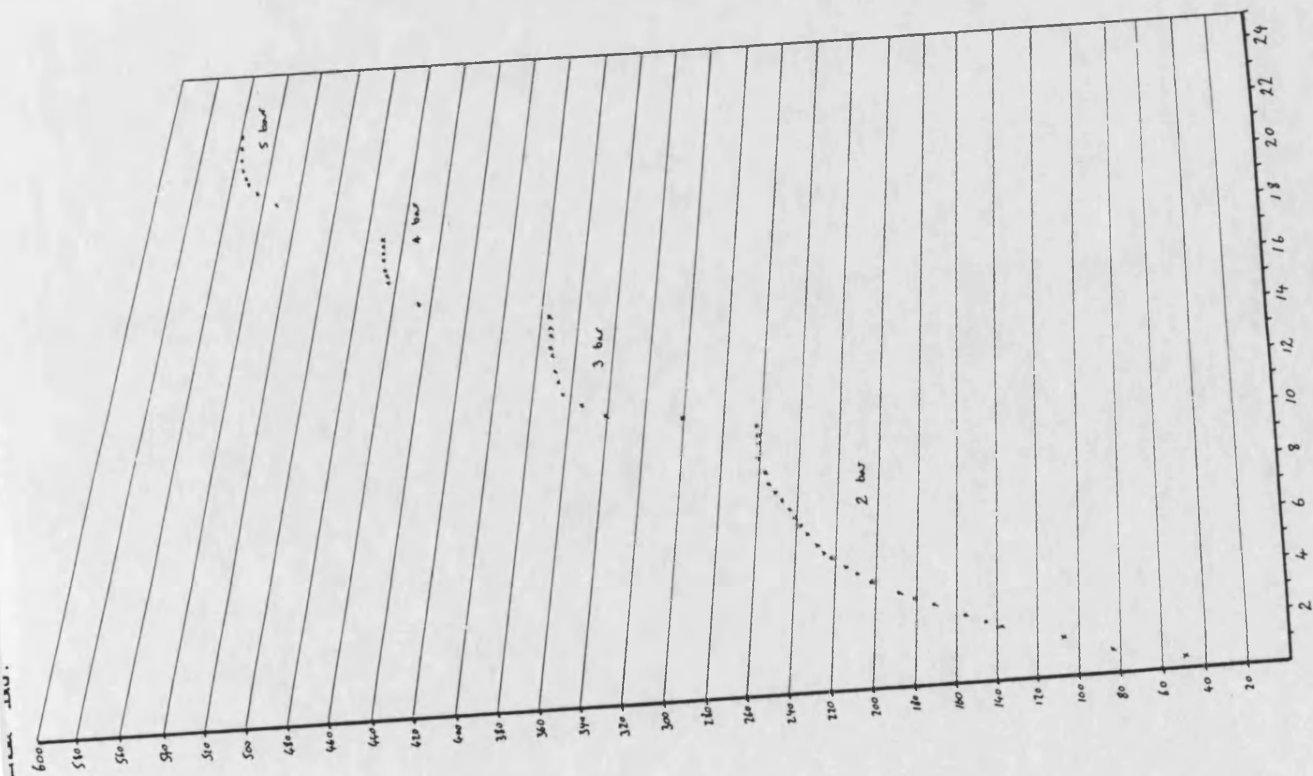
PROVING LINE DIV.

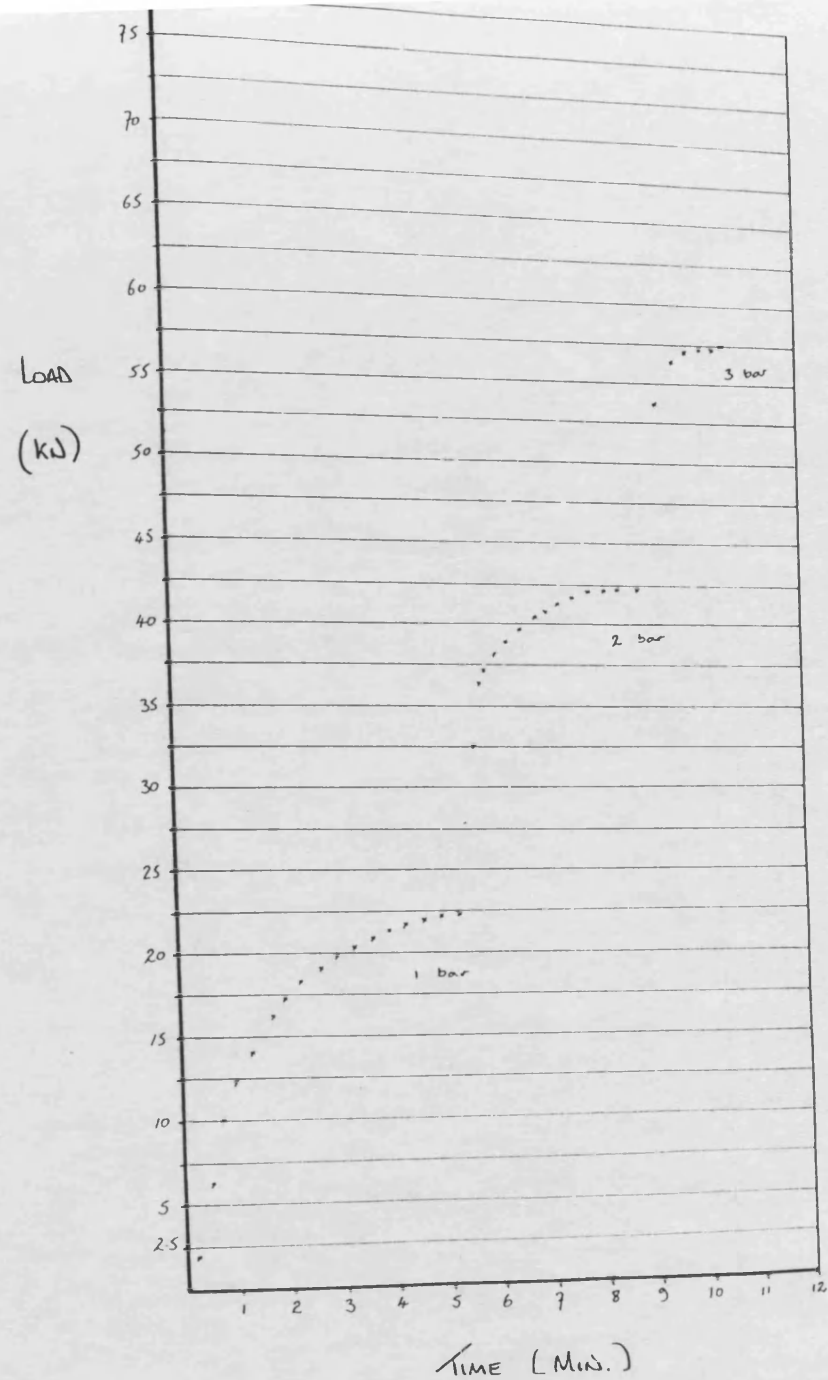
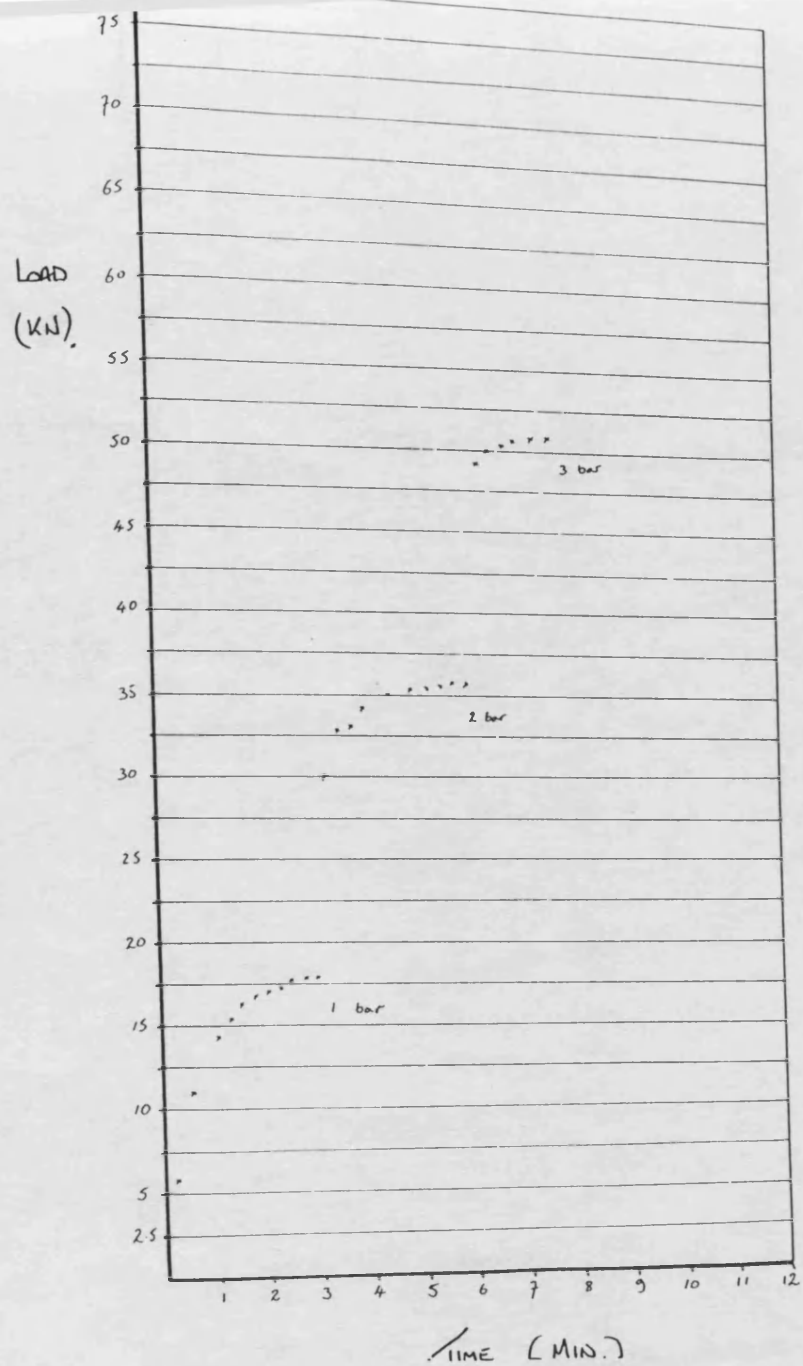


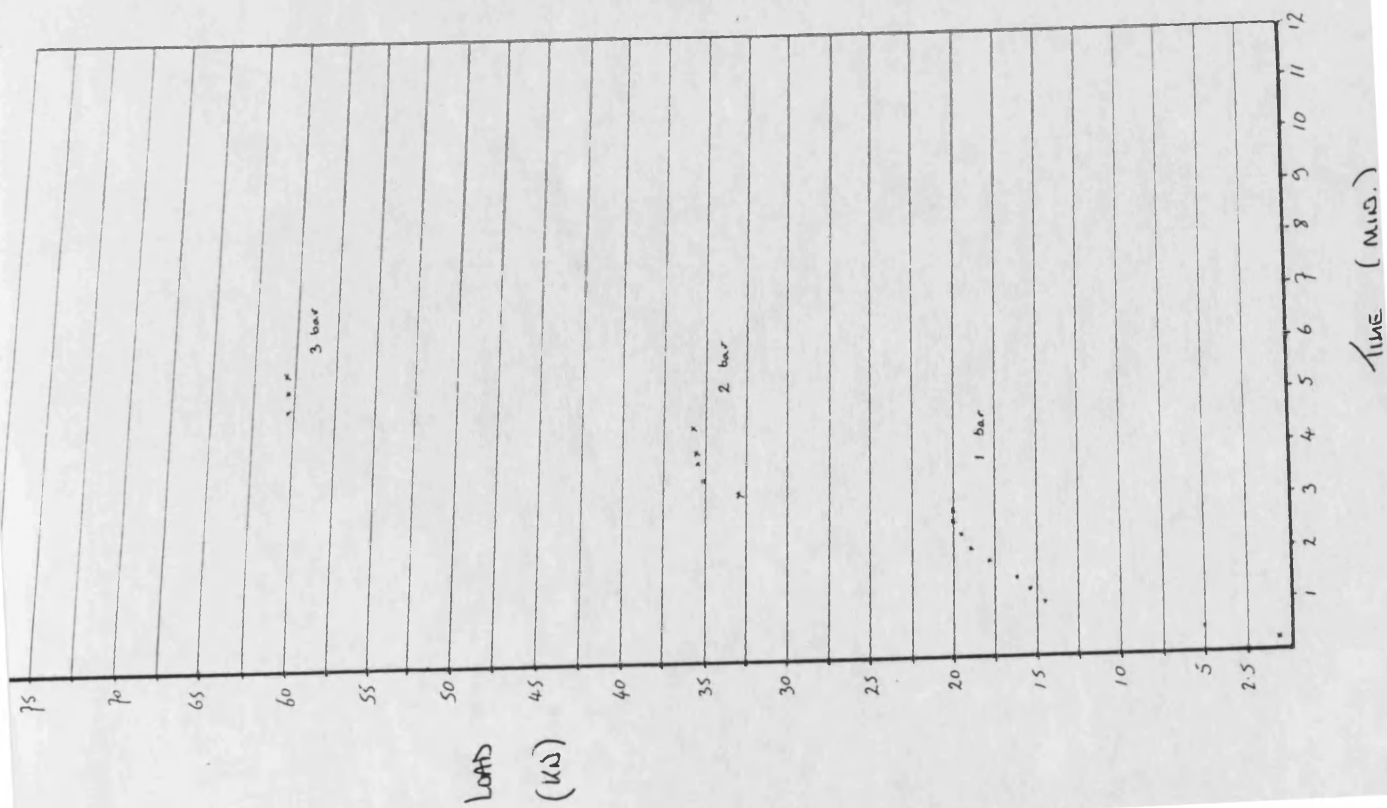
Headwater Lake Dam.



TIME (min.)







Test results for 2nd stage static calibration tests, as detailed in sections 8.3.6, 8.3.7 and 8.3.8. Sample sizes used - predominately 38mm and 100mm, only a few 254mm.

Includes;

1. Summary table.
2. Raw data plots using area correction method. Both multi-stage and single stage tests performed.
3. Derived Mohr circle diagrams for multi stage tests.

SUMMARY TABLE OF 2nd STAGE CALIBRATION TESTS

test no.	sample dia. mm	material	Test type	mass g	voids ratio	Rel. den. %	σ_3 kN/m ²	σ_1 kN/m ²	ϕ (°)
1	100	Cal. Sand	multi stage	2247	0.585	80	100 200 300 400	471 879 1131 1403	41 39 36 34
2	100	Cal. Sand	multi stage	2247	0.585	80	200 300 400 500	850 1214 1562 1878	38 37 36 35
3	100	Cal. Sand	single	2247	0.585	80	200	844	38
4	100	Cal. Sand	multi stage	2247	0.585	80	200 300 400	835 1173 1473	38 36 35
5	100	Cal. Sand	multi stage	2066	0.724	13	200 300 400	771 1087 1387	36 35 34
6	100	Cal. Sand	single	2086	0.707	21	200	724	35
7	100	Cal. Sand	multi stage	2209	0.612	67	200 300 400	777 1141 1479	36 36 35
8	100	Cal. Sand	multi stage	2140	0.664	42	200 300 400	720 1125 1447	34 35 35
9	100	Cal. Sand	multi stage	2086	0.707	21	200 300 400	727 1052 1346	35 34 33
10	100	Cal. Sand	single	2209	0.612	67	400	1605	37

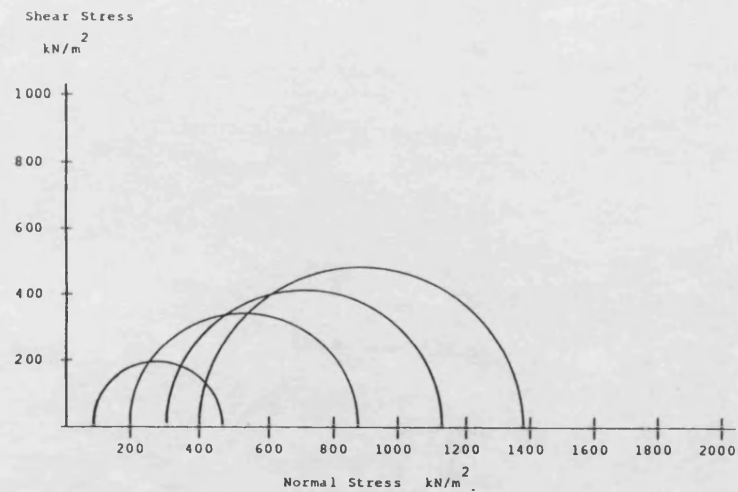
test no.	sample dia. mm	material	Test type	mass g	voids ratio	Rel. den. %	σ_3 kN/m ²	σ_1 kN/m ²	ϕ (°)
11	100	Cal. Sand	single	2209	0.612	67	400	1447	35
12	38	Cal. Sand	multi stage	103.5	0.612	67	200 300	788 1132	37 36
13	38	Cal. Sand	multi stage	103.5	0.612	67	200 300	764 1094	36 35
14	38	Cal. Sand	multi stage	100	0.668	40	200 300	913 1117	40 35
15	38	Cal. Sand	multi stage	97.2	0.716	17	200 300	865 1084	39 35
16	38	Cal. Sand	single	100	0.668	40	400	1499	35
17	100	Type (2) 1 - 2mm grading	single	2068	0.744	80	300	1442	41
18	38	ditto	single	96.86	0.744	80	200	1027	42
19	38	ditto	single	92.75	0.821	50	200	1074	43
20	38	ditto	multi stage	87.3	0.934	20	200 300	1133 1399	44 40
21	100	ditto	multi stage	2068	0.744	80	200 300	1142 1496	45 42
22	38	ditto	single	96.86	0.744	80	400	1891	41
23	38	ditto	multi stage	87.3	0.934	20	200 300	996 1296	42 39
24	38	ditto	multi stage	96.86	0.744	80	200 300 400	1150 1452 1736	44 41 39
25	38	ditto	multi stage	92.76	0.824	50	200 300 400	1090 1440 1706	44 41 38

test no.	sample dia. mm	material	Test type	mass g	voids ratio	Rel. den. %	σ_3 kN/m ²	σ_1 kN/m ²	ϕ (°)
26	100	Type (2) 1 - 2mm grading	multi stage	1980	0.821	50	200 300	1040 1357	43 40
27	100	ditto	multi stage	1895	0.903	30	200 300	1072 1394	43 40
28	254	ditto	single	28.3kg	0.62	63	200	879	39

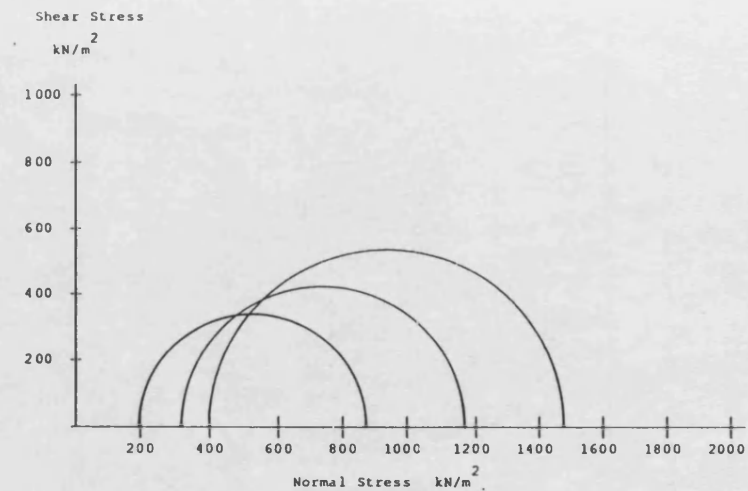
sample volume for 100mm samples = 1352 cc

sample volume for 38mm samples = 63.33 cc

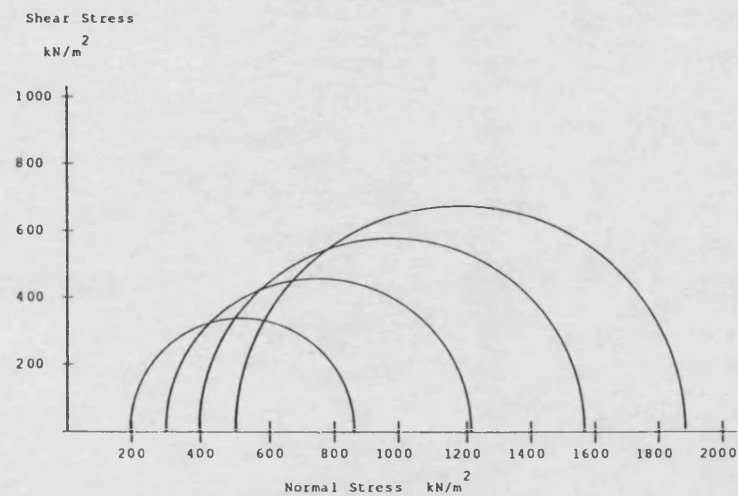
Tests on 38mm samples were carried out with drain line closed during axial load application, but the vacuum had been released. If this had any effect it would be to increase the apparent strength of the samples, as air would not be able to escape giving rise to an internal pressure. However, air does compress, still allowing sample compression.



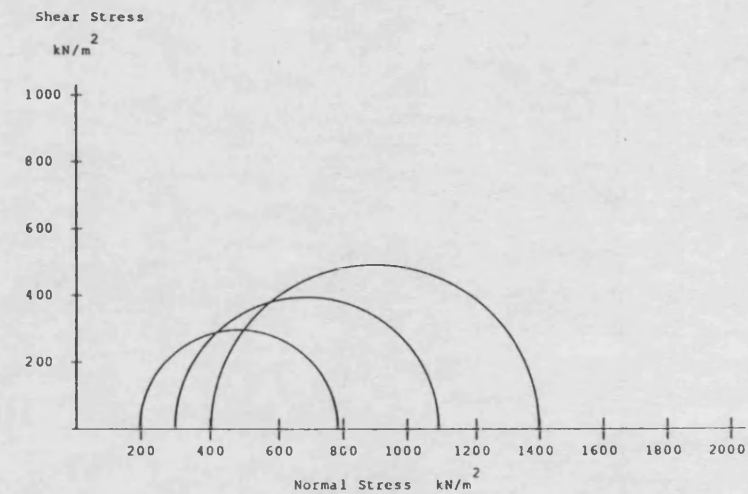
Mohrs circle plot for Test No. 1



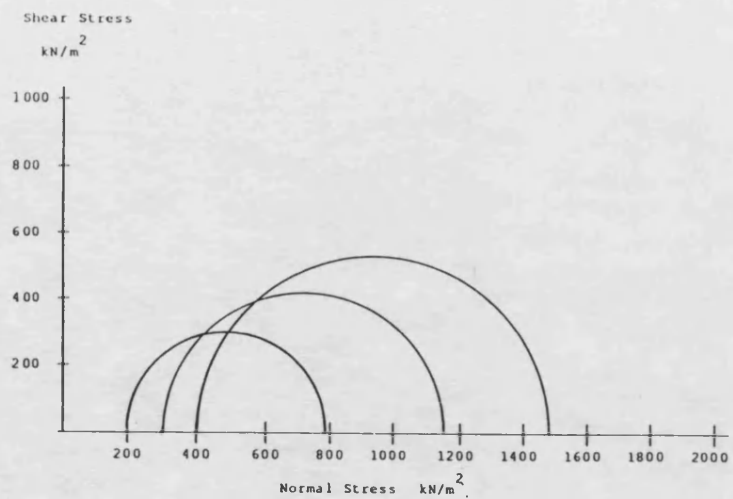
Mohrs circle plot for Test No. 4



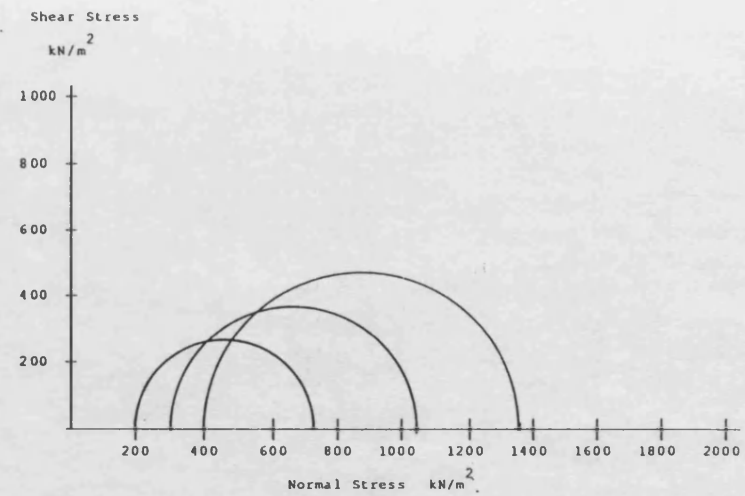
Mohrs circle plot for Test No. 2



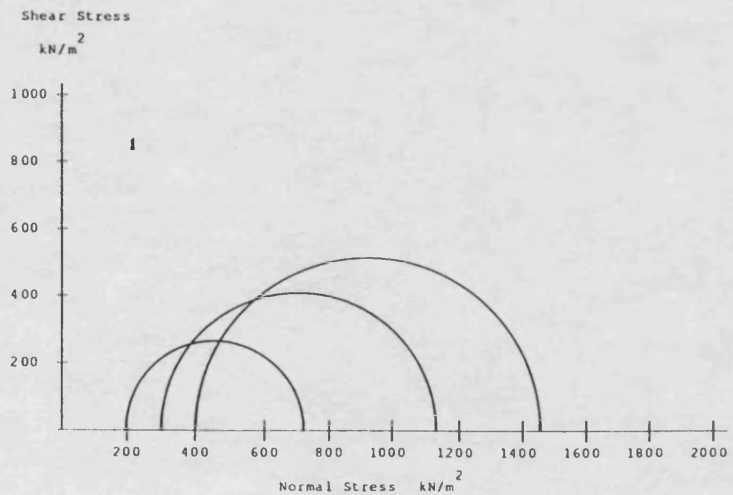
Mohrs circle plot for Test No. 5



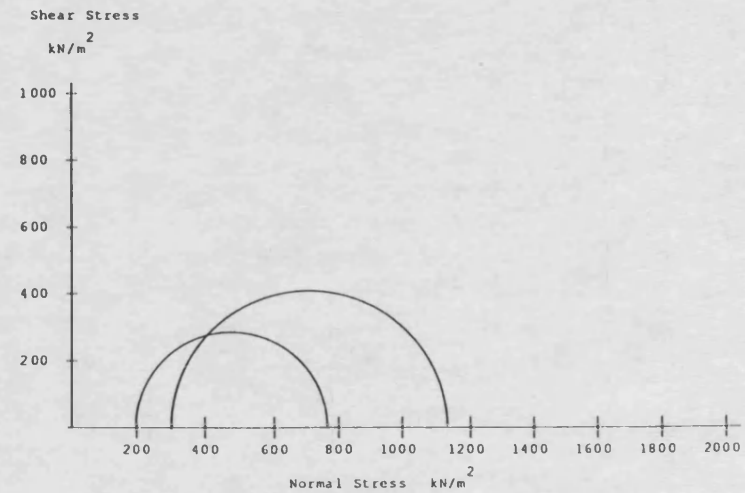
Mohrs circle plot for Test No. 7



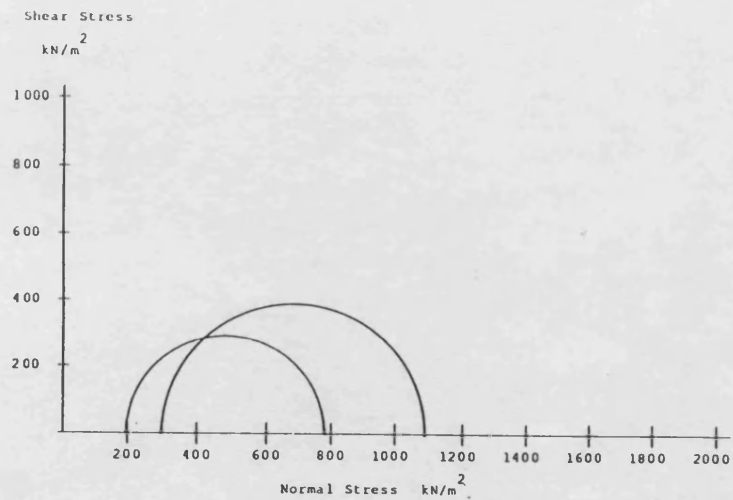
Mohrs circle plot for Test No. 9



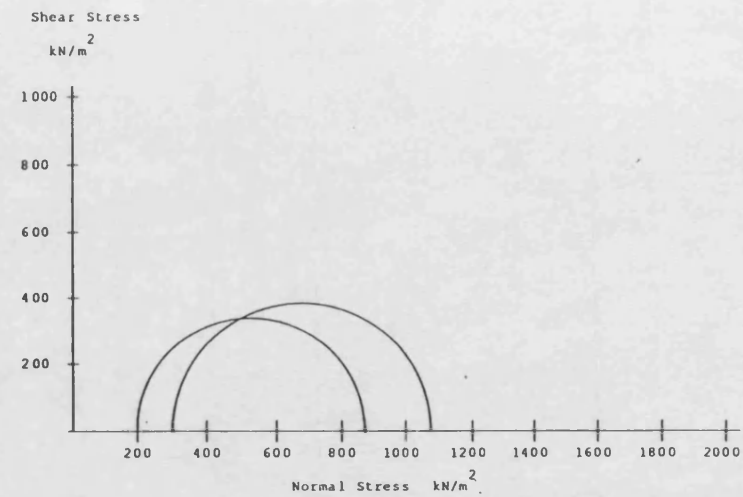
Mohrs circle plot for Test No. 8



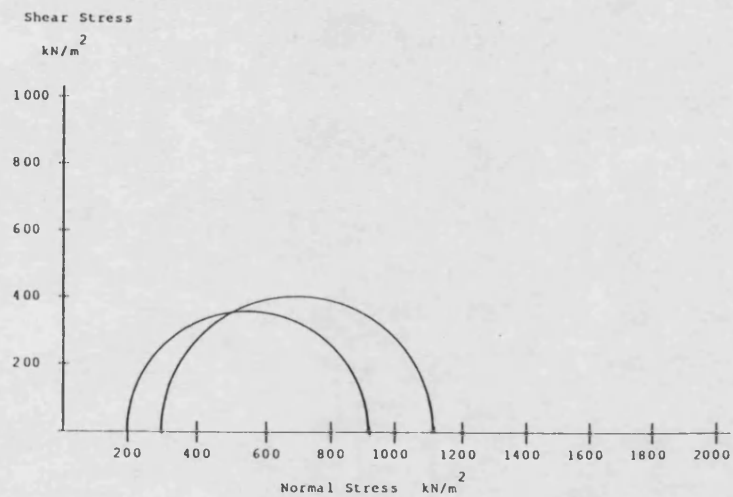
Mohrs circle plot for Test No. 12



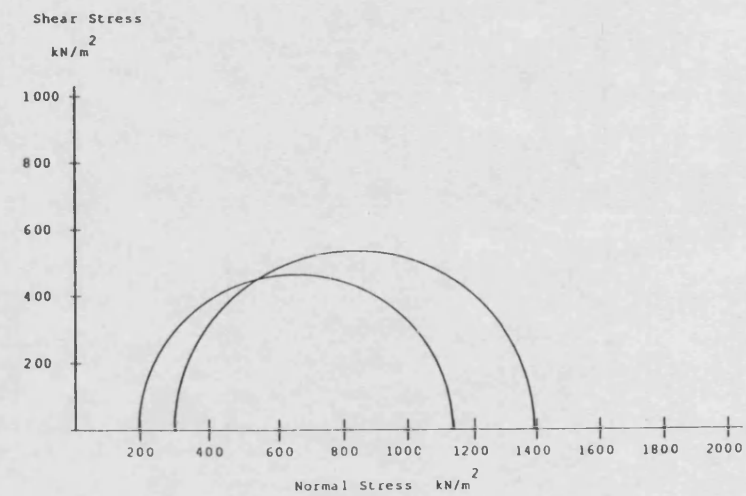
Mohrs circle plot for Test No. 13



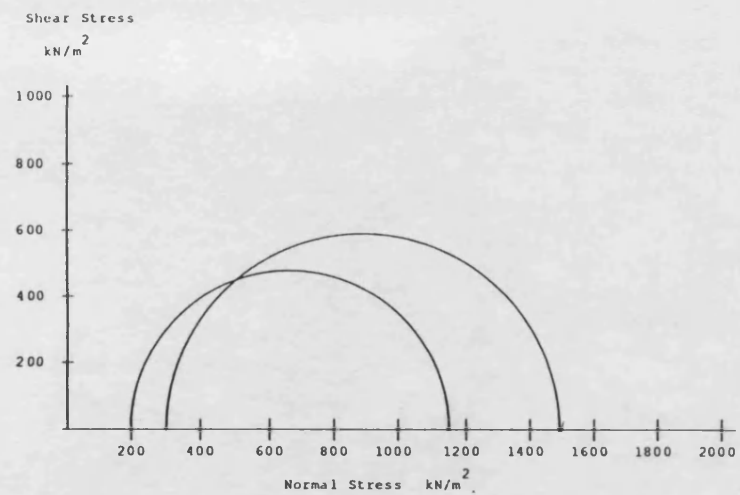
Mohrs circle plot for Test No. 15



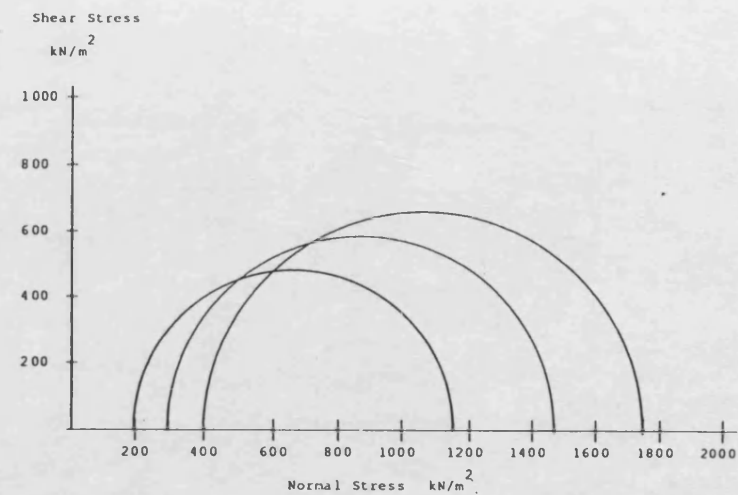
Mohrs circle plot for Test No. 14



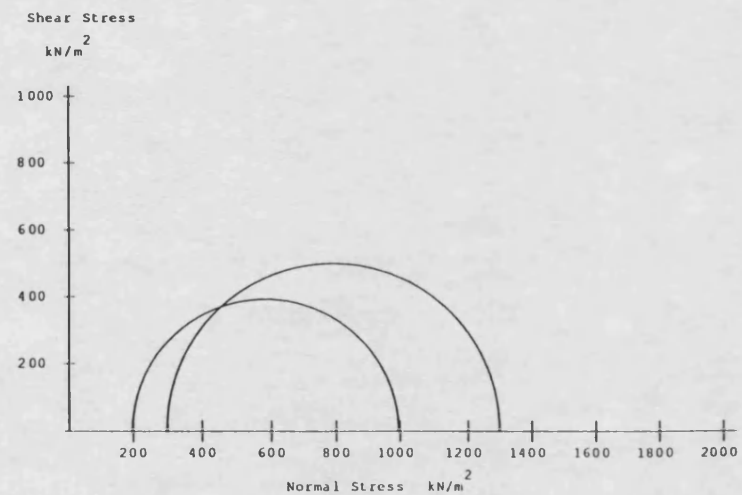
Mohrs circle plot for Test No. 20



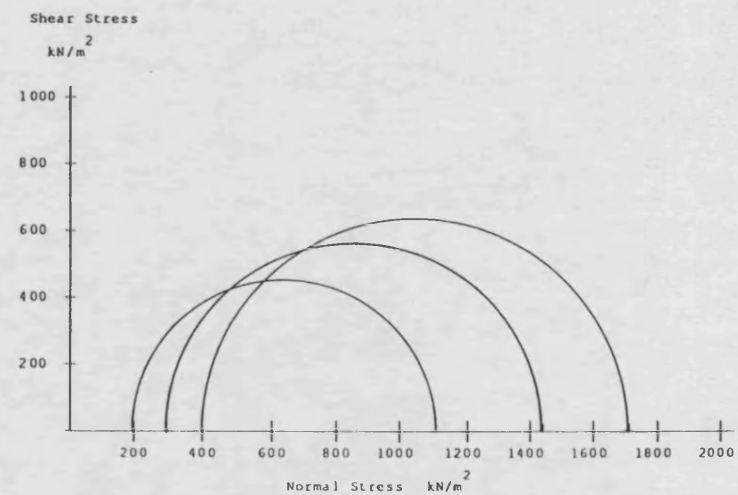
Mohr's circle plot for Test No. 21



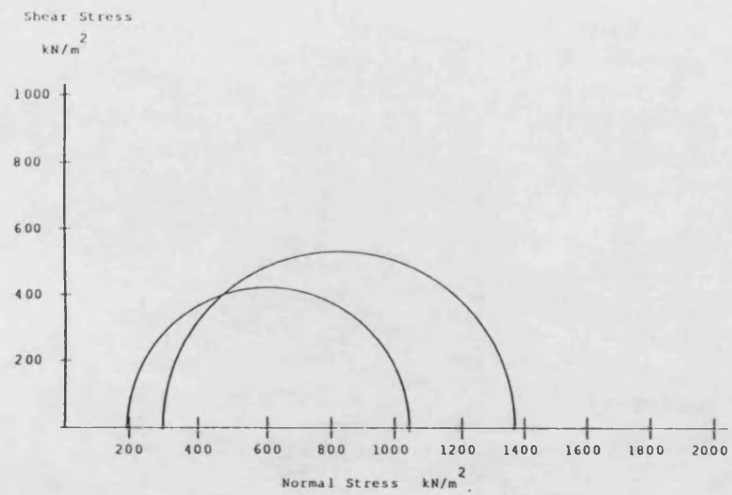
Mohr's circle plot for Test No. 24



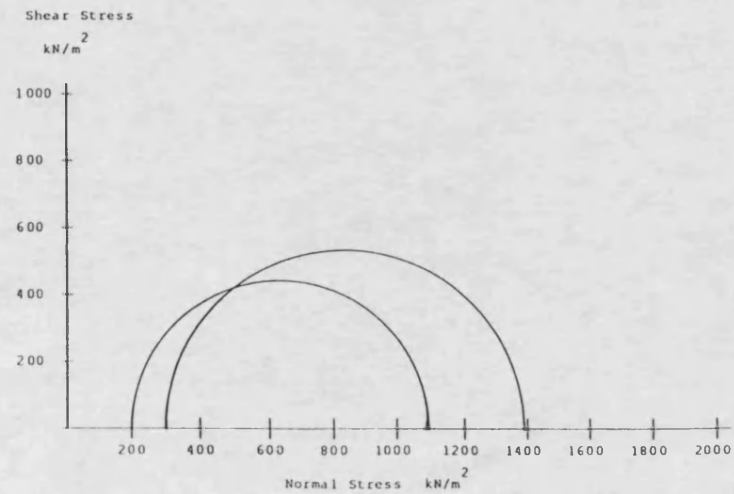
Mohr's circle plot for Test No. 23



Mohr's circle plot for Test No. 25



Mohrs circle plot for Test No. 26

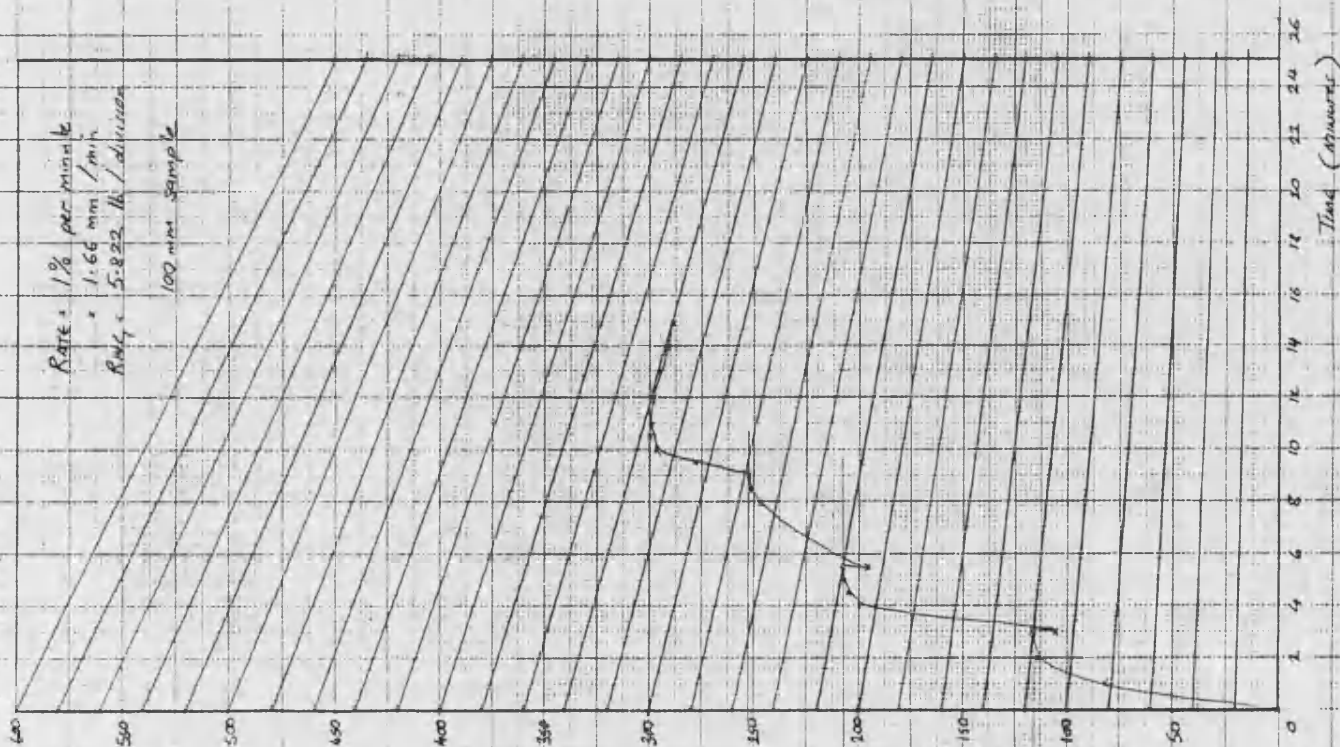


Mohrs circle plot for Test No. 27

TEST ① REMOTED Calibration sand

PROVING RING DIVISIONS

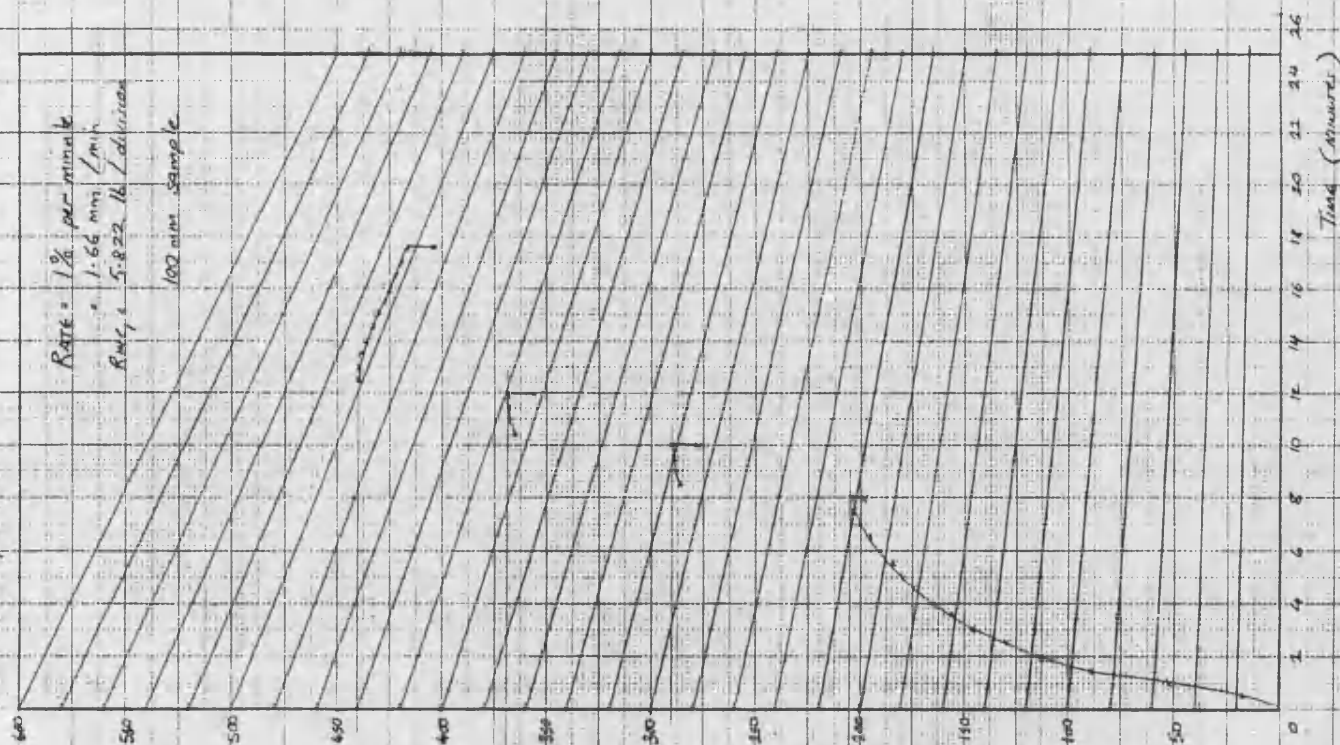
Rate: 1% per minute
= 1.66 mm/min
Rate: 5.822 lb/division
100 mm sample



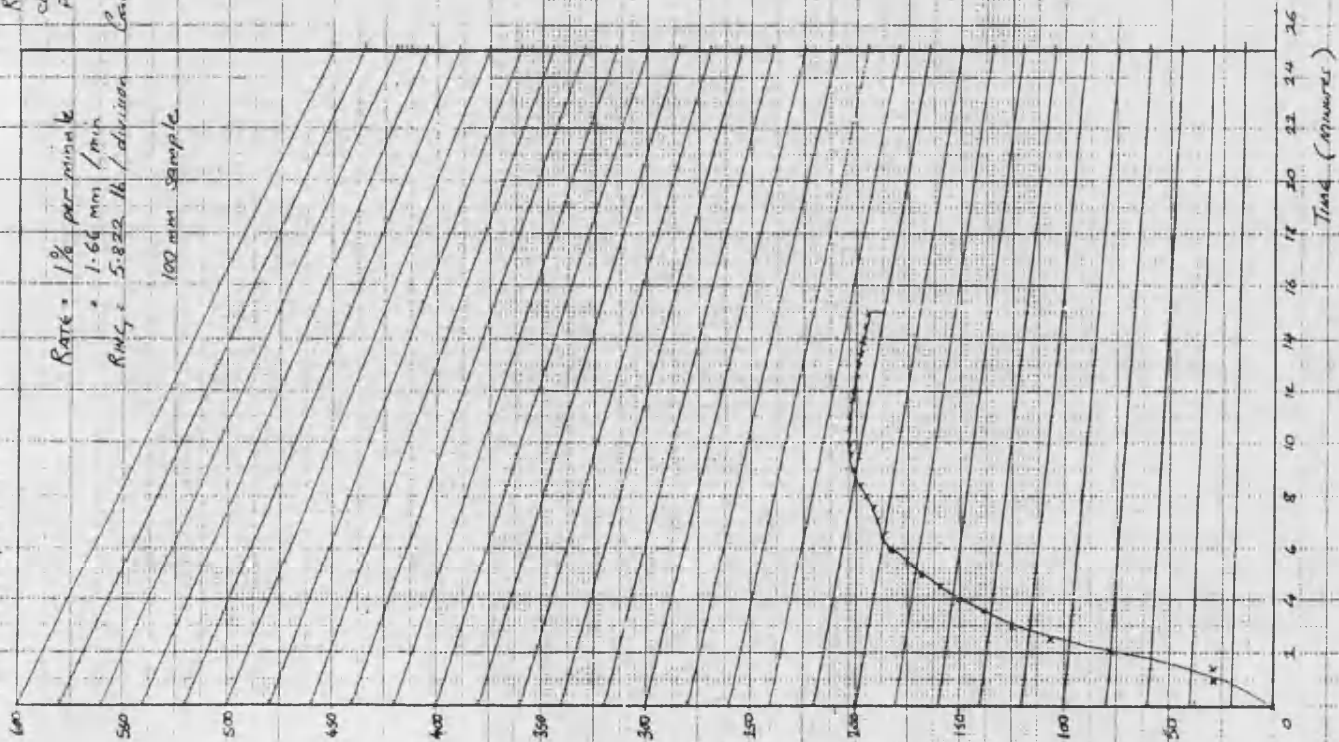
TEST ② REMOTED Calibration sand

PROVING RING DIVISIONS

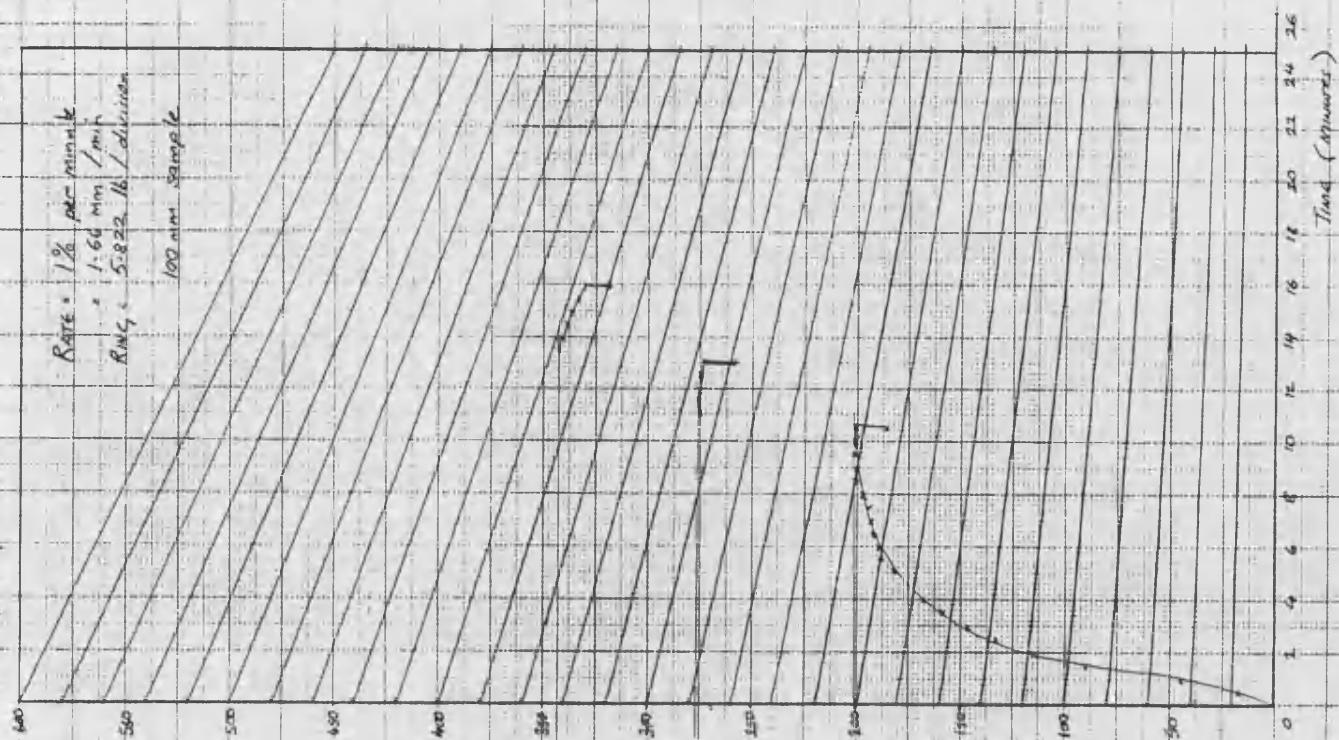
Rate: 1% per minute
= 1.66 mm/min
Rate: 5.822 lb/division
100 mm sample



TEST 3 6/1 Nov 92
 REPORT ON
 COLLECTED
 PAPER
 PROOFING RING DIVISIONS
 Rate: 1% per minute
 1.66 mm/min
 RWG: 5.822 lb/division
 100 mm sample
 Calibration sand



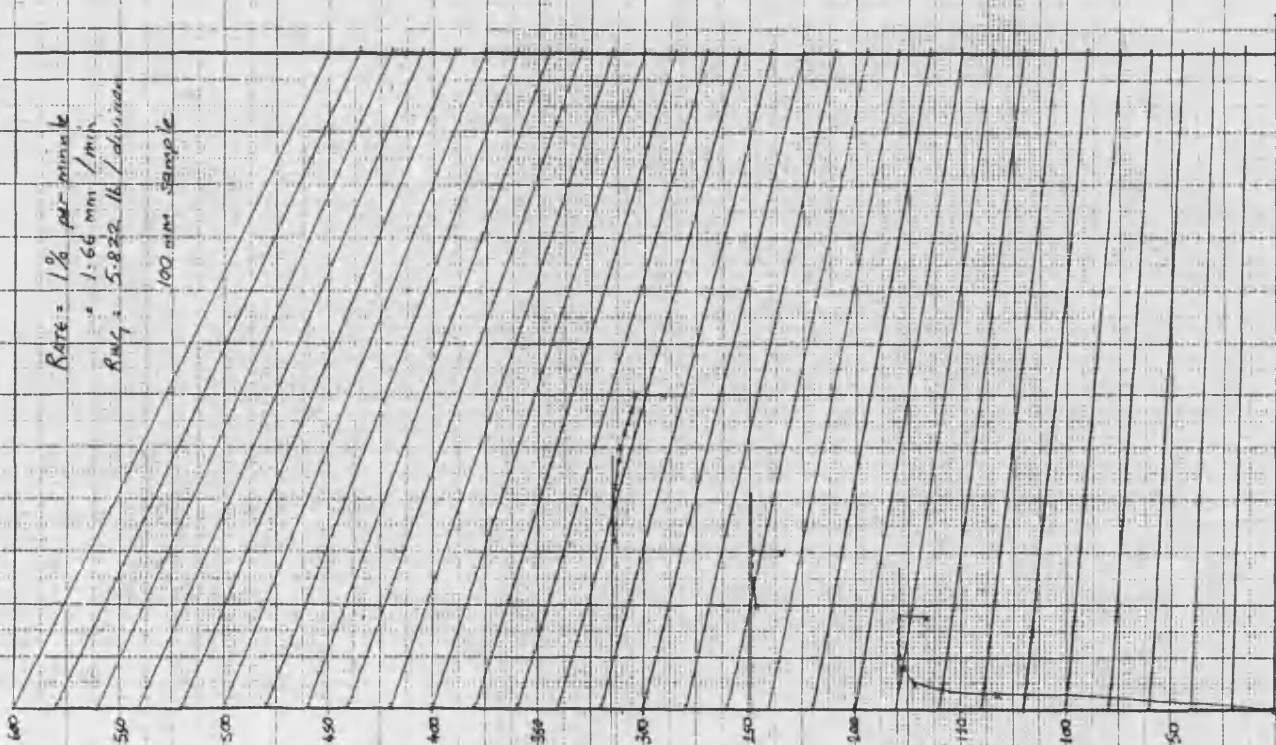
TEST 4 RECORDED
 CALIBRATION SAND
 PROOFING RING DIVISIONS
 Rate: 1% per minute
 1.66 mm/min
 RWG: 5.822 lb/division
 100 mm sample



TEST (5) RECALIBRATED
Calibration sand

PROVING RING DIVISIONS

RATE: 1% per minute
1.66 mm/min
RMF: 5.832 lb/division
100 mm sample

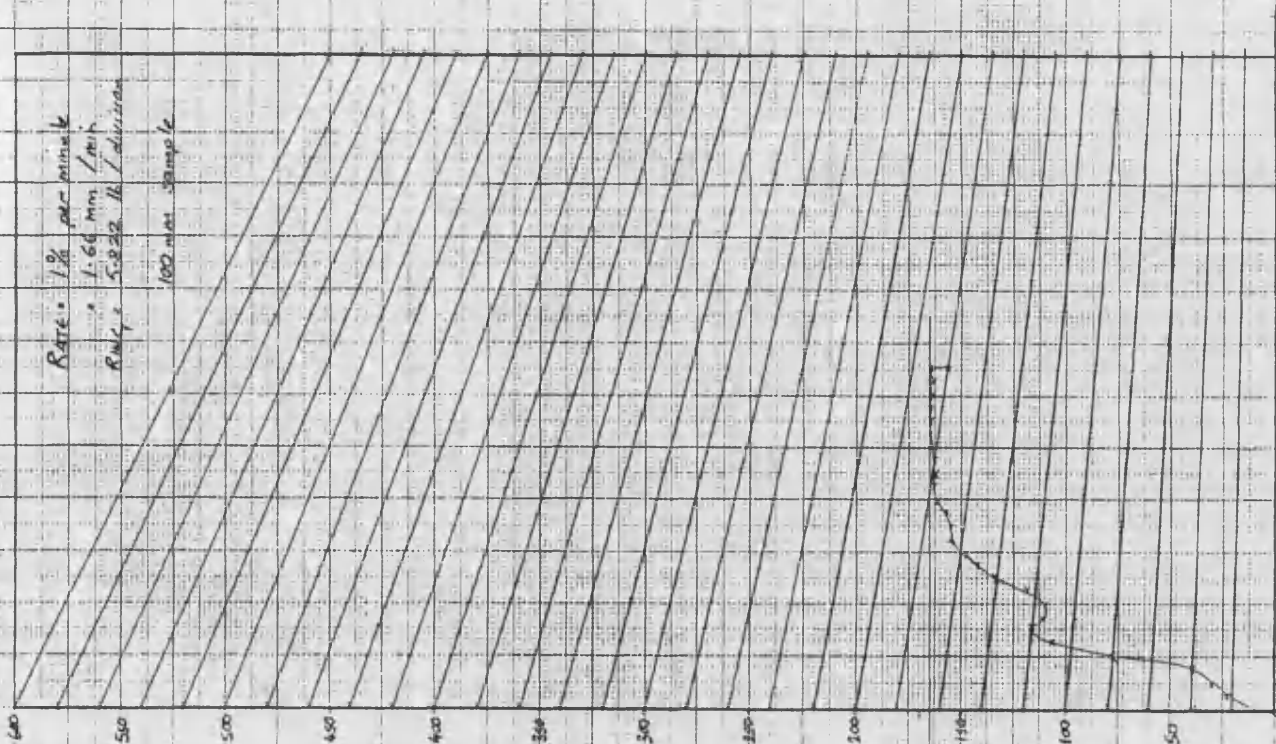


Time (minutes)

TEST (5) RECALIBRATED
Calibration sand

PROVING RING DIVISIONS

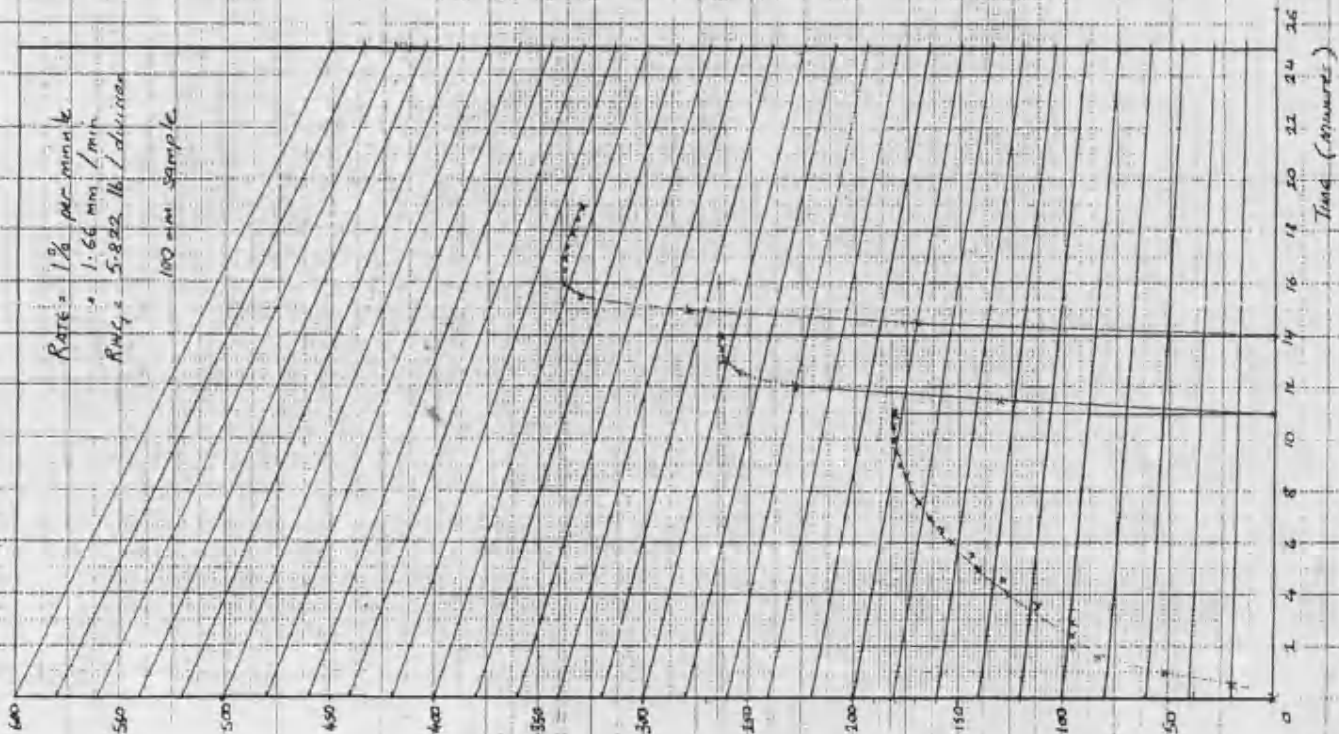
RATE: 1% per minute
1.66 mm/min
RMF: 5.832 lb/division
100 mm sample



Time (minutes)

TEST 7 27/7/92

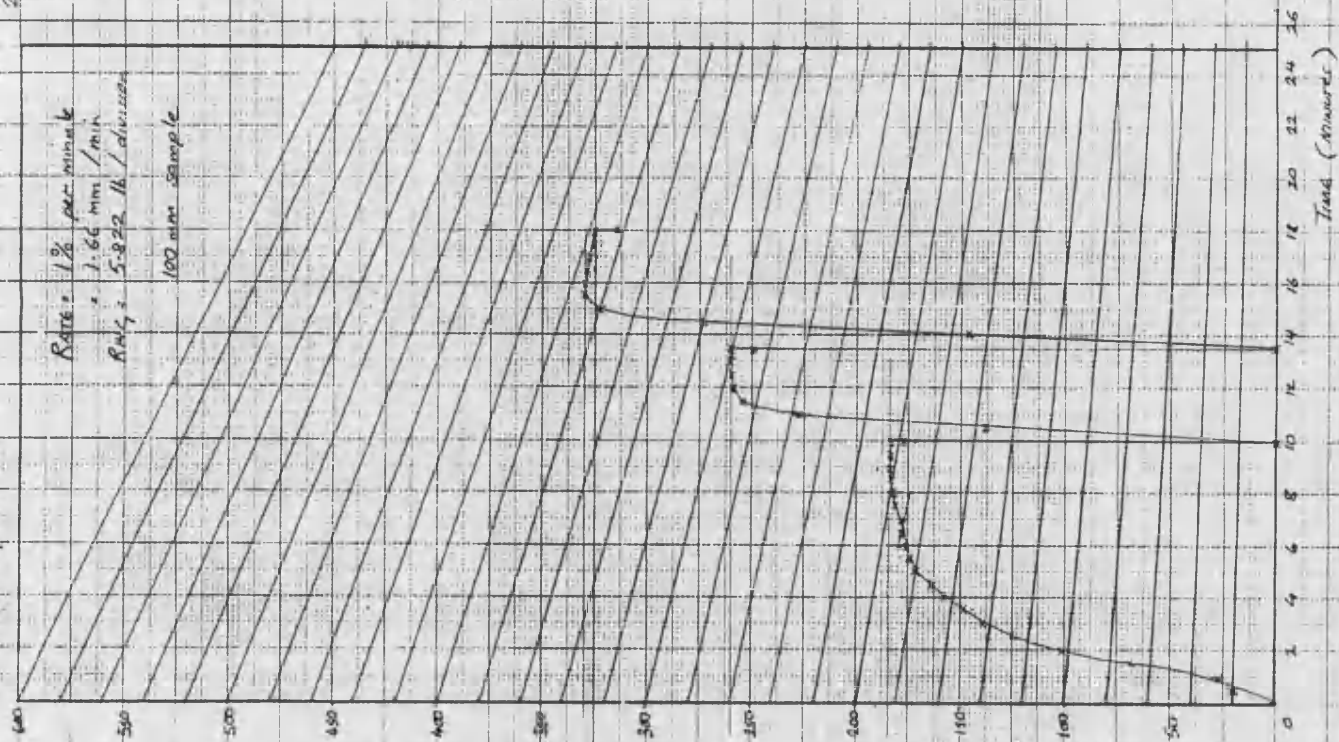
Peeling ring divisions



TEST 8

28/7/92

Peeling ring divisions

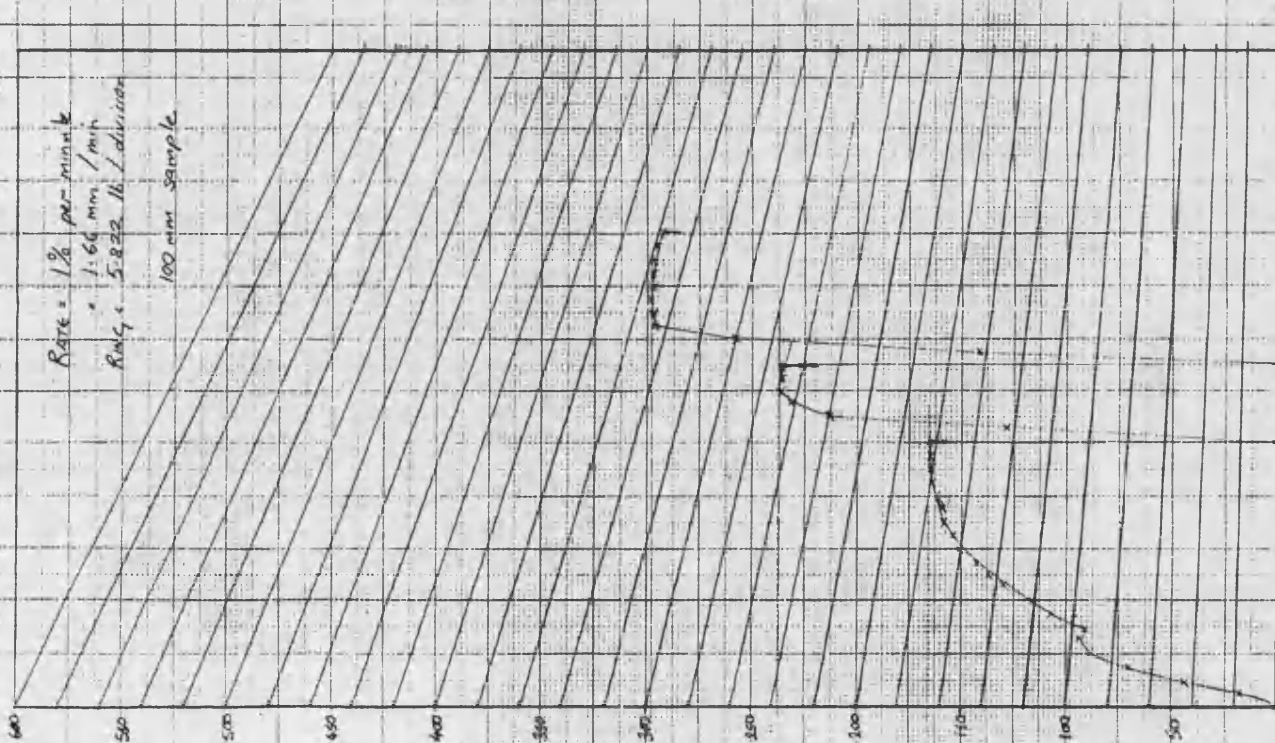


Proving ring divisions

TEST 9

29 JUL 58

Rate = 1% per minute
 1.66 mm/min
 Ring 5.822 lb/division
 100 mm sample



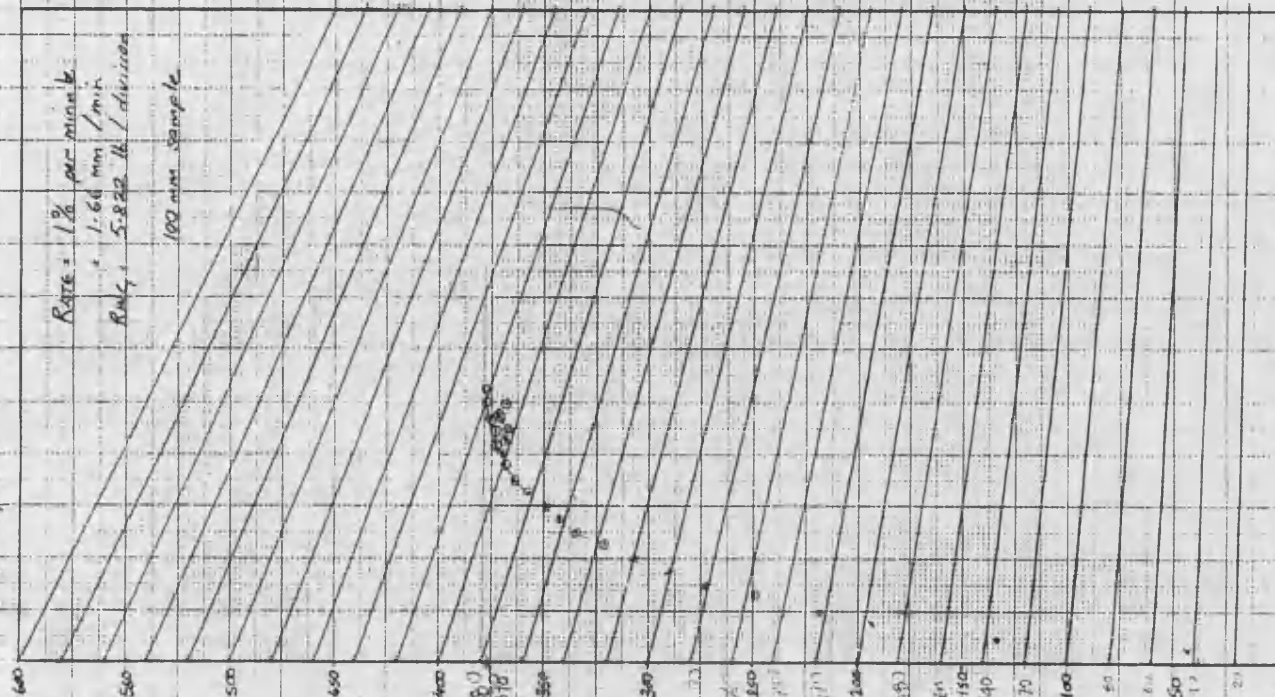
Time (minutes)

Proving ring divisions

TEST 10

4.21V
 PD. 80%

Rate = 1% per minute
 1.66 mm/min
 Ring 5.822 lb/division
 100 mm sample



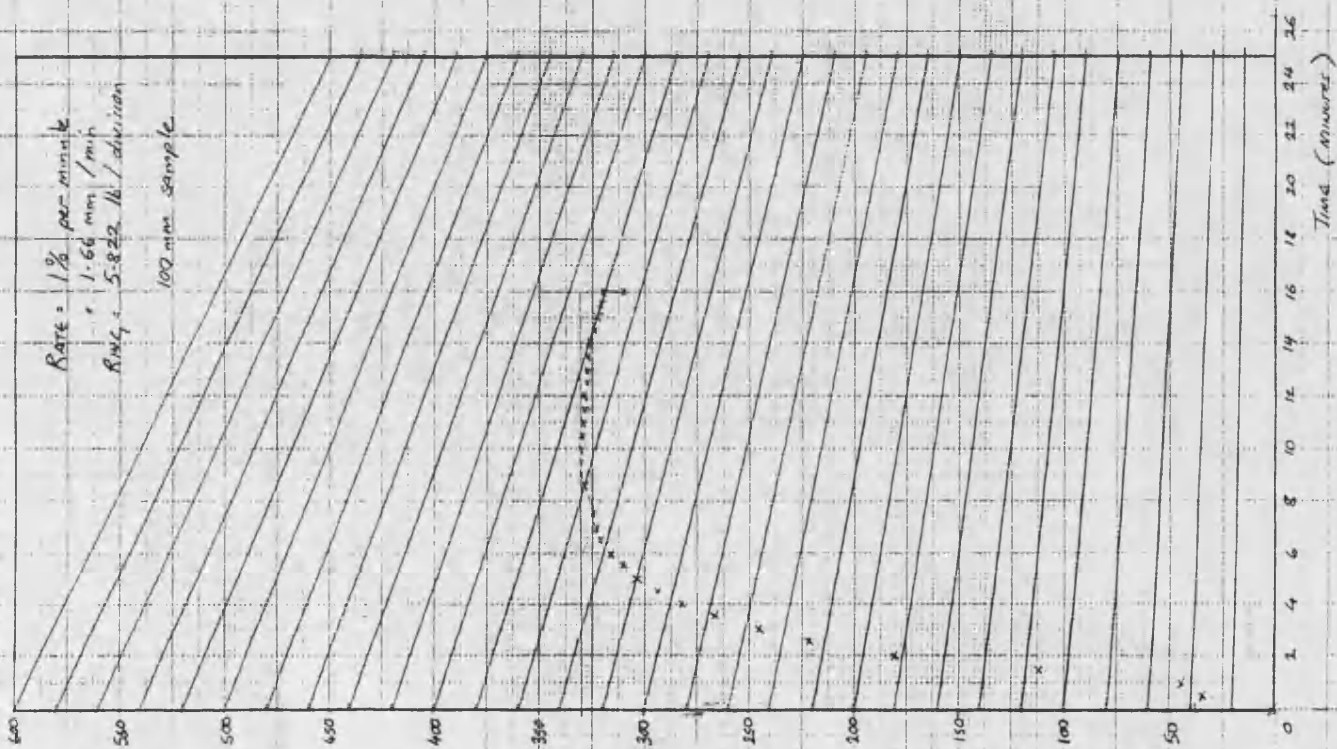
Time (minutes)

30 July '92

RP 50% Test II

Proving ring divisions

Rate: 1% per minute
 " 1.66 mm/min
 RMC: 5.822 lb/division
 100 mm sample



DIVISIONS

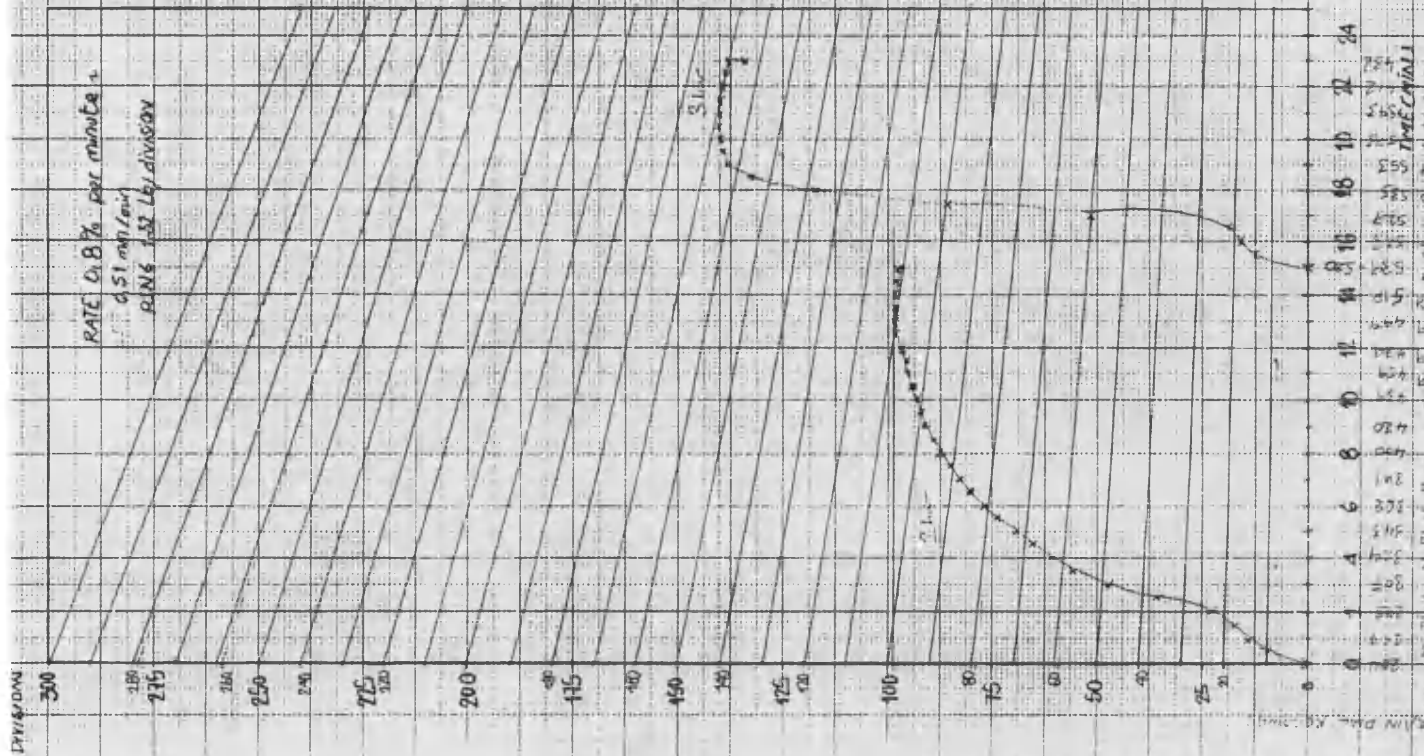
RATE 0.8% per minute
 0.51 mm/min
 RMC 1.52 lb/division

TEST 13

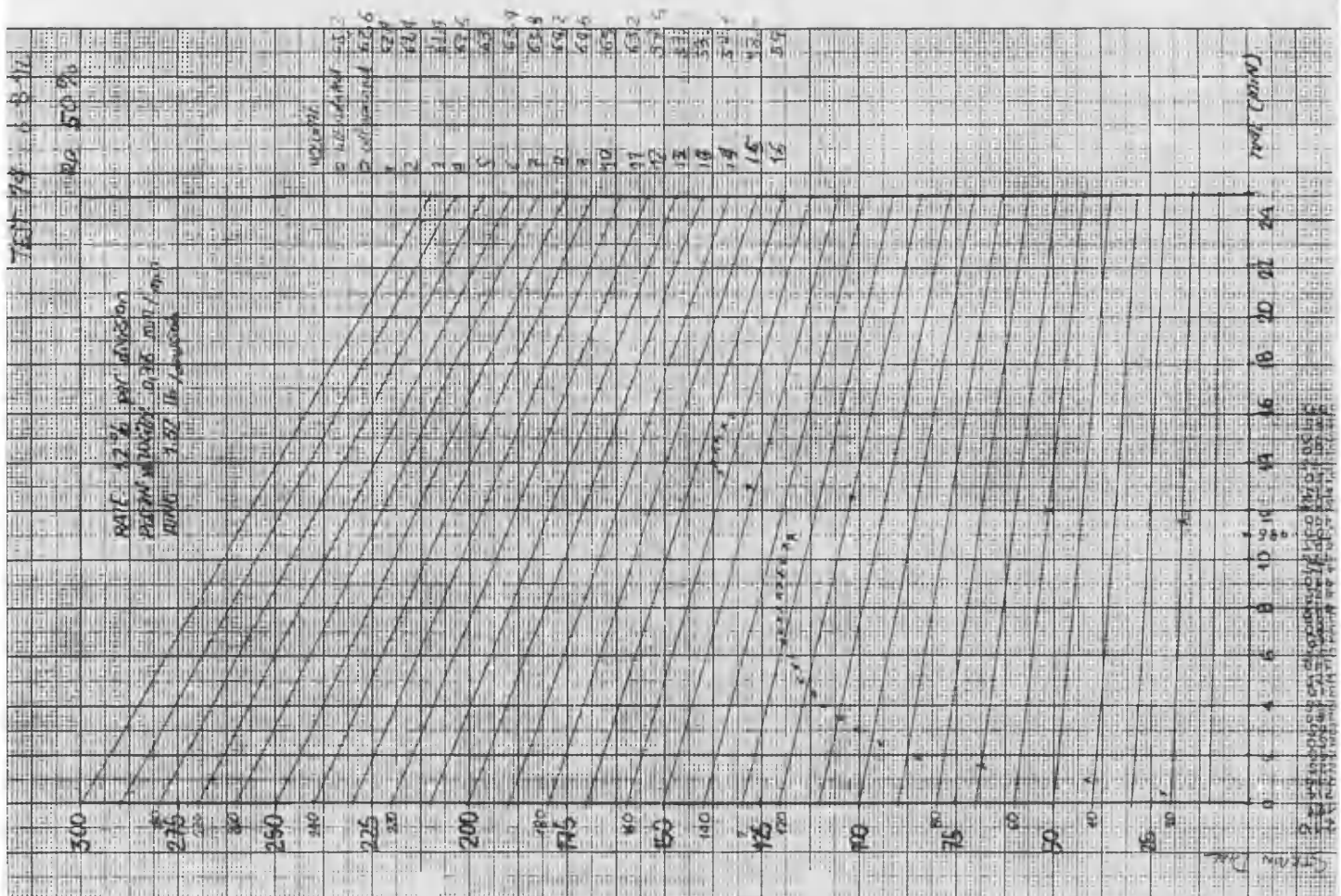
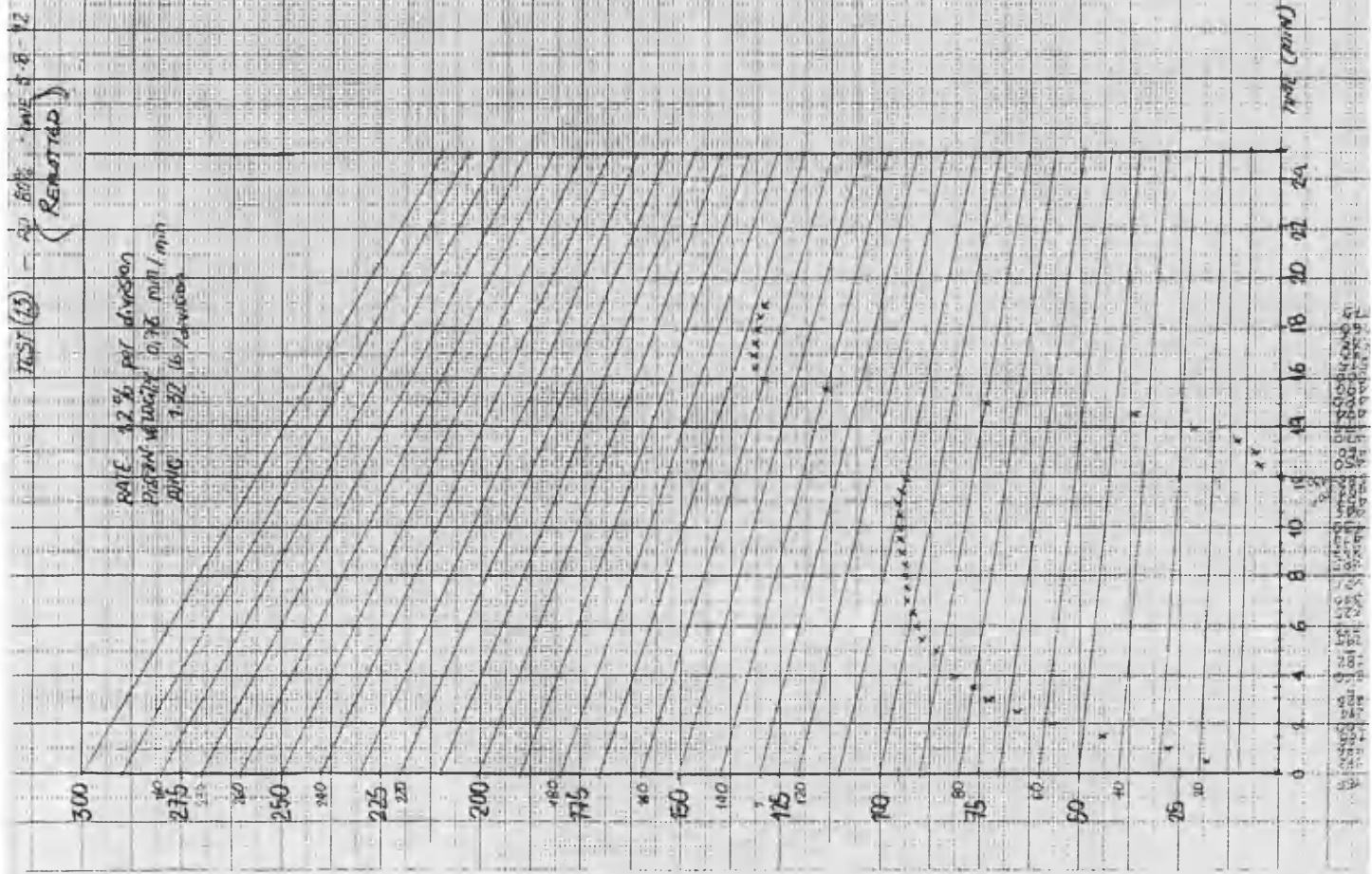
RD = 80%

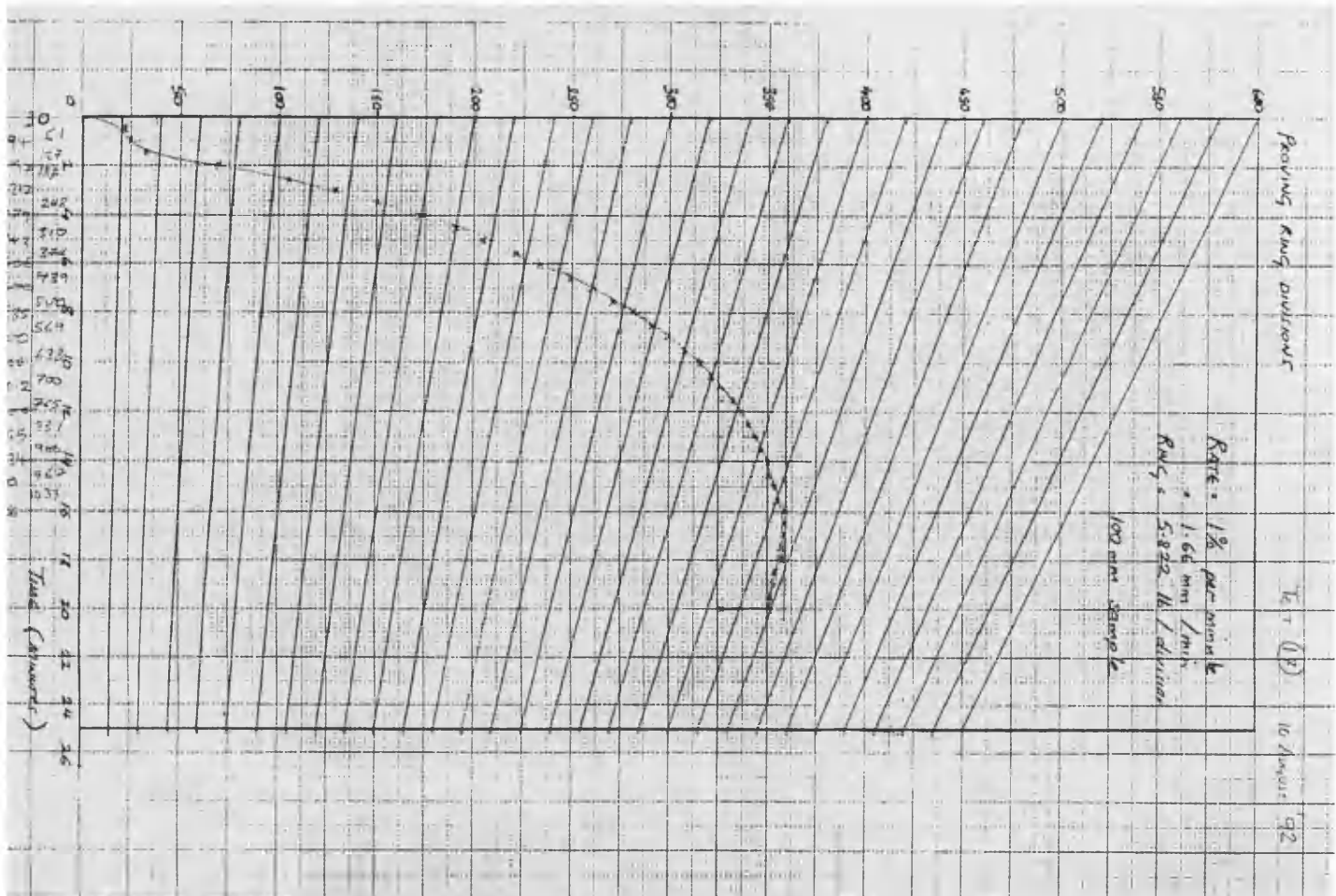
4-8-92

Time

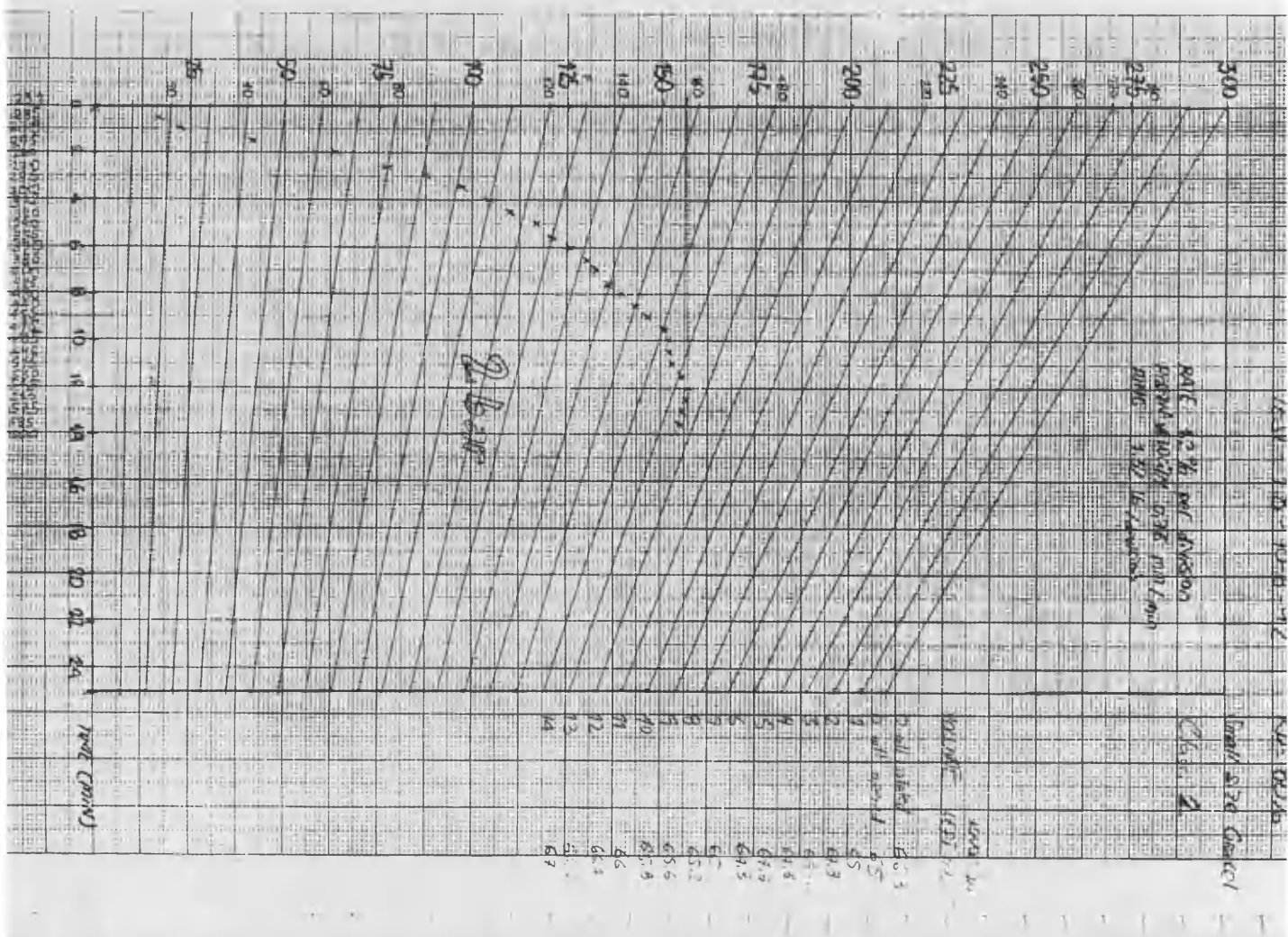


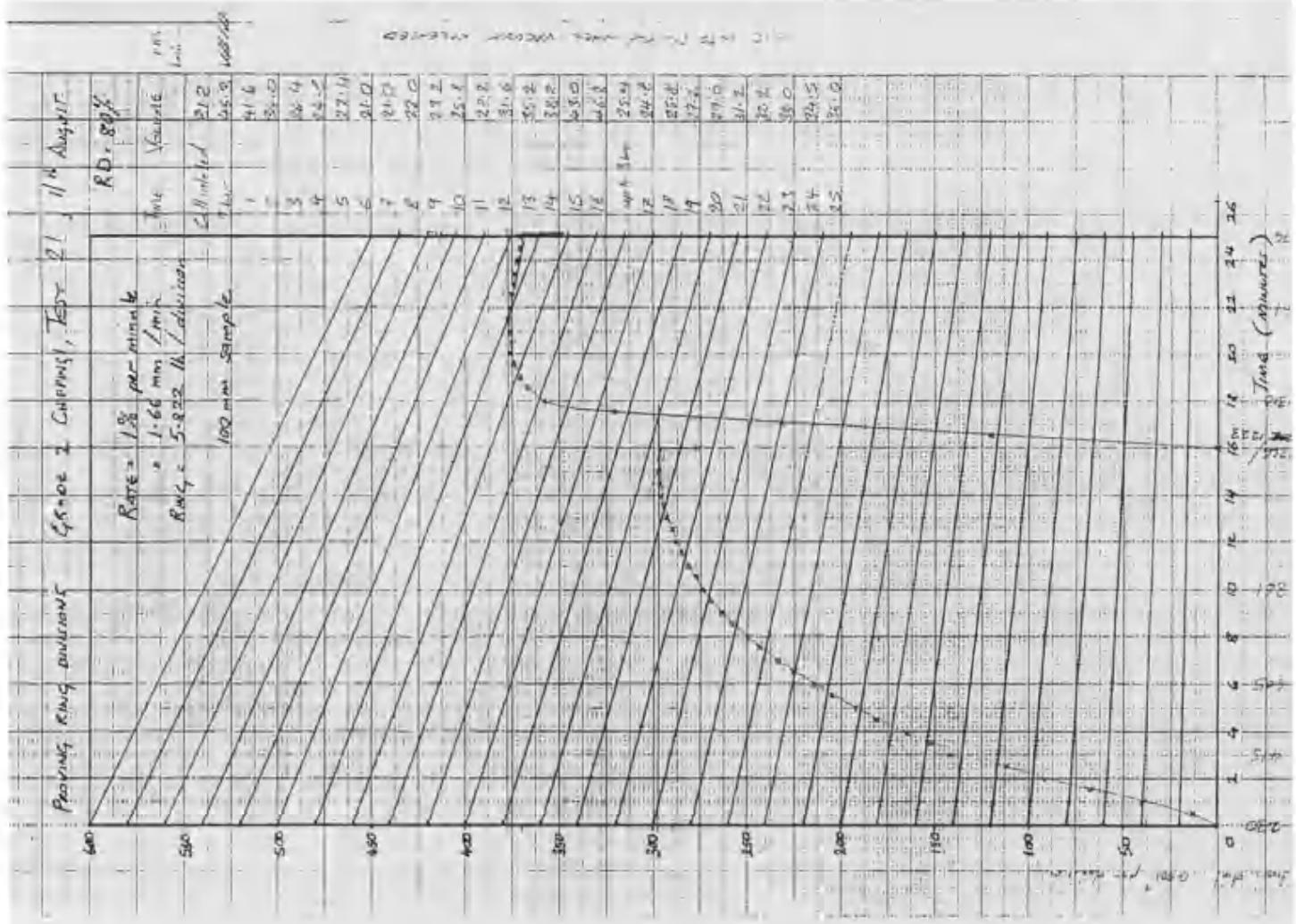
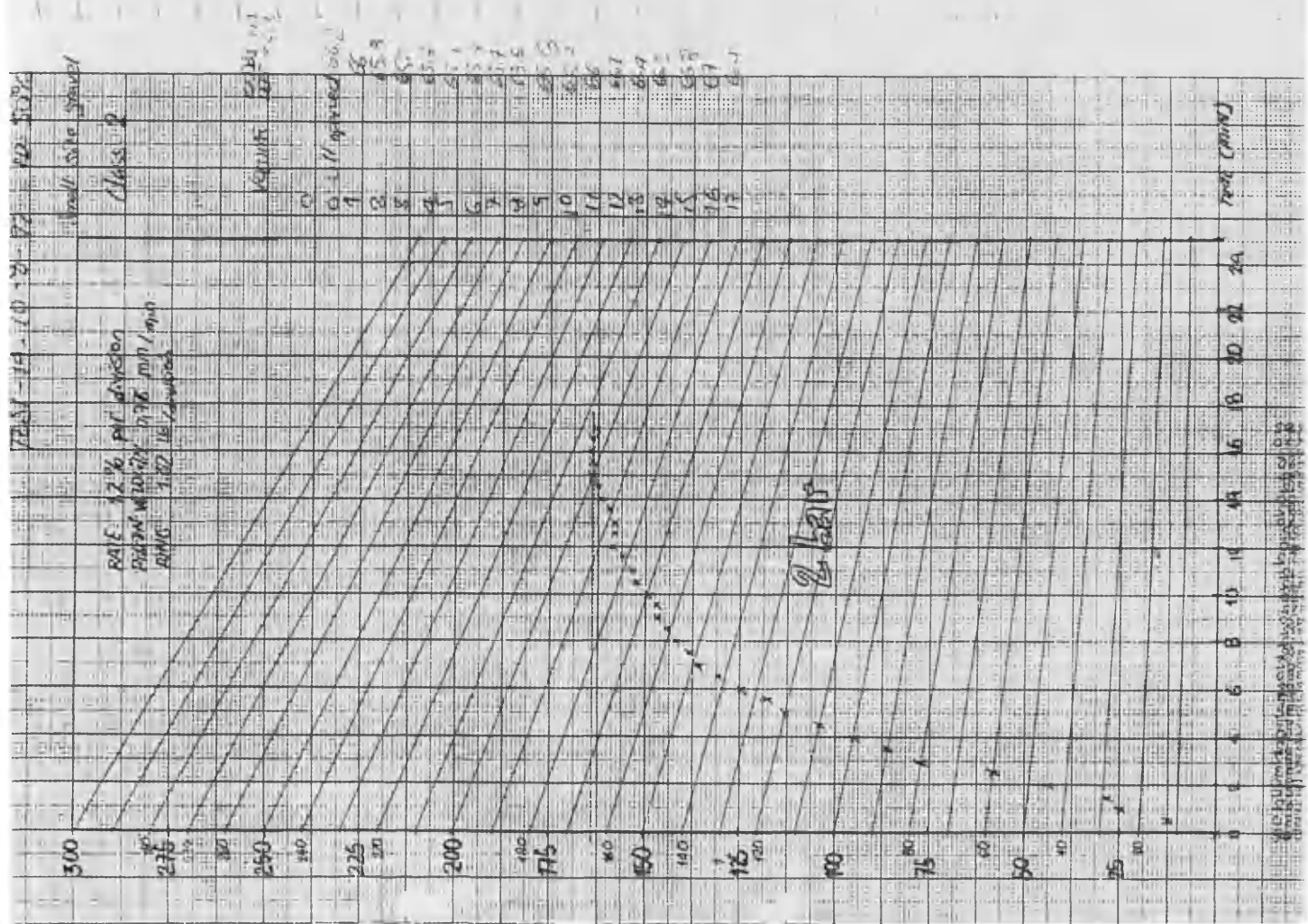
TIME (minutes)

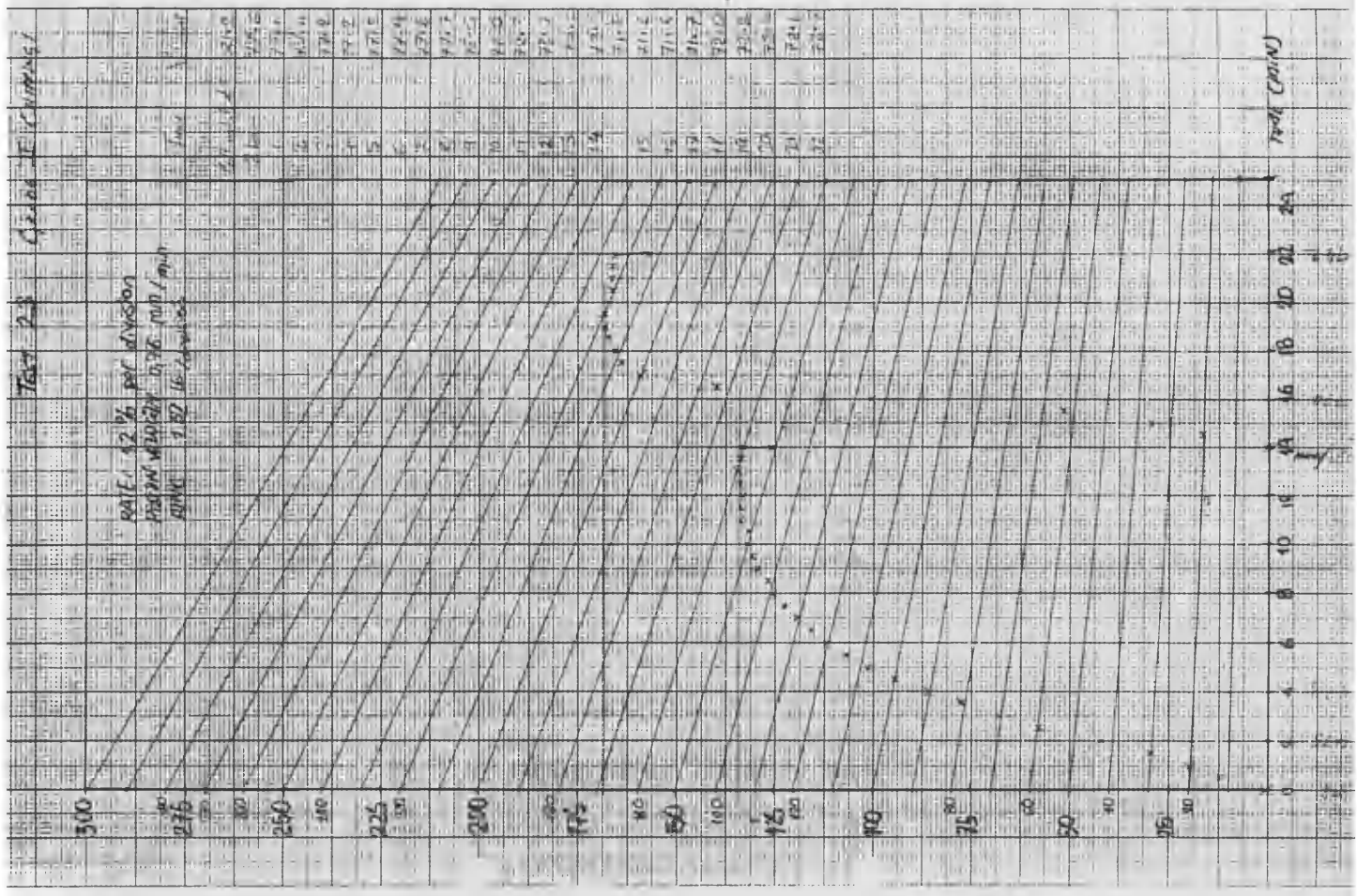
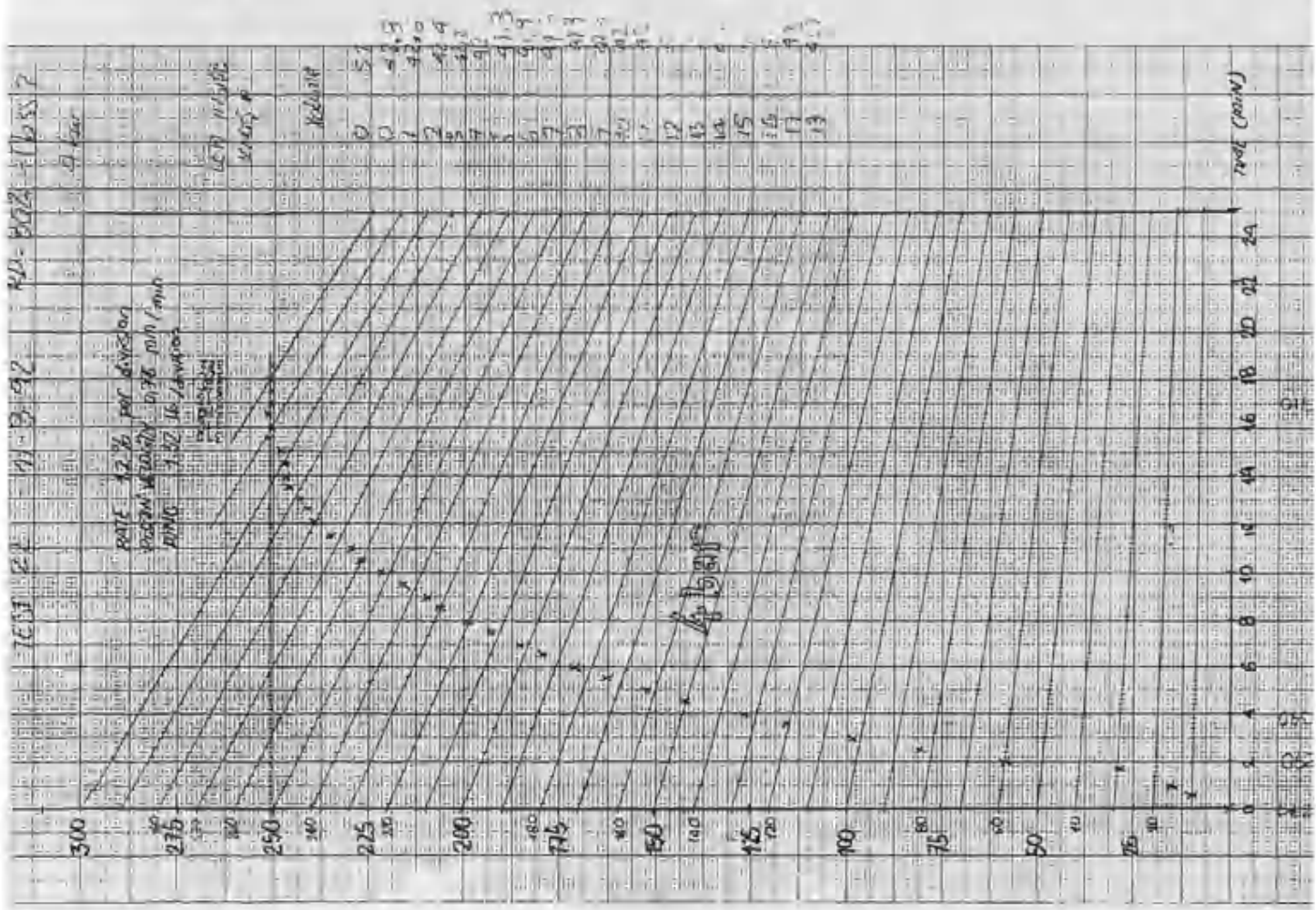


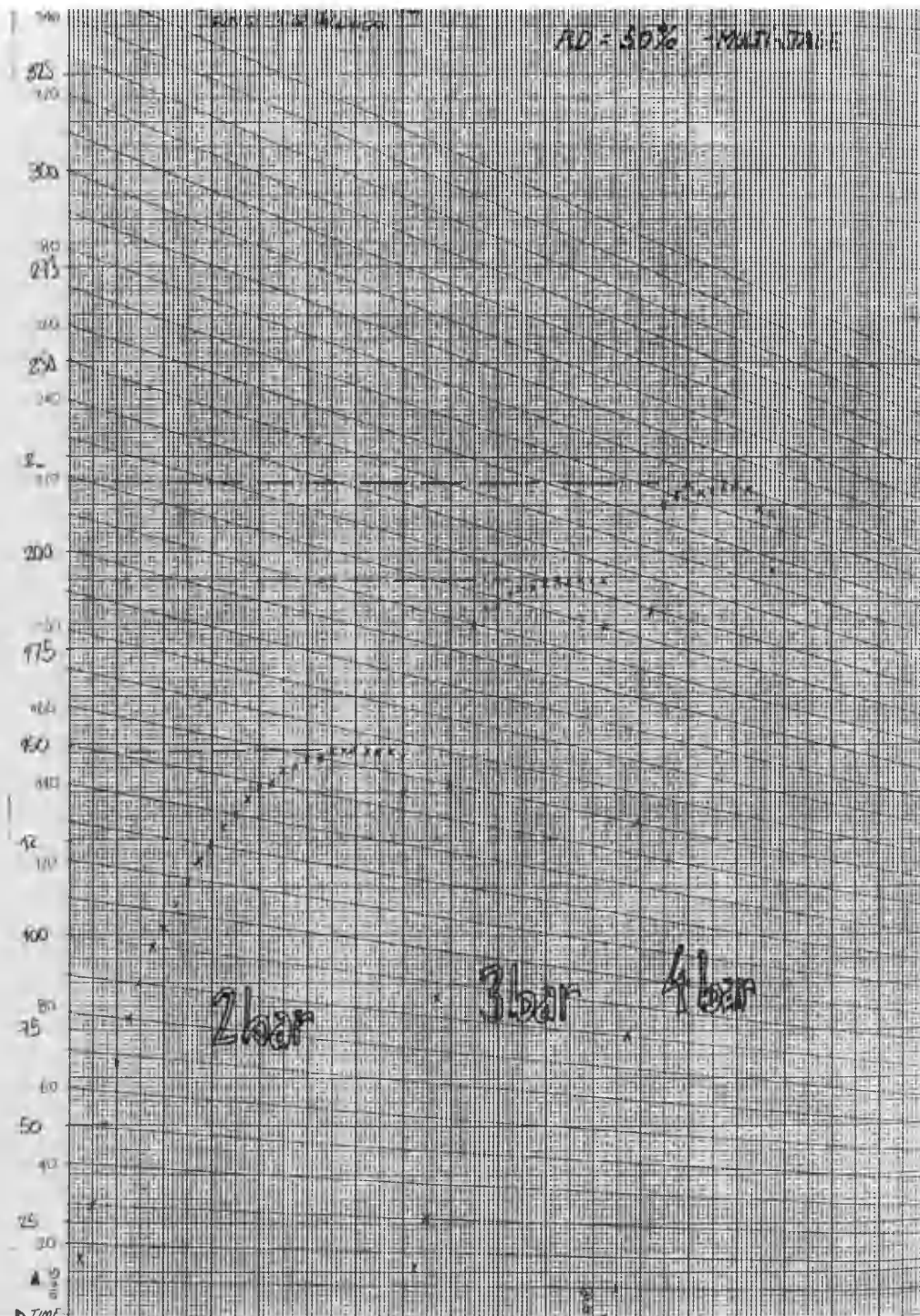
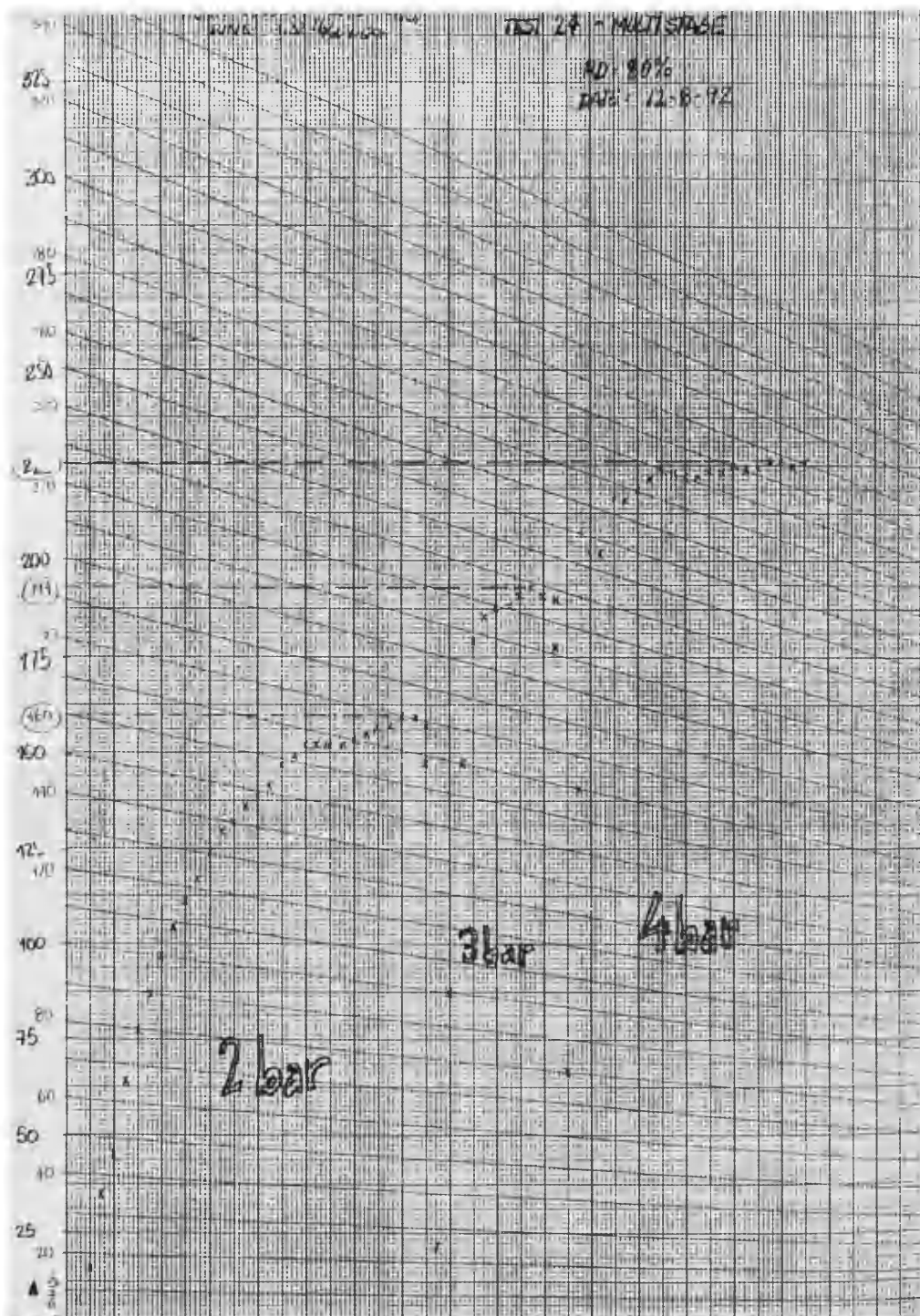


Continued after leakage at sample. Volume not referred to 2 bar





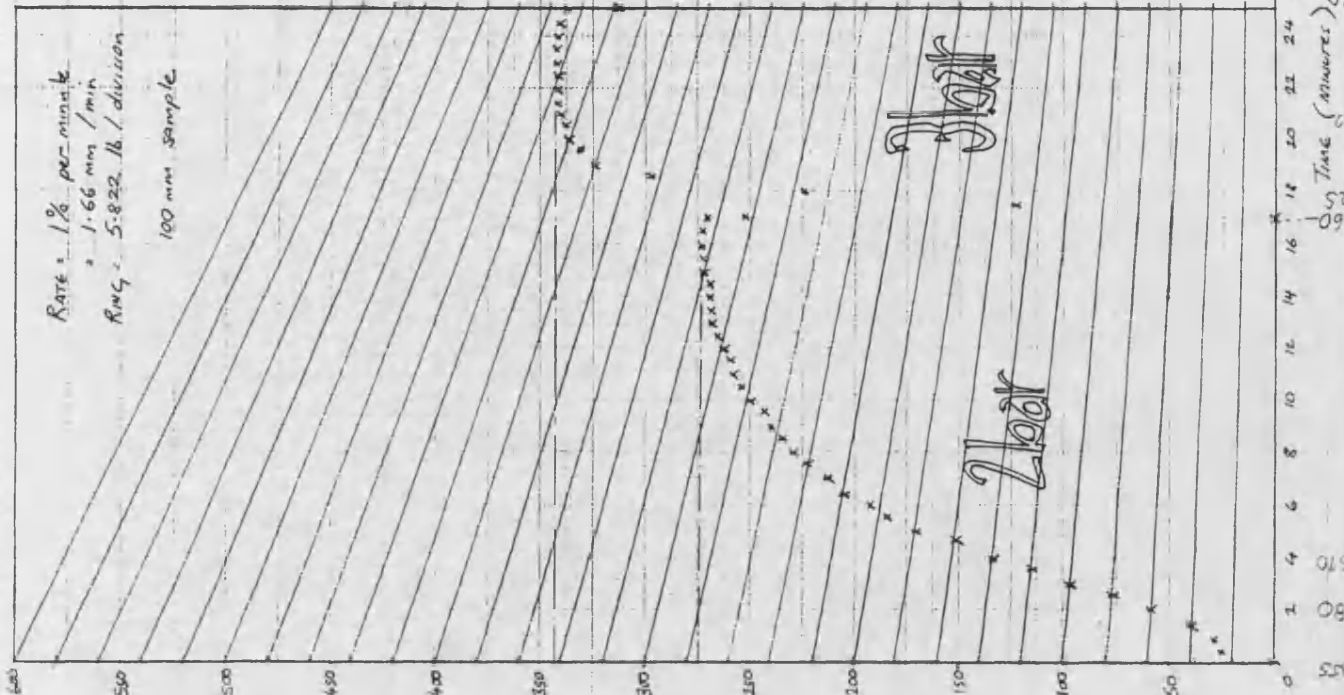




TEST 27 - 14-8-92 ; RD-30%

PROVING RING DIVISIONS

MULTI-STAGE - CLASS 2

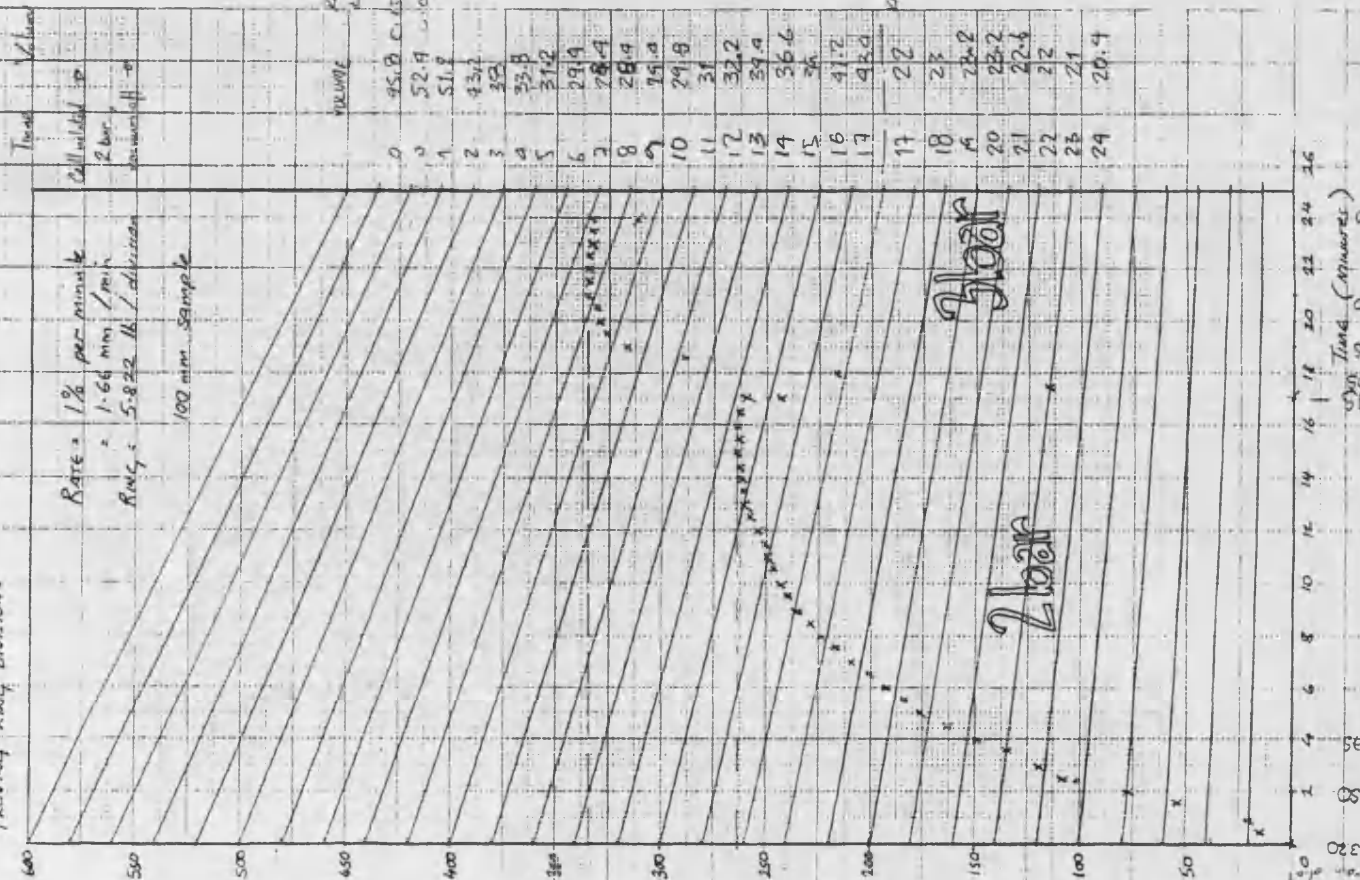


0 Calculated 26
 16 0.171 77.6
 20.0
 74.6
 70
 65
 29.2
 56
 54
 52.4
 52.2
 32.6
 54
 50.4
 57.2
 60
 62
 64.3
 63
 70.4
 49.4
 49.2
 48.6
 47
 50
 51
 51
 52

TEST 25 13-8-92 RD-50% - MULTI-STAGE CLASS 2

PROVING RING DIVISIONS

MULTI-STAGE CLASS 2



0 Calculated 26
 16 0.171 77.6
 20.0
 74.6
 70
 65
 29.2
 56
 54
 52.4
 52.2
 32.6
 54
 50.4
 57.2
 60
 62
 64.3
 63
 70.4
 49.4
 49.2
 48.6
 47
 50
 51
 51
 52

Test results for tests on large grained material in large cell, as detailed in section 8.4. Sample size - 254mm only.

Data output from ADU, utilising screen dumps and DIALOG software.

Includes;

1. 'Real time' plot of load response curve via DIALOG software (screen dump). Due to auto scaling these plots need to be read right to left.
2. Load vs displacement plot. XY plot from DIALOG software from logged data (screen dump).
3. Printout of all logged data. Time [secs], displacement [mm], load [KN] from non indicating load cell, and load [ADU units] from indicating load cell.
4. Calculation sheet for peak and '10%' friction angles.

CALCULATION SHEET FOR TEST Triax 2 and 2-2

Peak axial load difference [kN] = 48.5
Axial compression [mm] = 41.9
Cross - sectional area [m²] = 0.058
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 836
Cell pressure, σ_3 [kN/m²] = 250
Angle of friction, $\phi = 38.75^\circ$

} combination
giving
peak stress

Axial load difference at 10% axial strain [kN] = 43.19
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 767
Cell pressure, σ_3 [kN/m²] = 250
Angle of friction, $\phi = 37.27^\circ$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

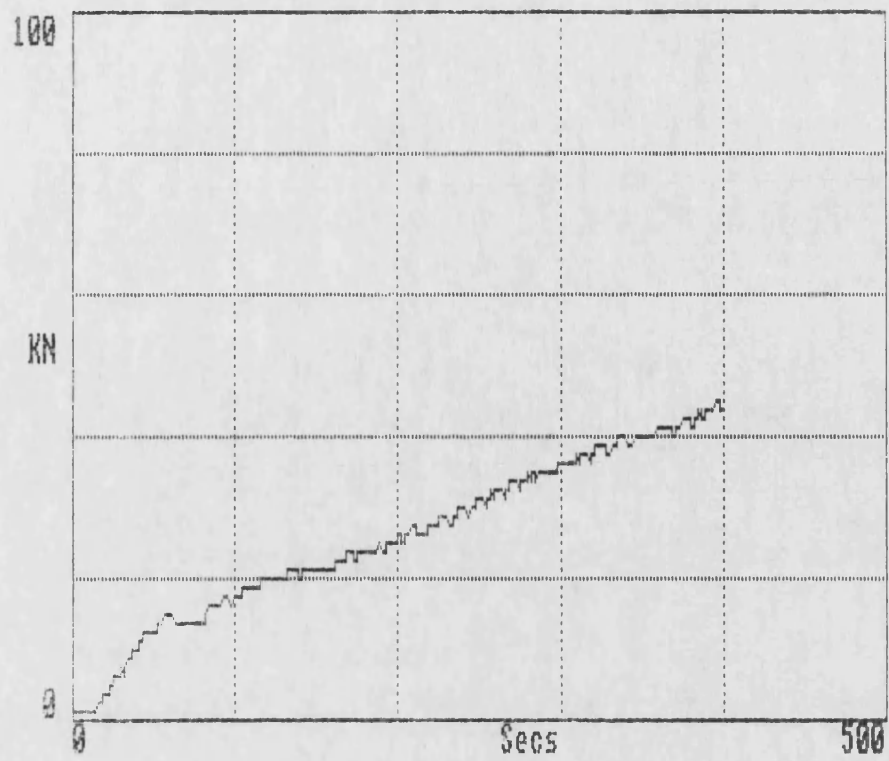
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

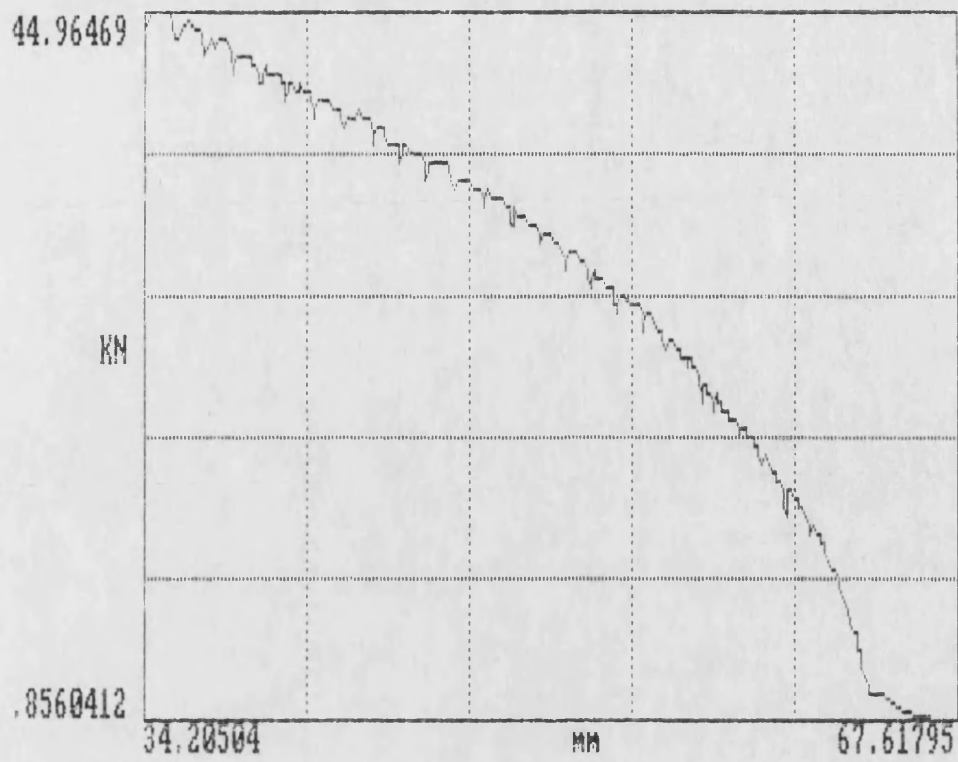
LOAD18 v Time



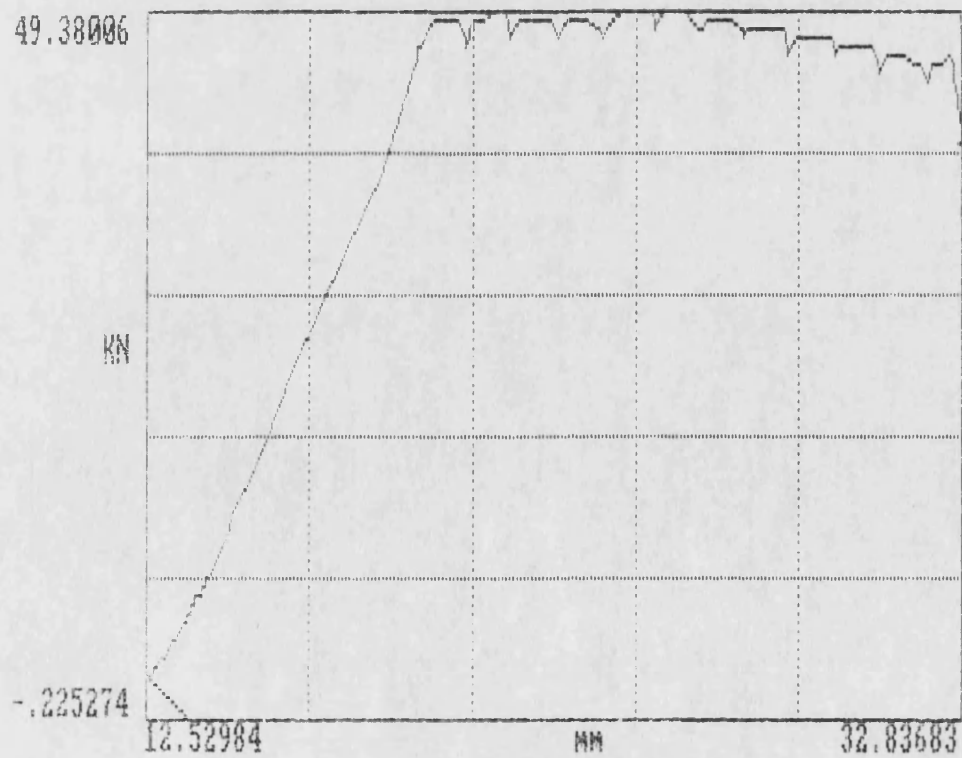
LOAD18 v Time



LOAD18 v disp17



LOAD18 v disp17



Date: 1st Started 01/01/83 No. Columns: 4
Time Task Started 17-52-17

Line	Secs	am	km	LOAD18	LOAD
1	2	3	4	5	6
0.0	67.546	0.855	18		
1.0	67.546	0.856	16		
2.0	67.546	0.856	16		
3.0	67.546	0.856	16		
4.0	67.546	0.856	16		
5.0	67.546	0.856	16		
6.0	67.546	0.856	16		
7.0	67.546	0.856	16		
8.0	67.546	0.856	16		
9.0	67.546	0.856	16		
10.0	67.546	0.856	16		
11.0	67.546	0.856	16		
12.0	67.546	0.856	16		
13.0	67.546	0.856	16		
14.0	67.546	0.856	16		
15.0	67.546	0.856	16		
16.0	67.546	0.856	16		
17.0	67.546	0.856	16		
18.0	67.546	0.856	16		
19.0	67.546	0.856	16		
20.0	67.546	0.856	16		
21.0	67.546	0.856	16		
22.0	67.546	0.856	16		
23.0	67.546	0.856	16		
24.0	67.546	0.856	16		
25.0	67.546	0.856	16		
26.0	67.546	0.856	16		
27.0	67.546	0.856	16		
28.0	67.546	0.856	16		
29.0	67.546	0.856	16		
30.0	67.546	0.856	16		
31.0	67.546	0.856	16		
32.0	67.546	0.856	16		
33.0	67.546	0.856	16		
34.0	67.546	0.856	16		
35.0	67.546	0.856	16		
36.0	67.546	0.856	16		
37.0	67.546	0.856	16		
38.0	67.546	0.856	16		
39.0	67.546	0.856	16		
40.0	67.546	0.856	16		
41.0	67.546	0.856	16		
42.0	67.546	0.856	16		
43.0	67.546	0.856	16		
44.0	67.546	0.856	16		
45.0	67.546	0.856	16		
46.0	67.546	0.856	16		
47.0	67.546	0.856	16		
48.0	67.546	0.856	16		
49.0	67.546	0.856	16		
50.0	67.546	0.856	16		

INMPL TEST/IRIAX2.DAT
Number of Readings: 400 No. Columns: 4
Time Task Started 17-52-17

Time	Secs	ma	km	LOAD18	LOAD
1	2	3	4	5	6
50.0	61.569	12.480	214		
51.0	61.497	12.300	211		
52.0	61.497	12.931	223		
53.0	61.353	13.652	237		
54.0	61.281	13.922	240		
55.0	61.137	14.237	245		
56.0	61.065	14.418	246		
57.0	60.921	14.688	253		
58.0	60.849	14.958	258		
59.0	60.705	15.183	261		
60.0	60.633	14.823	254		
61.0	60.633	14.598	250		
62.0	60.633	14.463	248		
63.0	60.633	14.327	247		
64.0	60.633	14.237	245		
65.0	60.561	14.147	243		
66.0	60.561	14.102	242		
67.0	60.561	14.012	242		
68.0	60.561	13.967	240		
69.0	60.561	13.922	239		
70.0	60.561	13.877	239		
71.0	60.561	13.877	239		
72.0	60.561	13.832	239		
73.0	60.561	13.787	237		
74.0	60.561	13.742	237		
75.0	60.561	13.742	235		
76.0	60.561	13.697	235		
77.0	60.561	13.697	235		
78.0	60.561	13.652	234		
79.0	60.561	13.652	235		
80.0	60.561	13.607	234		
81.0	60.489	14.147	243		
82.0	60.489	15.183	262		
83.0	60.345	15.859	272		
84.0	60.345	15.949	274		
85.0	60.273	15.814	269		
86.0	60.273	15.679	269		
87.0	60.201	15.724	270		
88.0	60.201	16.040	275		
89.0	60.067	16.445	284		
90.0	60.067	16.445	282		
91.0	59.913	16.490	287		
92.0	59.913	16.670	296		
93.0	59.841	16.896	296		
94.0	59.769	17.256	295		
95.0	59.697	17.346	296		
96.0	59.625	16.986	290		
97.0	59.625	16.760	284		
98.0	59.625	16.625	285		
99.0	59.793	15.941	291		
100.0	59.461	17.211	293		

TRIA101 TEST/TRIA02.DAT

Number of Readings 400 No. Columns 4
Date Task Started 01/01/83 Time Task Started 17-52-17

Time Secs	disp17 mm	LOAD18 KN	LOAD NON
1	2	3	4
102	101.0	59.481	17.394
103	102.0	59.481	17.571
104	103.0	59.409	17.797
105	104.0	59.337	18.022
106	105.0	59.265	18.247
107	106.0	59.193	18.427
108	107.0	59.049	18.472
109	108.0	58.977	18.698
110	109.0	58.905	18.923
111	110.0	58.833	19.103
112	111.0	58.761	19.193
113	112.0	58.689	19.148
114	113.0	58.617	18.833
115	114.0	58.617	18.653
116	115.0	58.617	18.923
117	116.0	58.545	19.374
118	117.0	58.473	19.464
119	118.0	58.473	19.419
120	119.0	58.401	19.644
121	120.0	58.329	19.644
122	121.0	58.329	19.599
123	122.0	58.257	19.689
124	123.0	58.257	19.779
125	124.0	58.185	19.914
126	125.0	58.185	20.094
127	126.0	58.113	20.320
128	127.0	58.041	20.365
129	128.0	58.041	20.410
130	129.0	57.969	20.410
131	130.0	57.897	20.365
132	131.0	57.897	20.500
133	132.0	57.825	20.635
134	133.0	57.825	20.770
135	134.0	57.752	20.950
136	135.0	57.752	20.996
137	136.0	57.680	21.041
138	137.0	57.608	20.905
139	138.0	57.608	20.680
140	139.0	57.536	20.500
141	140.0	57.536	20.410
142	141.0	57.536	20.500
143	142.0	57.536	20.860
144	143.0	57.464	20.996
145	144.0	57.464	21.041
146	145.0	57.464	21.086
147	146.0	57.392	21.086
148	147.0	57.392	21.176
149	148.0	57.392	21.221
150	149.0	57.320	21.401
151	150.0	57.320	21.536
152	151.0	57.248	21.671

TRIA101 TEST/TRIA02.DAT

Number of Readings 400 No. Columns 4
Date Task Started 01/01/83 Time Task Started 17-52-17

Time Secs	disp17 mm	LOAD18 KN	LOAD NON
1	2	3	4
153	152.0	57.248	21.807
154	153.0	57.176	21.536
155	154.0	57.176	21.356
156	155.0	57.176	21.266
157	156.0	57.176	21.131
158	157.0	57.104	21.041
159	158.0	57.104	20.950
160	159.0	57.104	20.860
161	160.0	57.104	20.815
162	161.0	57.104	21.266
163	162.0	57.032	21.942
164	163.0	57.032	22.392
165	164.0	56.960	22.572
166	165.0	56.960	22.618
167	166.0	56.888	22.708
168	167.0	56.888	22.843
169	168.0	56.816	22.978
170	169.0	56.744	23.158
171	170.0	56.672	23.203
172	171.0	56.600	23.203
173	172.0	56.600	23.158
174	173.0	56.528	22.888
175	174.0	56.528	22.708
176	175.0	56.528	22.753
177	176.0	56.456	23.428
178	177.0	56.384	23.699
179	178.0	56.384	23.654
180	179.0	56.312	23.654
181	180.0	56.240	23.699
182	181.0	56.168	23.834
183	182.0	56.168	23.789
184	183.0	56.096	23.744
185	184.0	56.024	24.014
186	185.0	56.024	24.104
187	186.0	55.952	24.375
188	187.0	55.808	24.375
189	188.0	55.808	24.420
190	189.0	55.736	24.555
191	190.0	55.664	24.285
192	191.0	55.664	24.014
193	192.0	55.592	23.834
194	193.0	55.592	24.194
195	194.0	55.520	24.645
196	195.0	55.448	25.005
197	196.0	55.448	25.005
198	197.0	55.376	25.050
199	198.0	55.304	25.251
200	199.0	55.232	25.456
201	200.0	55.088	25.681
202	201.0	55.016	26.132
203	202.0	54.800	26.087

***** TRIAXIAL TEST.DAT *****

Number of Readings 400 No. Columns 4
Date Task Started 01/01/83 Time Task Started 17-52-17

Time Secs	disp17 mm	LOAD18 KN	LOAD NON
1	2	3	4
204	203.0	54.728	25.456
205	204.0	54.728	25.321
206	205.0	54.656	26.177
207	206.0	54.512	26.537
208	207.0	54.368	26.808
209	208.0	54.224	26.898
210	209.0	54.008	27.168
211	210.0	53.864	27.438
212	211.0	53.792	27.123
213	212.0	53.720	26.627
214	213.0	53.720	26.357
215	214.0	53.648	26.177
216	215.0	53.646	26.042
217	216.0	53.648	26.357
218	217.0	53.576	26.672
219	218.0	53.576	26.672
220	219.0	53.504	26.988
221	220.0	53.432	27.619
222	221.0	53.360	27.844
223	222.0	53.288	27.889
224	223.0	53.216	27.934
225	224.0	53.144	27.979
226	225.0	53.072	28.294
227	226.0	52.928	28.430
228	227.0	52.928	28.385
229	228.0	52.784	28.655
230	229.0	52.712	28.880
231	230.0	52.640	28.339
232	231.0	52.568	28.024
233	232.0	52.568	27.844
234	233.0	52.568	27.799
235	234.0	52.496	26.526
236	235.0	52.424	29.060
237	236.0	52.352	29.286
238	237.0	52.208	29.556
239	238.0	52.064	29.781
240	239.0	51.920	30.067
241	240.0	51.776	30.142
242	241.0	51.632	30.187
243	242.0	51.560	29.511
244	243.0	51.488	29.196
245	244.0	51.488	29.331
246	245.0	51.416	30.142
247	246.0	51.272	30.367
248	247.0	51.200	30.592
249	248.0	51.056	30.772
250	249.0	50.911	30.998
251	250.0	50.767	31.088
252	251.0	50.623	31.153
253	252.0	50.551	31.313
254	253.0	50.407	30.727

***** TRIAXIAL TEST/TRIAX2.DAT *****

Number of Readings 400 No. Columns 4
Date Task Started 01/01/83 Time Task Started 17-52-17

Time Secs	disp17 mm	LOAD18 KN	LOAD NON
1	2	3	4
255	254.0	50.407	30.412
256	255.0	50.407	30.998
257	256.0	50.263	31.538
258	257.0	50.119	31.989
259	258.0	50.047	31.854
260	259.0	49.903	32.304
261	260.0	49.759	32.304
262	261.0	49.687	32.259
263	262.0	49.543	32.530
264	263.0	49.399	32.575
265	264.0	49.327	31.944
266	265.0	49.255	31.628
267	266.0	49.255	32.665
268	267.0	49.111	32.890
269	268.0	49.039	32.800
270	269.0	48.895	33.341
271	270.0	48.751	33.386
272	271.0	48.607	33.386
273	272.0	48.463	33.566
274	273.0	48.319	33.881
275	274.0	48.175	33.115
276	275.0	48.103	32.710
277	276.0	48.103	33.431
278	277.0	47.959	34.016
279	278.0	47.815	34.016
280	279.0	47.671	34.197
281	280.0	47.527	34.512
282	281.0	47.383	34.332
283	282.0	47.239	34.557
284	283.0	47.095	34.737
285	284.0	46.951	34.016
286	285.0	46.951	33.926
287	286.0	46.879	34.737
288	287.0	46.663	35.413
289	288.0	46.519	35.368
290	289.0	46.375	35.548
291	290.0	46.303	35.593
292	291.0	46.159	35.368
293	292.0	46.067	35.503
294	293.0	46.015	35.548
295	294.0	45.871	35.323
296	295.0	45.871	34.872
297	296.0	45.799	34.737
298	297.0	45.727	35.413
299	298.0	45.655	35.909
300	299.0	45.583	36.134
301	300.0	45.439	36.224
302	301.0	45.367	36.269
303	302.0	45.223	36.359
304	303.0	45.079	36.359
305	304.0	45.007	36.494

TRIAXIAL TEST/TRIAX2.DAT

Number of Readings 400 No. Columns 4
Date Task Started 01/01/83 Time Task Started 17-52-17

Time Secs	disp17 mm	LOAD18 KN	LOAD NON
1	2	3	4
306	305.0	44.865	616
307	306.0	44.791	604
308	307.0	44.719	599
309	308.0	44.719	602
310	309.0	44.647	613
311	310.0	44.503	617
312	311.0	44.359	616
313	312.0	44.287	617
314	313.0	44.143	626
315	314.0	44.070	629
316	315.0	43.854	634
317	316.0	43.710	632
318	317.0	43.638	623
319	318.0	43.566	616
320	319.0	43.566	620
321	320.0	43.422	639
322	321.0	43.278	640
323	322.0	43.134	639
324	323.0	42.990	646
325	324.0	42.845	642
326	325.0	42.702	644
327	326.0	42.558	646
328	327.0	42.486	642
329	328.0	42.414	633
330	329.0	42.342	627
331	330.0	42.342	631
332	331.0	42.270	643
333	332.0	42.198	648
334	333.0	42.126	650
335	334.0	41.982	655
336	335.0	41.838	659
337	336.0	41.766	659
338	337.0	41.622	659
339	338.0	41.478	659
340	339.0	41.405	660
341	340.0	41.334	657
342	341.0	41.262	648
343	342.0	41.190	642
344	343.0	41.190	641
345	344.0	41.118	652
346	345.0	41.046	660
347	346.0	40.974	664
348	347.0	40.902	664
349	348.0	40.830	665
350	349.0	40.686	668
351	350.0	40.614	673
352	351.0	40.542	671
353	352.0	40.398	672
354	353.0	40.326	674
355	354.0	40.182	675
356	355.0	40.110	668

TRIAXIAL TEST/TRIAX2.DAT

Number of Readings 400 No. Columns 4
Date Task Started 01/01/83 Time Task Started 17-52-17

Time Secs	disp17 mm	LOAD18 KN	LOAD NON
1	2	3	4
357	356.0	40.038	661
358	357.0	40.038	659
359	358.0	39.966	682
360	359.0	39.822	680
361	360.0	39.750	687
362	361.0	39.606	687
363	362.0	39.462	685
364	363.0	39.390	682
365	364.0	39.318	684
366	365.0	39.246	687
367	366.0	39.174	691
368	367.0	39.102	686
369	368.0	39.030	680
370	369.0	38.958	674
371	370.0	38.958	673
372	371.0	38.886	691
373	372.0	38.814	698
374	373.0	38.670	699
375	374.0	38.598	701
376	375.0	38.454	701
377	376.0	38.310	700
378	377.0	38.166	705
379	378.0	38.094	706
380	379.0	37.950	703
381	380.0	37.878	692
382	381.0	37.806	689
383	382.0	37.734	712
384	383.0	37.518	720
385	384.0	37.301	724
386	385.0	37.085	717
387	386.0	36.869	727
388	387.0	36.725	709
389	388.0	36.653	703
390	389.0	36.509	731
391	390.0	36.293	733
392	391.0	36.005	736
393	392.0	35.717	735
394	393.0	35.573	720
395	394.0	35.429	726
396	395.0	35.213	750
397	396.0	34.781	749
398	397.0	34.493	745
399	398.0	34.277	730
400	399.0	34.205	720

* 10% strain

TRIAXIAL TEST/TRIAX2-2.DAT

Number of Readings 215 No. Columns 4
Date Task Started 01/01/83 Time Task Started 18-03-51

Time Secs	displ mm	LOAD18 KN	LOAD NON
1	2	3	4
0	0.0	32.837	40.369 673
1	0.0	32.837	40.369 673
2	1.0	32.837	40.414 673
3	2.0	32.837	40.369 672
4	3.0	32.837	40.369 674
5	4.0	32.837	40.369 673
6	5.0	32.837	40.369 673
7	6.0	32.837	40.369 672
8	7.0	32.837	40.369 673
9	8.0	32.837	40.369 673
10	9.0	32.837	40.369 673
11	10.0	32.837	40.369 674
12	11.0	32.837	40.369 673
13	12.0	32.837	40.369 673
14	13.0	32.837	40.369 673
15	14.0	32.837	40.369 673
16	15.0	32.837	40.369 673
17	16.0	32.837	40.324 672
18	17.0	32.837	40.324 672
19	18.0	32.837	40.324 672
20	19.0	32.837	40.369 672
21	20.0	32.837	40.324 672
22	21.0	32.837	40.324 673
23	22.0	32.837	40.369 673
24	23.0	32.837	40.324 672
25	24.0	32.837	40.324 672
26	25.0	32.837	40.324 672
27	26.0	32.837	40.324 672
28	27.0	32.837	40.324 673
29	28.0	32.837	40.324 672
30	29.0	32.837	40.324 672
31	30.0	32.837	40.324 672
32	31.0	32.837	40.324 671
33	32.0	32.837	40.324 672
34	33.0	32.765	40.324 671
35	34.0	32.837	40.324 672
36	35.0	32.837	40.324 671
37	36.0	32.837	40.324 672
38	37.0	32.837	40.324 672
39	38.0	32.837	40.324 671
40	39.0	32.837	40.324 671
41	40.0	32.837	40.279 672
42	41.0	32.837	40.324 671
43	42.0	32.837	40.279 671
44	43.0	32.765	40.279 672
45	44.0	32.837	40.324 671
46	45.0	32.837	40.324 672
47	46.0	32.837	40.324 671
48	47.0	32.837	40.324 672
49	48.0	32.837	40.279 672
50	49.0	32.837	40.324 672

TRIAXIAL TEST/TRIAX2-2.DAT

Number of Readings 215 No. Columns 4
Date Task Started 01/01/83 Time Task Started 18-03-51

Time Secs	displ mm	LOAD18 KN	LOAD NON
1	2	3	4
51	50.0	32.837	40.279 671
52	51.0	32.837	40.279 672
53	52.0	32.837	40.955 684
54	53.0	32.765	42.261 706
55	54.0	32.765	43.253 721
56	55.0	32.693	44.469 742
57	56.0	32.693	45.686 760
58	57.0	32.549	46.001 764
59	58.0	32.405	45.866 763
60	59.0	32.261	45.686 759
61	60.0	32.117	45.550 755
62	61.0	32.045	44.694 743
63	62.0	31.973	44.694 744
64	63.0	31.829	45.595 759
65	64.0	31.685	45.731 760
66	65.0	31.469	45.046 766
67	66.0	31.325	44.136 767
68	67.0	31.181	44.271 768
69	68.0	31.037	44.136 767
70	69.0	30.893	45.641 757
71	70.0	30.821	45.100 749
72	71.0	30.749	46.181 766
73	72.0	30.605	46.722 777
74	73.0	30.460	46.722 775
75	74.0	30.316	47.172 784
76	75.0	30.100	47.127 783
77	76.0	29.956	47.172 783
78	77.0	29.812	46.677 774
79	78.0	29.740	46.001 763
80	79.0	29.596	47.398 789
81	80.0	29.380	47.758 793
82	81.0	29.159	47.802 793
83	82.0	28.940	47.533 789
84	83.0	28.804	47.533 789
85	84.0	28.568	47.172 781
86	85.0	28.516	46.452 770
87	86.0	28.372	48.073 800
88	87.0	28.156	48.344 802
89	88.0	27.862	48.299 800
90	89.0	27.724	48.164 799
91	90.0	27.508	48.254 800
92	91.0	27.415	47.308 784
93	92.0	27.369	47.848 795
94	93.0	27.149	48.659 807
95	94.0	26.866	48.704 809
96	95.0	26.716	48.524 805
97	96.0	26.501	48.569 805
98	97.0	26.351	47.892 793
99	98.0	26.204	47.848 794
100	99.0	26.160	49.020 813
101	100.0	25.926	49.155 815

File: 134 / 191677 - 2.1ed

Number of Readings: 215 No. Columns: 4
Date Task Started: 01/01/83 Date Task Started: 18-03-81

Time Secs	disp17 mm	LOAD18 KN	LOAD MM
1	2	3	4
102	101.0	25.636	49.335
103	102.0	25.420	49.155
104	103.0	25.276	48.975
105	104.0	25.132	48.073
106	105.0	25.060	49.110
107	106.0	24.772	49.290
108	107.0	24.412	49.290
109	108.0	24.196	48.659
110	109.0	23.980	48.454
111	110.0	23.908	47.443
112	111.0	23.764	48.254
113	112.0	23.475	48.975
114	113.0	23.259	48.614
115	114.0	23.043	48.524
116	115.0	22.899	47.938
117	116.0	22.755	47.262
118	117.0	22.611	48.749
119	118.0	22.323	48.975
120	119.0	22.107	48.859
121	120.0	21.891	48.704
122	121.0	21.675	48.344
123	122.0	21.603	47.533
124	123.0	21.459	49.110
125	124.0	21.171	49.380
126	125.0	20.955	48.859
127	126.0	20.739	48.614
128	127.0	20.595	48.254
129	128.0	20.523	47.485
130	129.0	20.451	45.992
131	130.0	20.451	47.353
132	131.0	20.367	48.479
133	132.0	20.091	49.065
134	133.0	19.875	48.749
135	134.0	19.731	48.614
136	135.0	19.587	48.344
137	136.0	19.443	47.713
138	137.0	19.371	47.172
139	138.0	19.299	46.722
140	139.0	19.299	46.561
141	140.0	18.363	37.891
142	141.0	17.355	31.403
143	142.0	16.058	23.474
144	143.0	15.410	18.518
145	144.0	14.978	15.534
146	145.0	14.690	13.787
147	146.0	14.546	12.615
148	147.0	14.402	11.759
149	148.0	14.258	11.033
150	149.0	14.186	10.492
151	150.0	14.114	10.002
152	151.0	14.042	9.597

* peak

Number of Readings: 215 No. Columns: 4
Date Task Started: 01/01/83 Date Task Started: 18-03-81

Time Secs	disp17 mm	LOAD18 KN	LOAD MM
1	2	3	4
153	152.0	13.970	9.325
154	153.0	13.898	9.011
155	154.0	13.898	8.741
156	155.0	13.826	8.515
157	156.0	13.826	8.535
158	157.0	13.754	8.155
159	158.0	13.754	7.885
160	159.0	13.682	7.794
161	160.0	13.682	7.614
162	161.0	13.682	7.524
163	162.0	13.610	7.434
164	163.0	13.610	7.344
165	164.0	13.538	7.209
166	165.0	13.538	6.938
167	166.0	13.538	6.848
168	167.0	13.538	6.803
169	168.0	13.466	6.713
170	169.0	13.466	6.668
171	170.0	13.466	6.623
172	171.0	13.466	6.443
173	172.0	13.394	6.263
174	173.0	13.322	5.902
175	174.0	13.250	5.477
176	175.0	13.178	5.091
177	176.0	13.106	4.641
178	177.0	13.034	4.415
179	178.0	12.962	4.235
180	179.0	12.890	4.055
181	180.0	12.890	3.675
182	181.0	12.818	3.740
183	182.0	12.818	3.559
184	183.0	12.746	3.424
185	184.0	12.746	3.289
186	185.0	12.674	3.199
187	186.0	12.674	3.109
188	187.0	12.674	2.974
189	188.0	12.602	2.884
190	189.0	12.530	2.793
191	190.0	12.530	2.658
192	191.0	13.610	-0.225
193	192.0	13.682	-0.180
194	193.0	13.682	-0.180
195	194.0	13.682	-0.180
196	195.0	13.682	-0.180
197	196.0	13.682	-0.180
198	197.0	13.682	-0.180
199	198.0	13.754	-0.180
200	199.0	13.754	-0.180
201	200.0	13.754	-0.180
202	201.0	13.754	-0.180
203	202.0	13.754	-0.180

Number of Readings 215 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 18-03-51

Time Secs	disp17 mm	LOAD16 KN	LOAD MM
	1	2	3
294	203.0	13.754	-0.180
295	204.0	13.754	-0.180
296	205.0	13.754	-0.180
297	206.0	13.682	-0.180
298	207.0	13.754	-0.180
299	208.0	13.754	-0.180
300	209.0	13.754	-0.180
301	210.0	13.754	-0.180
302	211.0	13.754	-0.180
303	212.0	13.754	-0.180
304	213.0	13.754	-0.180
305	214.0	13.754	-0.180

CALCULATION SHEET FOR TEST Triax 3

Peak axial load difference [kN] = 51.858
Axial compression [mm] = 25.852
Cross - sectional area [m²] = 0.05498
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 943
Cell pressure, σ_3 [kN/m²] = 380
Angle of friction, $\phi = 33.62^\circ$

} combination
giving
peak stress

Axial load difference at 10% axial strain [kN] =
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] =
Cell pressure, σ_3 [kN/m²] =
Angle of friction, $\phi =$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

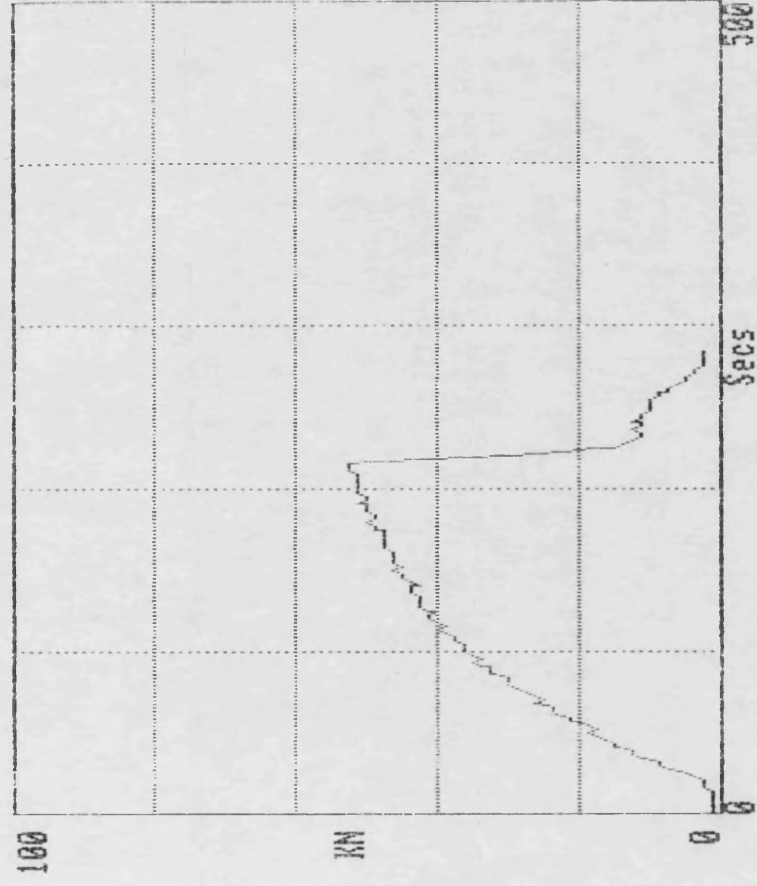
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

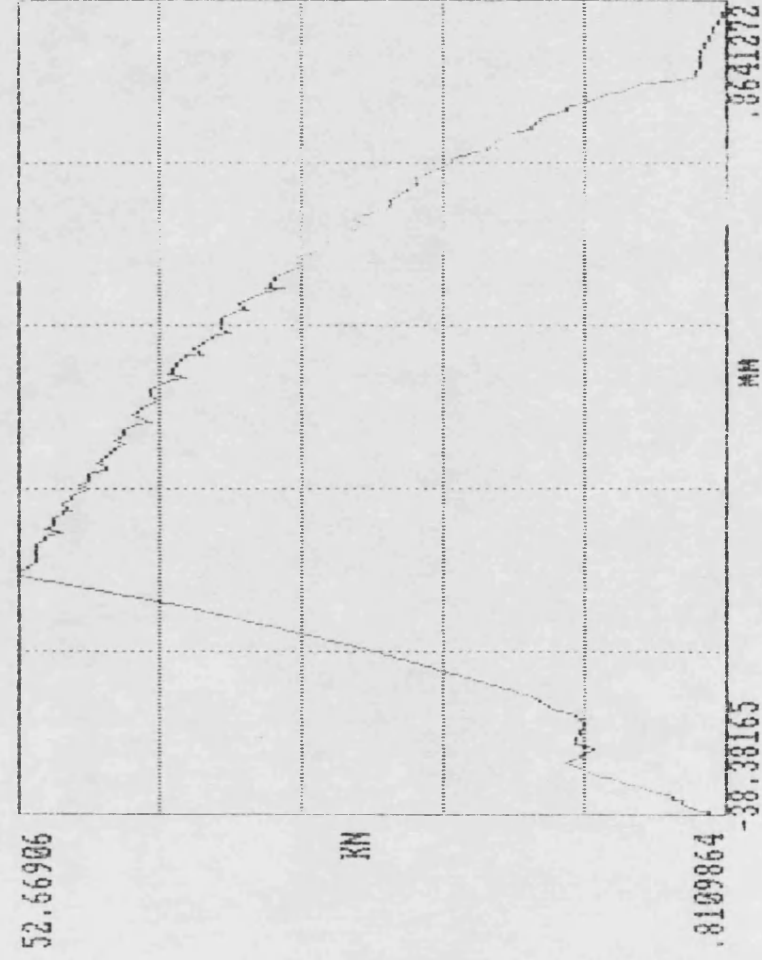
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v disp17



Number of Readings 142 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-16-54

Time Secs 1	disp17 mm 2	LOAD18 KN 3	LOAD NON 4
0	0.0	0.864	0.811 16
1	2.0	0.864	0.811 16
2	4.0	0.864	0.811 17
3	6.0	0.864	0.811 16
4	8.0	0.864	0.811 16
5	10.0	0.864	0.811 16
6	12.0	0.864	0.811 16
7	14.0	0.864	0.811 16
8	16.0	-1.656	2.613 45
9	18.0	-2.160	2.974 51
10	20.0	-2.232	2.838 48
11	22.0	-2.808	3.379 56
12	24.0	-3.096	5.001 84
13	26.0	-3.096	4.731 79
14	28.0	-3.312	6.983 117
15	30.0	-3.601	8.876 146
16	32.0	-3.673	9.056 149
17	34.0	-4.105	11.489 188
18	36.0	-4.393	12.255 198
19	38.0	-4.537	12.165 200
20	40.0	-4.897	14.463 235
21	42.0	-5.185	15.319 248
22	44.0	-5.401	14.868 239
23	46.0	-5.545	16.535 267
24	48.0	-5.905	17.752 286
25	50.0	-6.193	18.247 292
26	52.0	-6.265	17.481 281
27	54.0	-6.409	19.058 307
28	56.0	-6.769	20.770 332
29	58.0	-7.057	20.635 331
30	60.0	-7.201	21.491 344
31	62.0	-7.633	23.248 372
32	64.0	-8.065	23.383 372
33	66.0	-8.209	24.104 386
34	68.0	-8.713	25.681 410
35	70.0	-9.073	25.591 408
36	72.0	-9.217	26.132 419
37	74.0	-9.649	28.024 446
38	76.0	-10.081	27.664 440
39	78.0	-10.153	28.339 454
40	80.0	-10.586	30.097 479
41	82.0	-11.018	30.052 478
42	84.0	-11.162	29.961 478
43	86.0	-11.594	32.394 516
44	88.0	-12.026	32.034 508
45	90.0	-12.170	32.530 519
46	92.0	-12.458	33.926 539
47	94.0	-12.962	34.422 546
48	96.0	-13.106	33.250 527
49	98.0	-13.322	35.233 560
50	100.0	-13.754	36.269 576

Number of Readings 142 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-16-54

Time Secs 1	disp17 mm 2	LOAD18 KN 3	LOAD NON 4
51	102.0	-14.114	35.728 567
52	104.0	-14.186	36.179 574
53	106.0	-14.546	37.531 597
54	108.0	-14.978	38.026 604
55	110.0	-15.194	37.260 591
56	112.0	-15.410	38.657 616
57	114.0	-15.842	39.828 632
58	116.0	-16.202	39.063 619
59	118.0	-16.346	40.144 640
60	120.0	-16.706	41.225 654
61	122.0	-17.067	41.631 661
62	124.0	-17.283	40.324 640
63	126.0	-17.499	41.766 664
64	128.0	-17.859	42.802 680
65	130.0	-18.291	42.757 679
66	132.0	-18.435	42.397 675
67	134.0	-18.651	42.667 678
68	136.0	-18.795	43.433 690
69	138.0	-19.083	43.928 698
70	140.0	-19.371	43.613 692
71	142.0	-19.515	42.712 678
72	144.0	-19.587	44.244 704
73	146.0	-19.875	44.649 709
74	148.0	-20.091	45.010 716
75	150.0	-20.307	45.776 727
76	152.0	-20.523	44.604 707
77	154.0	-20.739	45.821 728
78	156.0	-21.027	46.497 739
79	158.0	-21.387	46.812 743
80	160.0	-21.603	46.497 738
81	162.0	-21.747	46.091 733
82	164.0	-21.963	47.217 750
83	166.0	-22.107	47.353 751
84	168.0	-22.323	47.803 760
85	170.0	-22.539	48.119 764
86	172.0	-22.683	47.308 750
87	174.0	-22.827	48.073 765
88	176.0	-22.971	48.479 771
89	178.0	-23.187	49.065 779
90	180.0	-23.475	49.380 785
91	182.0	-23.691	48.704 773
92	184.0	-23.836	49.245 783
93	186.0	-23.980	49.560 788
94	188.0	-24.196	50.056 794
95	190.0	-24.412	50.146 796
96	192.0	-24.700	50.777 806
97	194.0	-24.844	49.695 788
98	196.0	-24.988	50.597 804
99	198.0	-25.132	50.912 808
100	200.0	-25.420	51.408 815
101	202.0	-25.636	51.408 818

Number of Readings 142 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-16-54

Time Secs	displ7 mm	LOAD18 KN	LOAD NON
1	2	3	4
102	204.0	-25.852	51.137 811
103	206.0	-25.924	51.272 815
104	208.0	-26.140	51.588 818
105	210.0	-26.284	51.678 820
106	212.0	-26.428	52.128 828
107	214.0	-26.716	52.669 836 * peak stress
108	216.0	-27.004	51.948 823
109	218.0	-28.444	40.234 622
110	220.0	-30.244	28.430 443
111	222.0	-31.541	21.086 330
112	224.0	-32.333	17.076 268
113	226.0	-32.837	14.508 230
114	228.0	-33.413	13.381 212
115	230.0	-33.629	11.940 189
116	232.0	-33.845	11.038 175
117	234.0	-33.917	10.903 175
118	236.0	-34.637	12.030 191
119	238.0	-35.069	11.534 183
120	240.0	-35.141	10.768 171
121	242.0	-35.861	12.345 198
122	244.0	-36.293	11.489 182
123	246.0	-36.365	10.768 171
124	248.0	-36.437	10.227 163
125	250.0	-36.509	9.957 158
126	252.0	-36.581	9.687 155
127	254.0	-36.581	9.462 151
128	256.0	-36.653	9.146 146
129	258.0	-36.725	8.696 139
130	260.0	-36.869	8.020 128
131	262.0	-37.013	7.119 112
132	264.0	-37.301	5.902 94
133	266.0	-37.518	4.956 81
134	268.0	-37.590	4.551 74
135	270.0	-37.662	4.190 67
136	272.0	-37.806	3.830 62
137	274.0	-38.166	2.568 42
138	276.0	-38.238	2.388 40
139	278.0	-38.238	2.253 38
140	280.0	-38.310	2.118 37
141	282.0	-38.382	2.027 35
142	284.0	-38.382	1.937 33

CALCULATION SHEET FOR TEST Triax 4

Peak axial load difference [kN] = 55.688
Axial compression [mm] = 31.18

} combination
giving
peak stress

Cross - sectional area [m²] = 0.056

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 995

Cell pressure, σ_3 [kN/m²] = 400

Angle of friction, $\phi = 33.66^\circ$

Axial load difference at 10% axial strain [kN] =

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] =

Cell pressure, σ_3 [kN/m²] =

Angle of friction, $\phi =$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

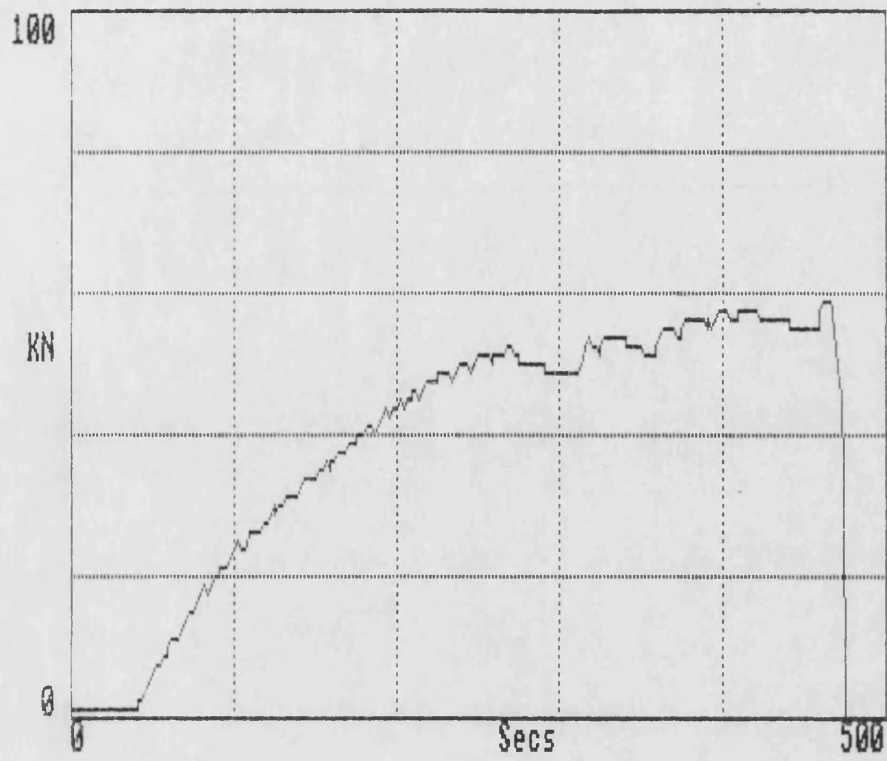
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

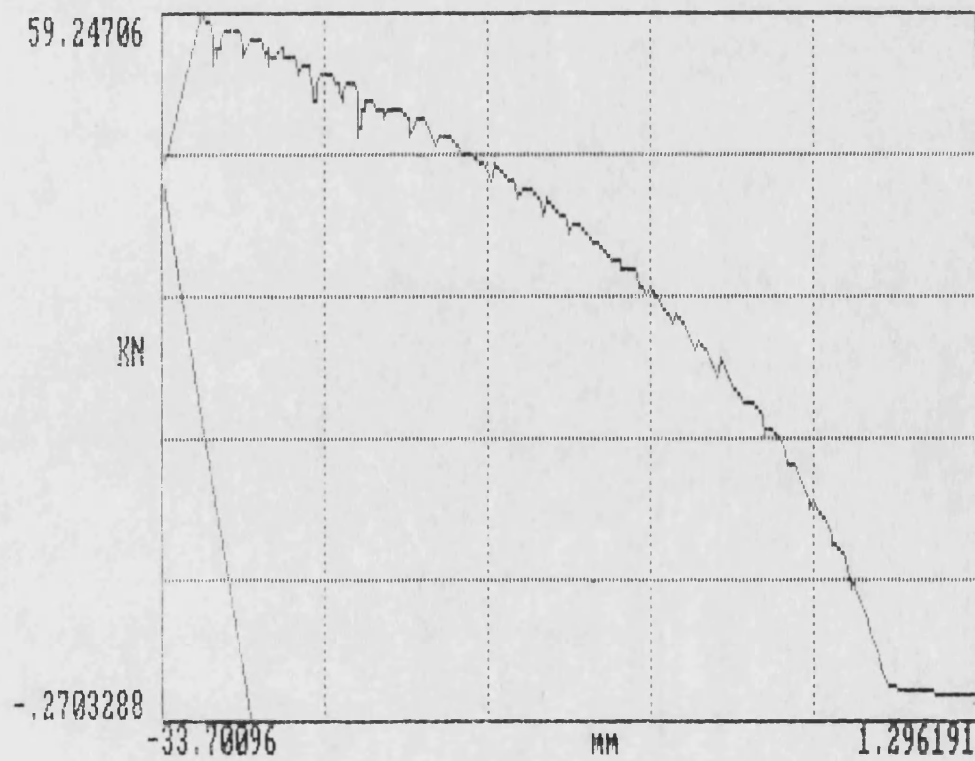
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v displ7



UNIAXIAL TEST/TRIAX4.DAT

Number of Readings 249 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-24-21

Time	disp17	LOAD18	LOAD
Secs	mm	KN	NON
1	2	3	4
0	0.0	1.224	1.622 26
1	2.0	1.224	1.622 26
2	4.0	1.224	1.622 24
3	6.0	1.224	1.622 24
4	8.0	1.224	1.577 25
5	10.0	1.224	1.622 26
6	12.0	1.224	1.622 26
7	14.0	1.224	1.622 26
8	16.0	1.224	1.622 25
9	18.0	1.224	1.622 26
10	20.0	1.224	1.622 25
11	22.0	1.224	1.622 25
12	24.0	1.224	1.622 26
13	26.0	1.224	1.622 25
14	28.0	1.224	1.622 24
15	30.0	1.224	1.622 25
16	32.0	1.224	1.622 25
17	34.0	1.224	1.622 25
18	36.0	1.224	1.622 25
19	38.0	1.224	1.622 25
20	40.0	1.296	1.622 24
21	42.0	0.720	1.937 31
22	44.0	-2.592	3.064 48
23	46.0	-2.736	3.785 55
24	48.0	-2.808	4.460 66
25	50.0	-3.024	5.767 84
26	52.0	-3.240	7.389 107
27	54.0	-3.384	7.479 105
28	56.0	-3.457	8.290 119
29	58.0	-3.601	9.326 132
30	60.0	-3.745	10.092 143
31	62.0	-3.889	10.723 152
32	64.0	-4.033	11.399 159
33	66.0	-4.105	11.174 157
34	68.0	-4.249	12.525 176
35	70.0	-4.465	13.877 196
36	72.0	-4.753	14.913 211
37	74.0	-4.897	14.733 206
38	76.0	-4.969	15.544 218
39	78.0	-5.257	16.625 234
40	80.0	-5.473	17.436 245
41	82.0	-5.689	18.382 259
42	84.0	-5.833	17.932 251
43	86.0	-5.905	18.427 257
44	88.0	-6.121	19.734 277
45	90.0	-6.337	20.545 287
46	92.0	-6.553	20.996 293
47	94.0	-6.769	21.401 298
48	96.0	-6.841	21.491 300
49	98.0	-6.985	22.753 318
50	100.0	-7.201	23.699 331

TRIAXIAL TEST/TRIAX4.DAT

Number of Readings 249 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-24-21

Time	disp17	LOAD18	LOAD
Secs	mm	KN	NON
1	2	3	4
51	102.0	-7.489	24.600 344
52	104.0	-7.705	24.014 333
53	106.0	-7.705	23.879 333
54	108.0	-7.921	25.636 357
55	110.0	-8.209	26.177 364
56	112.0	-8.425	26.808 373
57	114.0	-8.713	26.627 370
58	116.0	-8.785	26.808 374
59	118.0	-8.929	27.619 384
60	120.0	-9.145	28.069 390
61	122.0	-9.361	29.060 405
62	124.0	-9.649	29.871 415
63	126.0	-9.793	28.565 397
64	128.0	-9.937	29.646 412
65	130.0	-10.153	30.457 423
66	132.0	-10.370	31.086 433
67	134.0	-10.586	31.583 440
68	136.0	-10.730	31.223 432
69	138.0	-10.802	31.268 435
70	140.0	-10.946	32.079 446
71	142.0	-11.090	32.710 455
72	144.0	-11.306	33.386 464
73	146.0	-11.594	33.971 471
74	148.0	-11.810	33.205 461
75	150.0	-11.954	34.016 474
76	152.0	-12.170	34.827 485
77	154.0	-12.386	35.323 492
78	156.0	-12.674	35.909 500
79	158.0	-12.890	35.458 491
80	160.0	-13.034	35.999 500
81	162.0	-13.178	36.449 507
82	164.0	-13.394	37.305 517
83	166.0	-13.682	37.801 526
84	168.0	-13.970	37.305 517
85	170.0	-14.042	38.161 530
86	172.0	-14.330	38.747 538
87	174.0	-14.618	39.243 545
88	176.0	-14.906	39.964 554
89	178.0	-15.050	39.603 552
90	180.0	-15.410	40.549 564
91	182.0	-15.770	41.045 570
92	184.0	-16.058	41.631 578
93	186.0	-16.202	40.459 560
94	188.0	-16.346	41.811 581
95	190.0	-16.562	42.171 586
96	192.0	-16.850	42.937 597
97	194.0	-17.139	43.388 602
98	196.0	-17.283	42.036 583
99	198.0	-17.427	43.433 603
100	200.0	-17.787	44.334 616
101	202.0	-18.147	44.739 620

TRIAXIAL TEST/IRIAX4.DAT

Number of Readings 249 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-24-21

Time Secs	1	disp17 mm	2	LOAD18 KN	3	LOAD NON	4
102	204.0	-18.435	43.973	609			
103	206.0	-18.579	44.875	622			
104	208.0	-18.795	45.190	626			
105	210.0	-19.083	46.046	639			
106	212.0	-19.443	46.497	645			
107	214.0	-19.587	45.145	625			
108	216.0	-19.803	46.406	643			
109	218.0	-20.019	46.947	650			
110	220.0	-20.307	47.713	662			
111	222.0	-20.667	47.308	654			
112	224.0	-20.739	47.217	655			
113	226.0	-20.955	48.209	667			
114	228.0	-21.315	48.659	673			
115	230.0	-21.603	48.794	677			
116	232.0	-21.819	48.614	673			
117	234.0	-21.891	48.073	667			
118	236.0	-22.107	49.020	678			
119	238.0	-22.251	49.425	685			
120	240.0	-22.467	50.011	692			
121	242.0	-22.683	50.101	694			
122	244.0	-22.899	49.876	691			
123	246.0	-22.971	49.065	679			
124	248.0	-23.115	50.506	700			
125	250.0	-23.331	50.912	704			
126	252.0	-23.619	51.047	706			
127	254.0	-23.836	51.002	707			
128	256.0	-23.980	51.227	710			
129	258.0	-24.124	50.281	695			
130	260.0	-24.196	51.137	708			
131	262.0	-24.340	51.182	709			
132	264.0	-24.484	51.362	711			
133	266.0	-24.556	51.543	713			
134	268.0	-24.700	51.993	721			
135	270.0	-24.916	52.038	721			
136	272.0	-24.988	51.272	709			
137	274.0	-25.060	50.777	704			
138	276.0	-25.132	50.461	699			
139	278.0	-25.132	50.236	695			
140	280.0	-25.132	50.011	692			
141	282.0	-25.132	49.876	690			
142	284.0	-25.132	49.741	689			
143	286.0	-25.132	49.605	688			
144	288.0	-25.204	49.515	685			
145	290.0	-25.204	49.425	685			
146	292.0	-25.204	49.335	682			
147	294.0	-25.204	49.245	681			
148	296.0	-25.204	49.200	680			
149	298.0	-25.204	49.116	681			
150	300.0	-25.204	49.065	679			
151	302.0	-25.276	49.020	680			
152	304.0	-25.276	48.975	676			

TRIAXIAL TEST/IRIAX4.DAT

Number of Readings 249 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-24-21

Time Secs	1	disp17 mm	2	LOAD18 KN	3	LOAD NON	4
153	306.0	-25.276	48.884	679			
154	308.0	-25.276	48.839	678			
155	310.0	-25.276	48.794	677			
156	312.0	-25.276	48.794	676			
157	314.0	-25.276	50.056	696			
158	316.0	-25.348	52.264	726			
159	318.0	-25.492	53.255	737			
160	320.0	-25.708	53.029	733			
161	322.0	-25.852	52.534	727			
162	324.0	-25.924	51.858	718			
163	326.0	-25.996	52.714	731			
164	328.0	-26.140	53.300	738			
165	330.0	-26.284	53.525	742			
166	332.0	-26.428	53.840	746			
167	334.0	-26.572	53.840	746			
168	336.0	-26.716	54.111	749			
169	338.0	-26.932	54.246	752			
170	340.0	-27.004	53.390	739			
171	342.0	-27.004	52.894	732			
172	344.0	-27.076	52.534	727			
173	346.0	-27.076	52.264	724			
174	348.0	-27.076	52.083	721			
175	350.0	-27.148	51.948	719			
176	352.0	-27.148	51.768	718			
177	354.0	-27.148	51.678	716			
178	356.0	-27.148	51.543	715			
179	358.0	-27.148	51.453	714			
180	360.0	-27.220	52.714	732			
181	362.0	-27.292	53.795	745			
182	364.0	-27.292	54.426	754			
183	366.0	-27.436	54.697	757			
184	368.0	-27.580	54.922	761			
185	370.0	-27.724	54.967	761			
186	372.0	-27.868	54.246	751			
187	374.0	-27.868	53.795	745			
188	376.0	-28.012	55.282	766			
189	378.0	-28.156	55.688	770			
190	380.0	-28.372	55.643	770			
191	382.0	-28.516	56.048	775			
192	384.0	-28.732	55.823	773			
193	386.0	-28.876	55.823	773			
194	388.0	-29.020	55.778	771			
195	390.0	-29.020	55.057	763			
196	392.0	-29.164	56.319	780			
197	394.0	-29.308	55.508	768			
198	396.0	-29.452	56.724	786			
199	398.0	-29.596	57.039	789			
200	400.0	-29.740	57.084	790			
201	402.0	-29.956	57.310	793			
202	404.0	-30.100	56.454	781			
203	406.0	-30.172	55.823	773			

* Peak

Number of Readings 249 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-24-21

	Time Secs	displ? mm	LOAD18 KN	LOAD NON
	1	2	3	4
204	408.0	-30.172	56.273	781
205	410.0	-30.316	57.490	795
206	412.0	-30.532	57.445	794
207	414.0	-30.677	57.625	798
208	416.0	-30.821	57.895	802
209	418.0	-30.965	57.805	800
210	420.0	-31.109	57.220	792
211	422.0	-31.181	56.679	785
212	424.0	-31.181	56.319	779
213	426.0	-31.253	56.048	777
214	428.0	-31.253	55.868	774
215	430.0	-31.325	56.589	784
216	432.0	-31.325	56.364	780
217	434.0	-31.325	56.183	777
218	436.0	-31.397	56.003	775
219	438.0	-31.397	55.868	772
220	440.0	-31.397	55.733	771
221	442.0	-31.397	55.598	770
222	444.0	-31.469	55.508	768
223	446.0	-31.469	55.417	767
224	448.0	-31.469	55.327	766
225	450.0	-31.469	55.237	765
226	452.0	-31.469	55.192	765
227	454.0	-31.469	55.102	762
228	456.0	-31.469	55.057	763
229	458.0	-31.469	55.012	761
230	460.0	-31.541	56.949	788
231	462.0	-31.613	58.211	807
232	464.0	-31.757	58.751	813
233	466.0	-31.901	59.247	821
234	468.0	-32.765	53.615	736
235	470.0	-33.413	48.209	665
236	472.0	-33.701	45.550	630
237	474.0	-29.884	-0.090	-3
238	476.0	-22.539	-0.270	1
239	478.0	-22.539	-0.270	1
240	480.0	-22.539	-0.270	0
241	482.0	-22.539	-0.225	3
242	484.0	-28.516	-0.225	3
243	486.0	-30.532	-0.225	2
244	488.0	-30.532	-0.225	2
245	490.0	-30.532	-0.225	3
246	492.0	-30.532	-0.225	3
247	494.0	-30.460	-0.225	3
248	496.0	-30.460	-0.225	3
249	498.0	-30.460	-0.225	1

CALCULATION SHEET FOR TEST Triax 5

Peak axial load difference [kN] = 63.572
Axial compression [mm] = 35.72
Cross - sectional area [m²] = 0.0568
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 1119
Cell pressure, σ_3 [kN/m²] = 400
Angle of friction, $\phi = 35.67^\circ$

} combination
giving
peak stress

Axial load difference at 10% axial strain [kN] = 59.70
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 1060
Cell pressure, σ_3 [kN/m²] = 400
Angle of friction, $\phi = 34.74^\circ$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

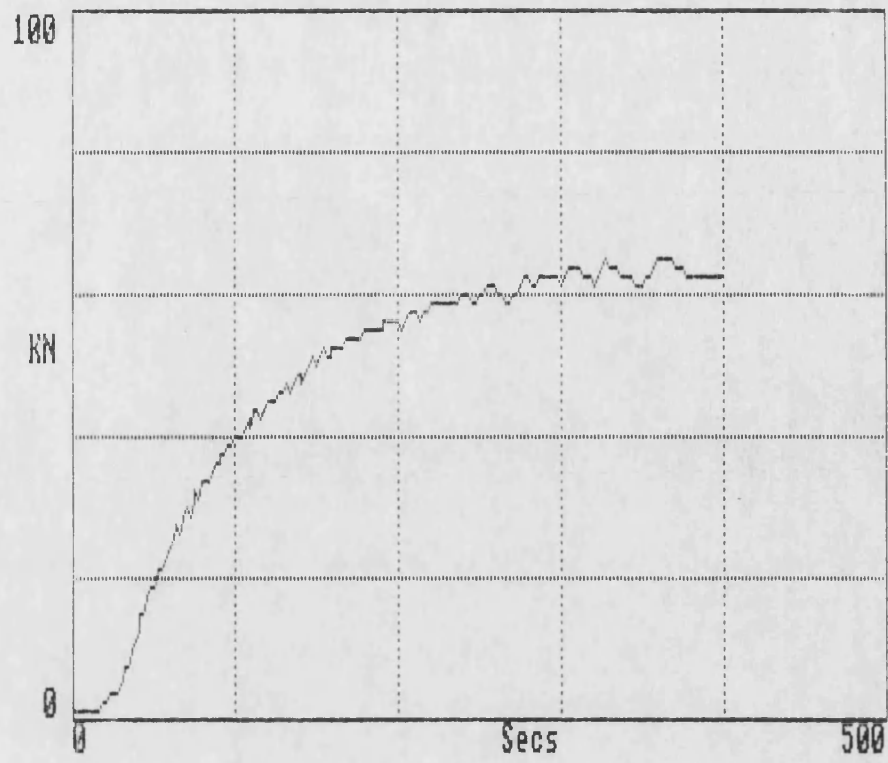
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

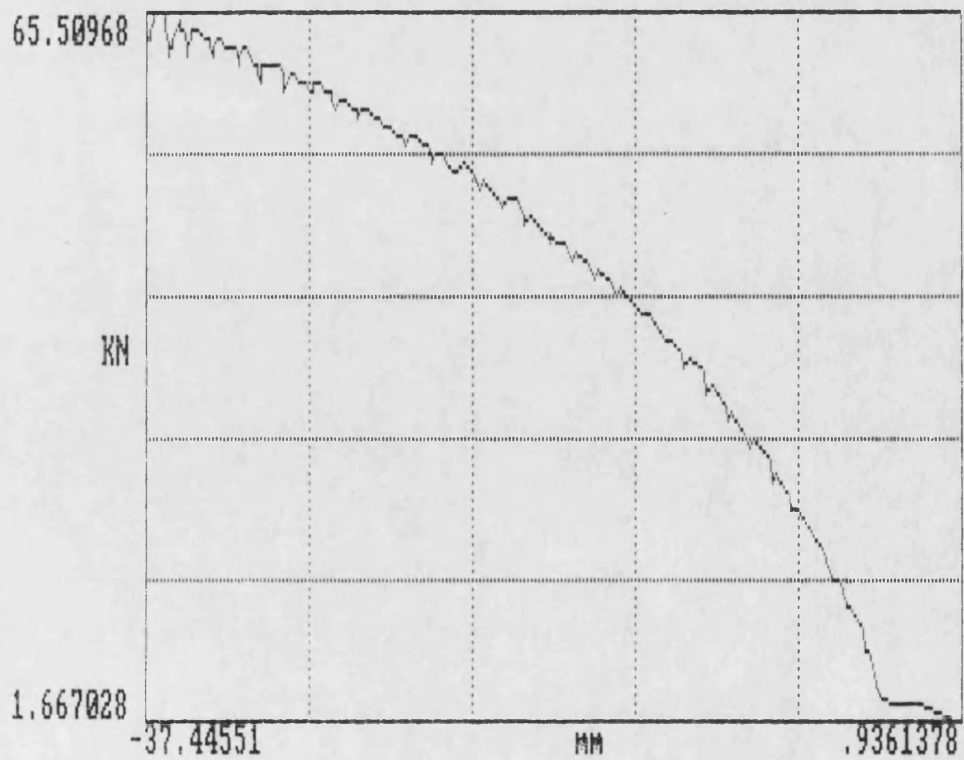
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v disp17



=====

Number of Readings 200 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-21-21
=====

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
0	0.0	0.936	1.667
1	2.0	0.936	1.667
2	4.0	0.936	1.667
3	6.0	0.936	1.667
4	8.0	0.936	1.667
5	10.0	0.936	1.667
6	12.0	0.936	1.667
7	14.0	0.936	1.667
8	16.0	0.936	1.667
9	18.0	0.864	1.982
10	20.0	-0.936	2.884
11	22.0	-1.512	3.379
12	24.0	-1.656	3.154
13	26.0	-2.232	3.649
14	28.0	-2.952	3.920
15	30.0	-3.096	5.091
16	32.0	-3.457	7.930
17	34.0	-3.529	7.704
18	36.0	-3.817	10.272
19	38.0	-4.321	11.849
20	40.0	-4.465	12.255
21	42.0	-4.897	14.823
22	44.0	-5.257	14.778
23	46.0	-5.617	17.256
24	48.0	-6.121	18.382
25	50.0	-6.265	18.878
26	52.0	-6.697	21.176
27	54.0	-7.057	20.860
28	56.0	-7.273	22.302
29	58.0	-7.705	24.149
30	60.0	-7.993	23.519
31	62.0	-8.209	25.501
32	64.0	-8.785	27.078
33	66.0	-9.073	26.087
34	68.0	-9.289	28.385
35	70.0	-9.793	29.556
36	72.0	-10.009	28.700
37	74.0	-10.226	30.682
38	76.0	-10.730	31.989
39	78.0	-11.090	30.998
40	80.0	-11.306	33.205
41	82.0	-11.738	34.287
42	84.0	-12.098	33.521
43	86.0	-12.314	35.188
44	88.0	-12.674	36.269
45	90.0	-13.034	36.044
46	92.0	-13.250	37.035
47	94.0	-13.754	37.981
48	96.0	-14.114	38.206
49	98.0	-14.330	38.927
50	100.0	-14.762	40.009

=====

Number of Readings 200 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-21-21
=====

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
51	102.0	-15.194	40.414
52	104.0	-15.338	40.324
53	106.0	-15.698	41.541
54	108.0	-16.130	42.352
55	110.0	-16.346	41.270
56	112.0	-16.706	43.343
57	114.0	-17.139	43.838
58	116.0	-17.427	42.892
59	118.0	-17.571	43.793
60	120.0	-17.787	44.514
61	122.0	-18.075	45.145
62	124.0	-18.363	45.595
63	126.0	-18.507	44.604
64	128.0	-18.651	45.956
65	130.0	-18.939	46.722
66	132.0	-19.299	47.308
67	134.0	-19.587	46.542
68	136.0	-19.659	47.308
69	138.0	-19.947	48.479
70	140.0	-20.379	48.839
71	142.0	-20.667	47.893
72	144.0	-20.883	48.930
73	146.0	-21.171	49.876
74	148.0	-21.531	50.687
75	150.0	-21.819	49.425
76	152.0	-22.035	50.957
77	154.0	-22.395	52.038
78	156.0	-22.827	51.408
79	158.0	-23.043	51.408
80	160.0	-23.259	52.309
81	162.0	-23.547	52.759
82	164.0	-23.836	53.029
83	166.0	-24.052	52.219
84	168.0	-24.268	53.165
85	170.0	-24.412	53.750
86	172.0	-24.772	54.066
87	174.0	-24.988	54.066
88	176.0	-25.204	53.615
89	178.0	-25.276	53.210
90	180.0	-25.420	54.426
91	182.0	-25.636	54.471
92	184.0	-25.852	55.282
93	186.0	-26.068	55.462
94	188.0	-26.284	55.192
95	190.0	-26.356	54.742
96	192.0	-26.500	55.823
97	194.0	-26.644	56.003
98	196.0	-26.932	56.409
99	198.0	-27.148	56.544
100	200.0	-27.364	56.679
101	202.0	-27.508	55.553

TRIAXIAL TEST/TRIAX5.DAT

Number of Readings 200 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-21-21

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
102	204.0	-27.580	56.634
103	206.0	-27.724	57.039
104	208.0	-27.940	57.715
105	210.0	-28.228	57.805
106	212.0	-28.444	57.715
107	214.0	-28.588	56.769
108	216.0	-28.660	57.175
109	218.0	-28.804	57.940
110	220.0	-28.948	58.211
111	222.0	-29.092	58.661
112	224.0	-29.308	58.797
113	226.0	-29.524	58.977
114	228.0	-29.668	58.346
115	230.0	-29.740	58.571
116	232.0	-29.884	58.887
117	234.0	-29.956	59.022
118	236.0	-30.100	59.202
119	238.0	-30.244	59.472
120	240.0	-30.460	59.698
121	242.0	-30.605	59.698
122	244.0	-30.749	59.517
123	246.0	-30.893	58.751
124	248.0	-30.965	58.616
125	250.0	-31.037	59.653
126	252.0	-31.109	60.464
127	254.0	-31.325	60.734
128	256.0	-31.469	60.734
129	258.0	-31.685	60.734
130	260.0	-31.829	60.689
131	262.0	-31.973	60.328
132	264.0	-32.045	59.653
133	266.0	-32.117	59.157
134	268.0	-32.117	58.842
135	270.0	-32.189	59.833
136	272.0	-32.261	60.599
137	274.0	-32.333	60.959
138	276.0	-32.477	61.815
139	278.0	-32.693	61.995
140	280.0	-32.909	62.131
141	282.0	-33.053	61.365
142	284.0	-33.125	60.689
143	286.0	-33.197	61.950
144	288.0	-33.341	62.446
145	290.0	-33.557	62.401
146	292.0	-33.701	62.671
147	294.0	-33.917	62.851
148	296.0	-34.133	62.987
149	298.0	-34.277	62.040
150	300.0	-34.349	61.365
151	302.0	-34.493	62.446
152	304.0	-34.565	63.167

10% strain

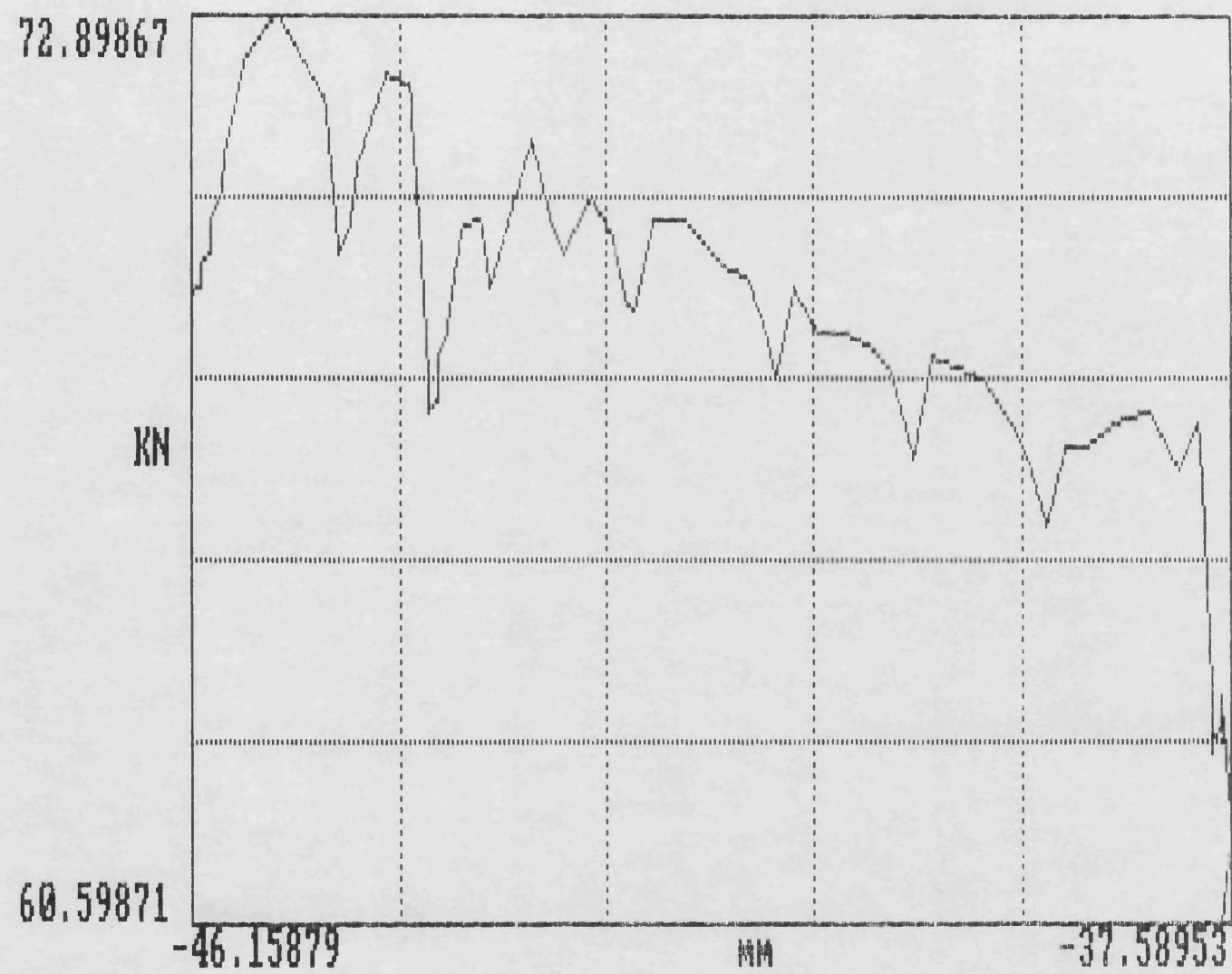
TRIAXIAL TEST/TRIAX5.DAT

Number of Readings 200 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-21-21

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
153	306.0	-34.709	63.302
154	308.0	-34.925	63.753
155	310.0	-35.069	63.527
156	312.0	-35.285	63.933
157	314.0	-35.429	63.077
158	316.0	-35.501	62.446
159	318.0	-35.501	62.040
160	320.0	-35.573	61.725
161	322.0	-35.573	62.671
162	324.0	-35.645	63.572
163	326.0	-35.789	63.978
164	328.0	-35.933	64.428
165	330.0	-36.005	63.978
166	332.0	-36.077	63.888
167	334.0	-36.149	63.302
168	336.0	-36.221	62.897
169	338.0	-36.221	62.536
170	340.0	-36.293	62.311
171	342.0	-36.293	62.086
172	344.0	-36.365	61.950
173	346.0	-36.365	61.770
174	348.0	-36.365	61.680
175	350.0	-36.365	61.545
176	352.0	-36.365	61.950
177	354.0	-36.437	62.626
178	356.0	-36.509	64.023
179	358.0	-36.509	64.744
180	360.0	-36.653	65.239
181	362.0	-36.797	65.375
182	364.0	-36.869	65.375
183	366.0	-37.013	65.510
184	368.0	-37.157	64.609
185	370.0	-37.229	64.113
186	372.0	-37.229	63.753
187	374.0	-37.301	63.437
188	376.0	-37.301	63.257
189	378.0	-37.301	63.077
190	380.0	-37.374	62.942
191	382.0	-37.374	62.806
192	384.0	-37.374	62.671
193	386.0	-37.374	62.581
194	388.0	-37.374	62.491
195	390.0	-37.446	62.401
196	392.0	-37.446	62.266
197	394.0	-37.446	62.221
198	396.0	-37.446	62.176
199	398.0	-37.446	62.086
200	400.0	-37.446	62.040

Peak

LOAD18 v displ7



Number of Readings 87 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-30-00

Time Secs	displ7 mm	LOAD18 KN	LOAD NON
1	2	3	4
0	0.0	-37.590	60.689 507
1	2.0	-37.590	60.689 502
2	4.0	-37.590	60.644 504
3	6.0	-37.590	60.644 503
4	8.0	-37.590	60.644 499
5	10.0	-37.662	60.599 501
6	12.0	-37.590	61.860 514
7	14.0	-37.662	63.572 525
8	16.0	-37.662	63.617 526
9	18.0	-37.662	63.572 525
10	20.0	-37.662	63.482 525
11	22.0	-37.662	63.437 525
12	24.0	-37.662	63.392 525
13	26.0	-37.662	63.347 524
14	28.0	-37.662	63.257 523
15	30.0	-37.662	63.212 525
16	32.0	-37.662	63.167 523
17	34.0	-37.734	63.122 522
18	36.0	-37.734	63.077 521
19	38.0	-37.734	63.032 520
20	40.0	-37.734	62.987 522
21	42.0	-37.734	62.942 523
22	44.0	-37.734	63.482 525
23	46.0	-37.806	66.411 550
24	48.0	-37.878	67.312 555
25	50.0	-38.022	66.726 552
26	52.0	-38.238	67.537 561
27	54.0	-38.526	67.402 558
28	56.0	-38.742	67.042 556
29	58.0	-38.958	67.132 559
30	60.0	-39.102	65.960 546
31	62.0	-39.246	66.951 556
32	64.0	-39.390	67.357 557
33	66.0	-39.606	68.033 563
34	68.0	-39.822	66.123 562
35	70.0	-40.038	68.258 567
36	72.0	-40.182	66.951 554
37	74.0	-40.398	68.078 564
38	76.0	-40.542	68.393 567
39	78.0	-40.758	68.664 568
40	80.0	-40.974	68.618 567
41	82.0	-41.190	69.249 571
42	84.0	-41.334	67.988 562
43	86.0	-41.406	68.664 569
44	88.0	-41.550	69.339 576
45	90.0	-41.766	69.520 575
46	92.0	-42.054	70.060 579
47	94.0	-42.342	70.060 579
48	96.0	-42.486	68.844 568
49	98.0	-42.558	68.979 570
50	100.0	-42.702	69.925 578

TRIAXIAL TEST/TRIAX5-2.DAT

Number of Readings 87 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-30-00

Time Secs	displ7 mm	LOAD18 KN	LOAD NON
1	2	3	4
51	102.0	-42.846	70.421 583
52	104.0	-43.062	69.745 577
53	106.0	-43.206	70.105 579
54	108.0	-43.350	71.142 587
55	110.0	-43.566	70.195 578
56	112.0	-43.710	69.249 571
57	114.0	-43.782	70.150 578
58	116.0	-43.926	69.970 577
59	118.0	-43.998	69.249 574
60	120.0	-44.070	68.799 570
61	122.0	-44.070	68.528 567
62	124.0	-44.143	68.258 564
63	126.0	-44.143	68.033 563
64	128.0	-44.143	67.898 560
65	130.0	-44.143	67.717 557
66	132.0	-44.143	67.627 558
67	134.0	-44.215	67.492 557
68	136.0	-44.215	67.762 559
69	138.0	-44.287	69.925 577
70	140.0	-44.359	71.953 598
71	142.0	-44.575	72.133 597
72	144.0	-44.791	70.826 586
73	146.0	-44.863	69.970 578
74	148.0	-44.935	69.745 576
75	150.0	-45.079	71.772 593
76	152.0	-45.223	72.223 595
77	154.0	-45.439	72.899 601
78	156.0	-45.727	72.223 595
79	158.0	-45.871	71.051 585
80	160.0	-45.943	70.466 581
81	162.0	-46.015	70.060 579
82	164.0	-46.015	69.745 575
83	166.0	-46.087	69.520 574
84	168.0	-46.087	69.294 572
85	170.0	-46.087	69.159 572
86	172.0	-46.159	69.069 572
87	174.0	-46.159	68.889 570

CALCULATION SHEET FOR TEST Triax 6

Peak axial load difference [kN] = 48.98
Axial compression [mm] = 40.04
Cross - sectional area [m²] = 0.0576
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 850
Cell pressure, σ_3 [kN/m²] = 200
Angle of friction, $\phi = 42.83^\circ$

} combination
giving
peak stress

Axial load difference at 10% axial strain [kN] = 45.06
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 800
Cell pressure, σ_3 [kN/m²] = 200
Angle of friction, $\phi = 41.81^\circ$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

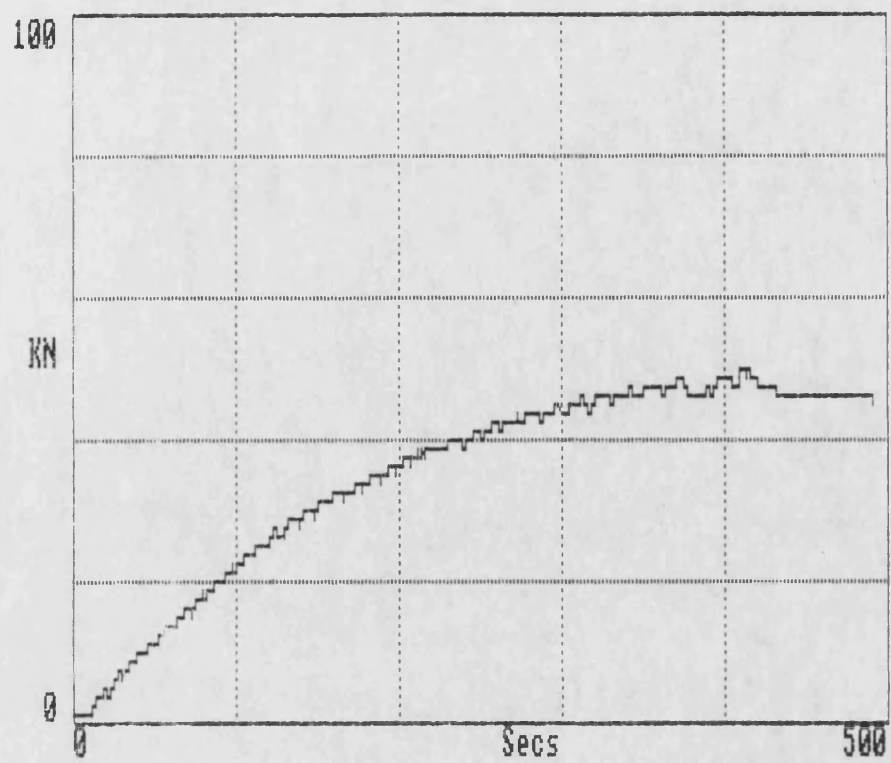
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

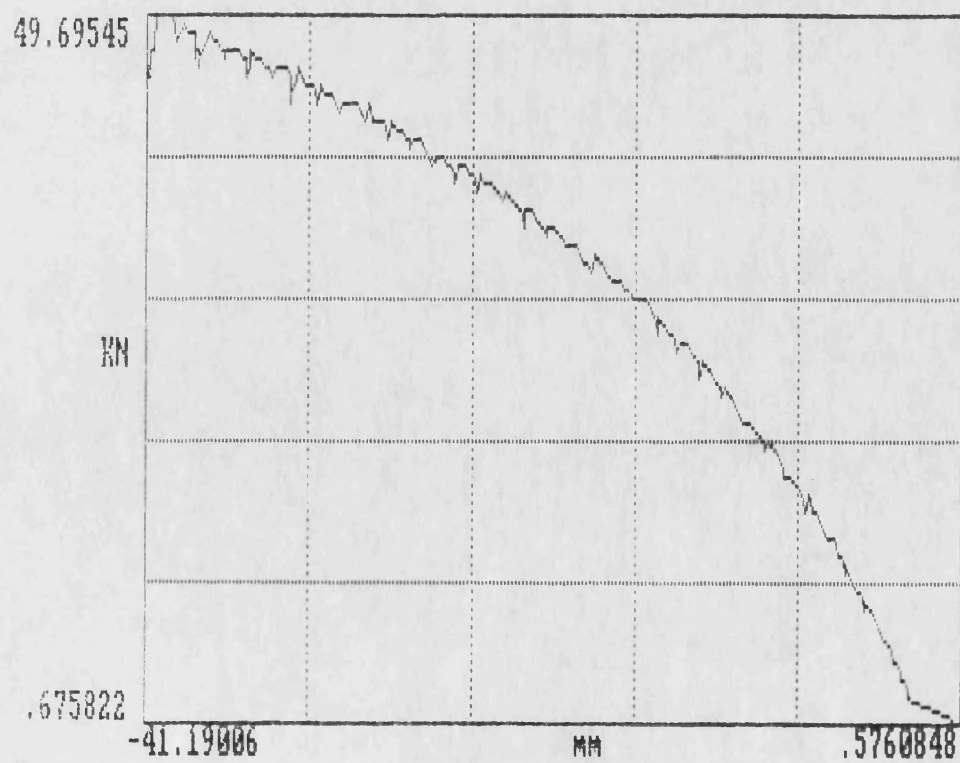
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v disp17



***** TEST INFORMATION *****

Number of Readings 331 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-46-44

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
0	0.0	0.576	0.721 13
1	2.0	0.576	0.721 12
2	4.0	0.576	0.721 13
3	6.0	0.576	0.676 13
4	8.0	0.576	0.721 13
5	10.0	0.576	0.721 12
6	12.0	0.576	0.721 13
7	14.0	0.576	0.721 13
8	16.0	0.576	0.721 13
9	18.0	0.576	0.721 13
10	20.0	0.072	1.577 29
11	22.0	-2.016	2.478 43
12	24.0	-2.160	3.064 52
13	26.0	-2.304	3.559 60
14	28.0	-2.448	3.965 65
15	30.0	-2.520	4.280 71
16	32.0	-2.664	4.776 81
17	34.0	-2.808	5.181 87
18	36.0	-3.024	6.082 102
19	38.0	-3.384	6.983 116
20	40.0	-3.457	6.848 113
21	42.0	-3.673	7.659 128
22	44.0	-3.961	8.470 140
23	46.0	-4.321	9.236 152
24	48.0	-4.393	8.876 145
25	50.0	-4.537	9.687 161
26	52.0	-4.753	10.137 166
27	54.0	-4.897	10.453 172
28	56.0	-5.113	11.174 183
29	58.0	-5.329	11.444 187
30	60.0	-5.401	11.309 185
31	62.0	-5.545	12.255 201
32	64.0	-5.689	12.525 206
33	66.0	-5.833	13.336 219
34	68.0	-6.121	13.832 226
35	70.0	-6.265	13.652 223
36	72.0	-6.409	14.282 234
37	74.0	-6.625	14.913 244
38	76.0	-6.841	15.138 249
39	78.0	-6.985	15.589 255
40	80.0	-7.273	16.355 267
41	82.0	-7.417	15.589 255
42	84.0	-7.561	16.670 273
43	86.0	-7.777	17.301 284
44	88.0	-8.137	17.797 291
45	90.0	-8.497	17.707 289
46	92.0	-8.569	18.427 302
47	94.0	-8.785	18.923 309
48	96.0	-8.929	19.374 317
49	98.0	-9.217	20.139 330
50	100.0	-9.433	19.599 319

TRIAXIAL TEST/TRIAX6.DAT

Number of Readings 331 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-46-44

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
51	102.0	-9.577	20.365 333
52	104.0	-9.793	21.041 344
53	106.0	-10.153	21.807 357
54	108.0	-10.586	21.716 353
55	110.0	-10.730	22.122 362
56	112.0	-10.946	22.933 375
57	114.0	-11.162	23.248 380
58	116.0	-11.522	23.834 389
59	118.0	-11.738	23.248 378
60	120.0	-11.810	24.059 393
61	122.0	-12.098	24.645 402
62	124.0	-12.314	25.186 412
63	126.0	-12.674	25.546 416
64	128.0	-12.818	24.825 404
65	130.0	-12.962	26.087 425
66	132.0	-13.250	26.357 430
67	134.0	-13.538	26.988 442
68	136.0	-13.898	26.853 437
69	138.0	-13.970	26.402 431
70	140.0	-14.186	27.619 451
71	142.0	-14.330	27.754 453
72	144.0	-14.618	28.520 466
73	146.0	-14.906	28.880 472
74	148.0	-15.050	27.934 456
75	150.0	-15.122	28.835 472
76	152.0	-15.338	29.421 481
77	154.0	-15.626	30.097 493
78	156.0	-16.058	29.871 486
79	158.0	-16.202	29.961 491
80	160.0	-16.418	30.637 501
81	162.0	-16.634	30.998 508
82	164.0	-16.922	31.268 511
83	166.0	-17.211	31.178 509
84	168.0	-17.355	31.133 509
85	170.0	-17.571	31.944 523
86	172.0	-17.859	32.394 531
87	174.0	-18.219	32.935 539
88	176.0	-18.435	31.764 518
89	178.0	-18.579	32.710 536
90	180.0	-18.723	32.800 536
91	182.0	-18.939	33.386 546
92	184.0	-19.227	33.791 554
93	186.0	-19.515	33.566 548
94	188.0	-19.659	33.566 549
95	190.0	-19.803	33.881 555
96	192.0	-19.947	34.377 563
97	194.0	-20.307	35.053 574
98	196.0	-20.595	35.278 578
99	198.0	-20.739	34.152 559
100	200.0	-20.955	35.278 577
101	202.0	-21.171	35.458 580

=====				
Number of Readings 331 No. Columns 4				
Date Task Started 01/01/83 Time Task Started 00-46-44				
=====				
Time	displ7	LOAD18	LOAD	
Secs	mm	KN	NON	
1	2	3	4	

102	204.0	-21.387	36.089	591
103	206.0	-21.819	36.269	594
104	208.0	-21.891	35.233	576
105	210.0	-22.107	36.314	594
106	212.0	-22.323	36.810	603
107	214.0	-22.683	37.305	610
108	216.0	-23.043	36.765	602
109	218.0	-23.187	37.395	613
110	220.0	-23.475	37.891	622
111	222.0	-23.836	38.116	625
112	224.0	-24.124	38.387	629
113	226.0	-24.268	37.305	611
114	228.0	-24.412	38.702	634
115	230.0	-24.700	38.927	638
116	232.0	-24.916	39.153	642
117	234.0	-25.204	39.108	641
118	236.0	-25.348	38.297	627
119	238.0	-25.420	38.927	640
120	240.0	-25.636	39.378	646
121	242.0	-25.780	39.558	650
122	244.0	-26.068	39.874	654
123	246.0	-26.284	40.144	659
124	248.0	-26.500	39.423	647
125	250.0	-26.572	39.333	647
126	252.0	-26.716	40.144	660
127	254.0	-26.932	40.324	662
128	256.0	-27.148	41.090	676
129	258.0	-27.436	41.315	679
130	260.0	-27.652	40.504	664
131	262.0	-27.796	41.135	677
132	264.0	-27.940	41.405	681
133	266.0	-28.156	41.856	688
134	268.0	-28.372	41.946	688
135	270.0	-28.660	42.397	695
136	272.0	-28.804	41.315	679
137	274.0	-28.948	42.261	696
138	276.0	-29.164	42.306	695
139	278.0	-29.380	42.532	699
140	280.0	-29.596	42.622	700
141	282.0	-29.884	43.388	713
142	284.0	-30.028	42.171	693
143	286.0	-30.172	42.892	706
144	288.0	-30.316	43.298	712
145	290.0	-30.605	43.748	719
146	292.0	-30.821	43.433	714
147	294.0	-31.109	43.748	718
148	296.0	-31.253	42.937	705
149	298.0	-31.325	43.027	708
150	300.0	-31.469	43.748	720
151	302.0	-31.613	43.928	721
152	304.0	-31.829	44.289	728

TRIAXIAL TEST/TRIAX6.DAT

=====				
Number of Readings 331 No. Columns 4				
Date Task Started 01/01/83 Time Task Started 00-46-44				
=====				
Time	displ7	LOAD18	LOAD	
Secs	mm	KN	NON	
1	2	3	4	

153	306.0	-32.045	44.469	731
154	308.0	-32.261	44.694	735
155	310.0	-32.405	43.613	716
156	312.0	-32.477	43.343	714
157	314.0	-32.621	44.649	734
158	316.0	-32.837	44.649	734
159	318.0	-32.981	44.875	738
160	320.0	-33.197	45.280	745
161	322.0	-33.413	45.776	753
162	324.0	-33.629	44.694	734
163	326.0	-33.701	44.109	726
164	328.0	-33.701	43.793	721
165	330.0	-33.773	45.190	744
166	332.0	-33.989	45.956	755
167	334.0	-34.205	46.226	760
168	336.0	-34.421	45.911	755
169	338.0	-34.637	46.091	758
170	340.0	-34.709	45.100	743
171	342.0	-34.853	46.001	756
172	344.0	-34.997	46.001	756
173	346.0	-35.141	46.181	760
174	348.0	-35.357	46.722	769
175	350.0	-35.501	46.767	770
176	352.0	-35.789	46.947	771
177	354.0	-35.933	46.136	757
178	356.0	-36.005	45.641	749
179	358.0	-36.077	46.542	766
180	360.0	-36.221	46.812	770
181	362.0	-36.365	47.082	774
182	364.0	-36.581	47.353	779
183	366.0	-36.797	47.443	780
184	368.0	-36.941	47.533	781
185	370.0	-37.157	46.992	772
186	372.0	-37.157	46.361	762
187	374.0	-37.229	46.947	772
188	376.0	-37.374	47.443	780
189	378.0	-37.518	47.803	786
190	380.0	-37.734	48.073	791
191	382.0	-37.950	48.254	794
192	384.0	-38.238	48.073	790
193	386.0	-38.382	47.127	773
194	388.0	-38.454	46.587	767
195	390.0	-38.454	46.271	761
196	392.0	-38.526	46.091	758
197	394.0	-38.526	45.911	755
198	396.0	-38.526	46.091	759
199	398.0	-38.598	47.172	776
200	400.0	-38.670	47.037	774
201	402.0	-38.670	46.722	769
202	404.0	-38.742	47.037	774
203	406.0	-38.814	48.479	797

10% strain

Number of Readings 331 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-46-44

Time Secs	displ mm	LOAD KN	LOAD NON
204	408.0	-38.956	48.479
205	410.0	-39.174	48.344
206	412.0	-39.318	48.930
207	414.0	-39.534	48.524
208	416.0	-39.606	47.668
209	418.0	-39.678	47.578
210	420.0	-39.894	49.605
211	422.0	-40.182	49.425
212	424.0	-40.398	49.650
213	426.0	-40.614	49.695
214	428.0	-40.758	48.569
215	430.0	-40.830	48.028
216	432.0	-40.830	47.668
217	434.0	-40.902	47.443
218	436.0	-40.902	47.262
219	438.0	-40.902	47.082
220	440.0	-40.974	46.992
221	442.0	-40.974	46.857
222	444.0	-40.974	46.767
223	446.0	-40.974	46.677
224	448.0	-40.974	46.587
225	450.0	-40.974	46.497
226	452.0	-41.046	46.452
227	454.0	-41.046	46.361
228	456.0	-41.046	46.316
229	458.0	-41.046	46.271
230	460.0	-41.046	46.226
231	462.0	-41.046	46.181
232	464.0	-41.046	46.136
233	466.0	-41.046	46.091
234	468.0	-41.046	46.046
235	470.0	-41.046	46.046
236	472.0	-41.046	46.001
237	474.0	-41.046	45.956
238	476.0	-41.046	45.911
239	478.0	-41.046	45.911
240	480.0	-41.046	45.866
241	482.0	-41.046	45.821
242	484.0	-41.046	45.821
243	486.0	-41.046	45.776
244	488.0	-41.118	45.776
245	490.0	-41.046	45.731
246	492.0	-41.118	45.686
247	494.0	-41.118	45.686
248	496.0	-41.118	45.686
249	498.0	-41.118	45.641
250	500.0	-41.118	45.641
251	502.0	-41.046	45.595
252	504.0	-41.118	45.595
253	506.0	-41.118	45.550
254	508.0	-41.118	45.550

peak

TRIAXIAL TEST/TRIAX6.DAT
Number of Readings 331 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-46-44

Time Secs	displ mm	LOAD KN	LOAD NON
255	510.0	-41.118	45.505
256	512.0	-41.118	45.505
257	514.0	-41.118	45.460
258	516.0	-41.118	45.460
259	518.0	-41.118	45.415
260	520.0	-41.118	45.460
261	522.0	-41.118	45.415
262	524.0	-41.118	45.415
263	526.0	-41.118	45.370
264	528.0	-41.118	45.370
265	530.0	-41.118	45.370
266	532.0	-41.118	45.325
267	534.0	-41.118	45.325
268	536.0	-41.118	45.325
269	538.0	-41.118	45.280
270	540.0	-41.118	45.280
271	542.0	-41.118	45.280
272	544.0	-41.118	45.280
273	546.0	-41.118	45.235
274	548.0	-41.118	45.235
275	550.0	-41.118	45.190
276	552.0	-41.118	45.190
277	554.0	-41.118	45.190
278	556.0	-41.118	45.190
279	558.0	-41.118	45.145
280	560.0	-41.118	45.145
281	562.0	-41.118	45.145
282	564.0	-41.118	45.145
283	566.0	-41.118	45.100
284	568.0	-41.118	45.100
285	570.0	-41.118	45.100
286	572.0	-41.118	45.100
287	574.0	-41.190	45.100
288	576.0	-41.118	45.055
289	578.0	-41.118	45.055
290	580.0	-41.118	45.055
291	582.0	-41.118	45.010
292	584.0	-41.118	45.010
293	586.0	-41.190	45.010
294	588.0	-41.190	45.010
295	590.0	-41.190	45.010
296	592.0	-41.190	44.965
297	594.0	-41.190	44.965
298	596.0	-41.190	44.965
299	598.0	-41.190	44.920
300	600.0	-41.190	44.920
301	602.0	-41.190	44.920
302	604.0	-41.190	44.920
303	606.0	-41.190	44.920
304	608.0	-41.190	44.920
305	610.0	-41.190	44.875

Number of Readings 331 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-46-44

	Time		disp17	LOAD18	LOAD
	Secs	mm			
	1	2	3	4	
306	612.0	-41.190	44.875	738	
307	614.0	-41.190	44.875	739	
308	616.0	-41.190	44.830	738	
309	618.0	-41.190	44.830	738	
310	620.0	-41.190	44.830	738	
311	622.0	-41.190	44.830	738	
312	624.0	-41.190	44.830	738	
313	626.0	-41.190	44.830	738	
314	628.0	-41.190	44.784	737	
315	630.0	-41.190	44.784	738	
316	632.0	-41.190	44.784	736	
317	634.0	-41.190	44.784	736	
318	636.0	-41.190	44.784	737	
319	638.0	-41.190	44.739	737	
320	640.0	-41.190	44.739	735	
321	642.0	-41.190	44.739	736	
322	644.0	-41.190	44.739	736	
323	646.0	-41.190	44.739	736	
324	648.0	-41.190	44.739	736	
325	650.0	-41.190	44.739	736	
326	652.0	-41.190	44.694	735	
327	654.0	-41.190	44.694	734	
328	656.0	-41.190	44.694	735	
329	658.0	-41.190	44.694	735	
330	660.0	-41.190	44.649	734	
331	662.0	-41.190	44.649	735	

Peak axial load difference [kN] = 69.83 } combination
Axial compression [mm] = 58.0 } giving
Cross - sectional area [m^2] = 0.0614 } peak stress
Stress difference, $\sigma_1 - \sigma_3$ [kN/m^2] = 1138
Cell pressure, σ_3 [kN/m^2] = 300
Angle of friction, $\phi = 40.89^\circ$

Axial load difference at 10% axial strain [kN] = 54.98

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 976

Cell pressure, σ_3 [kN/m²] = 300

Angle of friction, $\phi = 38.28^\circ$

$$\text{Cross - sectional area} = \frac{\text{Sample volume}}{\text{Orig. height - axial comp.}}$$

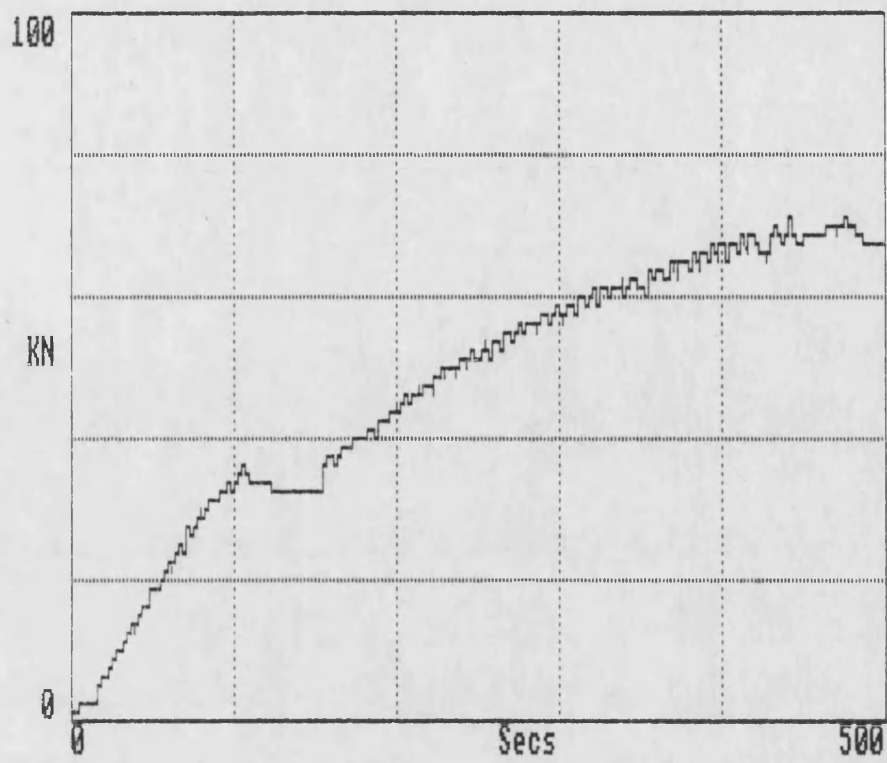
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m^3 ,

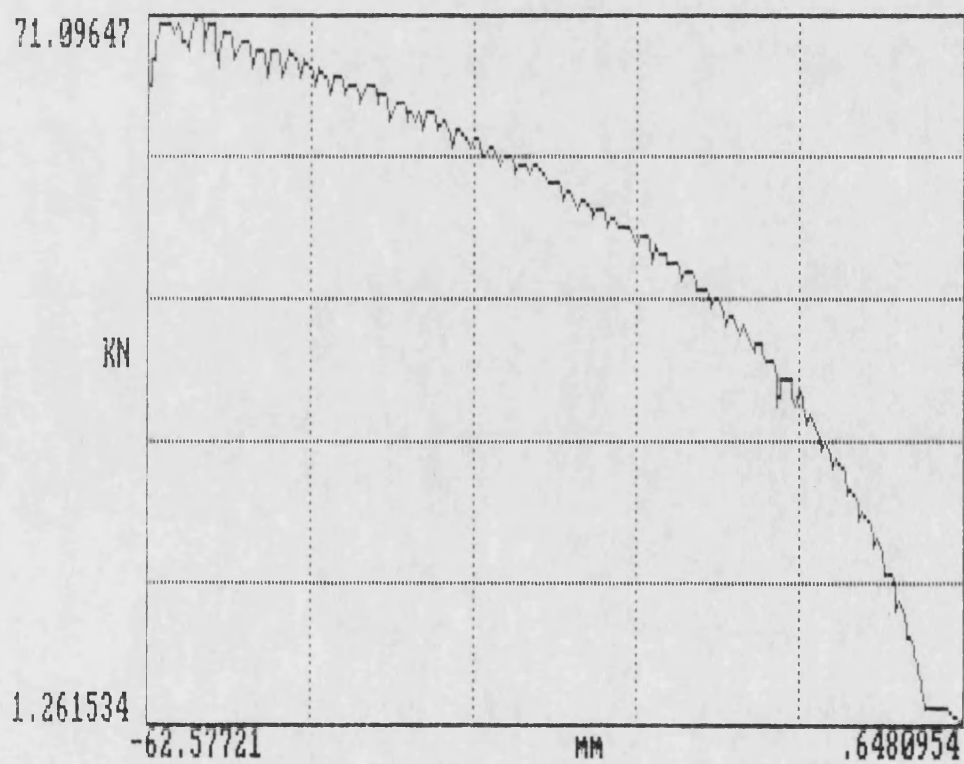
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m^2

LOAD18 v Time



LOAD18 v disp17



Number of Readings 400		No. Columns 4		
Date Task Started 01/01/83	Time Task Started 00-47-59			
=====				
Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4	

0	0.0	0.648	1.262	23
1	2.0	0.648	1.307	23
2	4.0	0.648	1.307	22
3	6.0	0.648	1.307	22
4	8.0	0.648	1.307	22
5	10.0	0.648	1.307	22
6	12.0	0.648	1.307	21
7	14.0	0.648	1.577	27
8	16.0	-0.576	2.613	45
9	18.0	-1.224	2.658	43
10	20.0	-1.440	2.703	45
11	22.0	-1.872	2.793	46
12	24.0	-2.016	2.703	43
13	26.0	-2.160	2.974	49
14	28.0	-2.376	4.505	76
15	30.0	-2.520	5.902	96
16	32.0	-2.664	6.353	102
17	34.0	-2.736	6.983	113
18	36.0	-2.952	8.065	130
19	38.0	-3.168	9.011	146
20	40.0	-3.457	10.002	162
21	42.0	-3.529	9.642	156
22	44.0	-3.673	10.948	179
23	46.0	-3.961	12.300	200
24	48.0	-4.177	13.336	215
25	50.0	-4.393	12.886	207
26	52.0	-4.537	14.237	231
27	54.0	-4.825	15.679	253
28	56.0	-5.185	16.445	264
29	58.0	-5.329	16.130	260
30	60.0	-5.617	18.337	296
31	62.0	-6.049	19.374	310
32	64.0	-6.265	18.788	301
33	66.0	-6.337	19.824	319
34	68.0	-6.553	21.086	337
35	70.0	-6.985	22.167	354
36	72.0	-7.273	21.401	341
37	74.0	-7.417	22.708	364
38	76.0	-7.705	24.059	387
39	78.0	-8.137	25.005	400
40	80.0	-8.209	24.239	387
41	82.0	-8.641	26.718	427
42	84.0	-9.145	27.258	436
43	86.0	-9.289	26.492	424
44	88.0	-9.577	28.520	457
45	90.0	-10.009	29.376	469
46	92.0	-10.298	28.745	459
47	94.0	-10.442	29.916	479
48	96.0	-10.730	31.088	497
49	98.0	-11.090	31.809	508
50	100.0	-11.306	31.043	495

=====				
=====				
Number of Readings 400		No. Columns 4		
Date Task Started 01/01/83		Time Task Started 00-47-59		
=====				
Time	disp17	LOAD18	LOAD	
Secs	mm	KN	NON	
1	2	3	4	

51	102.0	-11.378	31.358	502
52	104.0	-11.594	32.530	520
53	106.0	-11.810	33.296	534
54	108.0	-12.098	34.242	548
55	110.0	-12.386	32.935	526
56	112.0	-12.458	33.926	545
57	114.0	-12.674	34.963	562
58	116.0	-13.106	35.638	571
59	118.0	-13.466	35.008	559
60	120.0	-13.538	34.332	549
61	122.0	-13.538	33.971	544
62	124.0	-13.538	33.746	541
63	126.0	-13.610	33.521	537
64	128.0	-13.610	33.386	535
65	130.0	-13.610	33.296	533
66	132.0	-13.610	33.205	532
67	134.0	-13.610	33.115	531
68	136.0	-13.610	33.025	528
69	138.0	-13.610	32.980	528
70	140.0	-13.682	32.890	528
71	142.0	-13.682	32.845	527
72	144.0	-13.682	32.800	525
73	146.0	-13.682	32.710	525
74	148.0	-13.682	32.665	524
75	150.0	-13.682	32.620	524
76	152.0	-13.682	32.620	522
77	154.0	-13.682	32.530	522
78	156.0	-13.682	32.530	522
79	158.0	-13.682	32.485	521
80	160.0	-13.682	32.439	520
81	162.0	-13.682	32.394	520
82	164.0	-13.682	33.070	532
83	166.0	-13.754	35.323	567
84	168.0	-13.898	36.900	592
85	170.0	-14.114	37.350	598
86	172.0	-14.402	37.305	597
87	174.0	-14.546	37.125	595
88	176.0	-14.762	37.891	608
89	178.0	-14.978	38.477	617
90	180.0	-15.338	38.792	623
91	182.0	-15.554	38.161	612
92	184.0	-15.698	39.153	628
93	186.0	-15.986	39.783	638
94	188.0	-16.274	40.279	647
95	190.0	-16.634	39.738	637
96	192.0	-16.776	40.504	650
97	194.0	-17.067	41.225	663
98	196.0	-17.571	41.811	671
99	198.0	-17.787	40.594	651
100	200.0	-18.003	41.901	673
101	202.0	-18.363	42.982	691

Number of Readings 400 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-47-59

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
102	204.0	-18.723	43.027
103	206.0	-18.939	42.306
104	208.0	-19.227	43.793
105	210.0	-19.515	44.199
106	212.0	-19.947	44.199
107	214.0	-20.163	44.784
108	216.0	-20.451	45.460
109	218.0	-20.883	45.776
110	220.0	-21.171	44.694
111	222.0	-21.387	46.497
112	224.0	-21.891	46.812
113	226.0	-22.251	46.406
114	228.0	-22.467	47.217
115	230.0	-22.899	47.893
116	232.0	-23.331	48.073
117	234.0	-23.547	46.767
118	236.0	-23.836	49.020
119	238.0	-24.340	49.290
120	240.0	-24.628	48.344
121	242.0	-24.844	49.425
122	244.0	-25.276	50.416
123	246.0	-25.780	50.011
124	248.0	-25.996	50.461
125	250.0	-26.428	51.182
126	252.0	-26.788	51.092
127	254.0	-27.004	50.011
128	256.0	-27.364	51.993
129	258.0	-27.796	51.858
130	260.0	-28.084	51.362
131	262.0	-28.228	51.723
132	264.0	-28.588	52.939
133	266.0	-29.020	53.120
134	268.0	-29.380	51.993
135	270.0	-29.524	53.029
136	272.0	-29.884	54.021
137	274.0	-30.244	54.021
138	276.0	-30.532	52.804
139	278.0	-30.821	54.651
140	280.0	-31.253	54.922
141	282.0	-31.613	54.291
142	284.0	-31.829	55.057
143	286.0	-32.261	55.913
144	288.0	-32.693	56.003
145	290.0	-32.909	55.012
146	292.0	-33.269	56.319
147	294.0	-33.701	56.228
148	296.0	-34.061	55.643
149	298.0	-34.205	56.048
150	300.0	-34.493	56.949
151	302.0	-34.853	57.175
152	304.0	-35.213	56.679

10% strain

TRIAXIAL TEST/ TRIAX7.DAT

Number of Readings 400 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-47-59

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
153	306.0	-35.429	57.355
154	308.0	-35.789	57.670
155	310.0	-36.077	58.166
156	312.0	-36.437	57.084
157	314.0	-36.653	57.535
158	316.0	-36.941	58.526
159	318.0	-37.374	59.022
160	320.0	-37.662	57.760
161	322.0	-37.806	59.247
162	324.0	-38.310	59.878
163	326.0	-38.742	59.562
164	328.0	-38.958	58.256
165	330.0	-39.174	60.058
166	332.0	-39.606	60.824
167	334.0	-40.038	59.472
168	336.0	-40.182	60.148
169	338.0	-40.542	61.455
170	340.0	-41.046	61.320
171	342.0	-41.262	59.607
172	344.0	-41.550	61.545
173	346.0	-41.838	61.004
174	348.0	-42.054	61.815
175	350.0	-42.414	61.320
176	352.0	-42.558	60.689
177	354.0	-42.846	62.266
178	356.0	-43.206	62.401
179	358.0	-43.566	61.770
180	360.0	-43.710	61.365
181	362.0	-43.854	60.734
182	364.0	-43.926	60.193
183	366.0	-44.215	63.617
184	368.0	-44.647	63.257
185	370.0	-44.863	62.040
186	372.0	-45.151	63.753
187	374.0	-45.727	64.203
188	376.0	-46.087	62.491
189	378.0	-46.231	62.987
190	380.0	-46.591	64.383
191	382.0	-47.023	64.473
192	384.0	-47.239	63.077
193	386.0	-47.671	65.194
194	388.0	-48.175	64.699
195	390.0	-48.463	63.572
196	392.0	-48.679	65.329
197	394.0	-49.255	65.780
198	396.0	-49.615	64.473
199	398.0	-49.903	66.050
200	400.0	-50.479	66.681
201	402.0	-50.911	64.879
202	404.0	-51.128	66.726
203	406.0	-51.776	67.357

=====

Number of Readings	400	No. Columns	4
Date Task Started	01/01/83	Time Task Started	00-47-59

=====

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
204	408.0	-52.136	65.149
205	410.0	-52.496	67.447
206	412.0	-53.144	67.357
207	414.0	-53.432	65.284
208	416.0	-53.792	67.672
209	418.0	-54.296	67.717
210	420.0	-54.656	65.960
211	422.0	-54.872	68.078
212	424.0	-55.520	68.799
213	426.0	-55.880	66.546
214	428.0	-56.240	68.934
215	430.0	-56.744	69.114
216	432.0	-57.032	67.402
217	434.0	-57.176	66.501
218	436.0	-57.248	65.960
219	438.0	-57.248	65.600
220	440.0	-57.320	67.853
221	442.0	-57.536	70.015
222	444.0	-57.969	69.970
223	446.0	-58.185	68.168
224	448.0	-58.257	67.402
225	450.0	-58.401	69.249
226	452.0	-58.689	71.096
227	454.0	-59.193	70.421
228	456.0	-59.409	68.528
229	458.0	-59.481	67.717
230	460.0	-59.553	67.312
231	462.0	-59.625	68.258
232	464.0	-59.697	68.664
233	466.0	-59.769	68.754
234	468.0	-59.841	68.438
235	470.0	-59.913	68.664
236	472.0	-59.985	68.889
237	474.0	-60.129	69.610
238	476.0	-60.201	69.925
239	478.0	-60.417	70.105
240	480.0	-60.561	69.520
241	482.0	-60.633	69.384
242	484.0	-60.777	70.105
243	486.0	-60.993	70.466
244	488.0	-61.209	70.556
245	490.0	-61.497	70.331
246	492.0	-61.713	70.150
247	494.0	-61.857	69.024
248	496.0	-61.929	68.393
249	498.0	-61.929	67.943
250	500.0	-62.001	67.627
251	502.0	-62.001	67.402
252	504.0	-62.073	67.402
253	506.0	-62.073	67.537
254	508.0	-62.145	67.357

peak

TRIAXIAL TEST/IRIAX/.DAI

=====

Number of Readings	400	No. Columns	4
Date Task Started	01/01/83	Time Task Started	00-47-59

=====

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
255	510.0	-62.145	67.132
256	512.0	-62.145	66.996
257	514.0	-62.145	66.906
258	516.0	-62.145	66.771
259	518.0	-62.145	66.681
260	520.0	-62.145	66.591
261	522.0	-62.217	66.501
262	524.0	-62.217	66.411
263	526.0	-62.217	66.366
264	528.0	-62.217	66.276
265	530.0	-62.217	66.231
266	532.0	-62.217	66.140
267	534.0	-62.217	66.095
268	536.0	-62.217	66.005
269	538.0	-62.217	65.960
270	540.0	-62.289	65.915
271	542.0	-62.289	65.870
272	544.0	-62.289	65.825
273	546.0	-62.289	65.780
274	548.0	-62.289	65.690
275	550.0	-62.289	65.645
276	552.0	-62.289	65.600
277	554.0	-62.289	65.555
278	556.0	-62.289	65.510
279	558.0	-62.289	65.465
280	560.0	-62.289	65.420
281	562.0	-62.289	65.375
282	564.0	-62.289	65.329
283	566.0	-62.289	65.284
284	568.0	-62.289	65.239
285	570.0	-62.361	65.239
286	572.0	-62.361	65.194
287	574.0	-62.361	65.149
288	576.0	-62.361	65.104
289	578.0	-62.361	65.059
290	580.0	-62.361	65.059
291	582.0	-62.361	65.014
292	584.0	-62.361	64.969
293	586.0	-62.361	64.924
294	588.0	-62.361	64.924
295	590.0	-62.361	64.879
296	592.0	-62.361	64.879
297	594.0	-62.361	64.834
298	596.0	-62.361	64.789
299	598.0	-62.361	64.744
300	600.0	-62.361	64.744
301	602.0	-62.361	64.699
302	604.0	-62.361	64.699
303	606.0	-62.361	64.654
304	608.0	-62.361	64.654
305	610.0	-62.361	64.609

Number of Readings 400 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-47-59

	Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
306	612.0	-62.361	64.564	1049
307	614.0	-62.433	64.564	1047
308	616.0	-62.433	64.518	1047
309	618.0	-62.433	64.518	1048
310	620.0	-62.433	64.473	1047
311	622.0	-62.433	64.473	1046
312	624.0	-62.433	64.428	1046
313	626.0	-62.433	64.428	1045
314	628.0	-62.433	64.383	1045
315	630.0	-62.433	64.383	1045
316	632.0	-62.433	64.338	1043
317	634.0	-62.433	64.338	1044
318	636.0	-62.433	64.293	1044
319	638.0	-62.433	64.293	1043
320	640.0	-62.433	64.248	1043
321	642.0	-62.433	64.248	1043
322	644.0	-62.433	64.248	1042
323	646.0	-62.433	64.203	1041
324	648.0	-62.433	64.158	1042
325	650.0	-62.433	64.158	1042
326	652.0	-62.433	64.158	1042
327	654.0	-62.433	64.113	1040
328	656.0	-62.433	64.068	1040
329	658.0	-62.433	64.068	1040
330	660.0	-62.433	64.068	1039
331	662.0	-62.433	64.068	1039
332	664.0	-62.433	64.023	1039
333	666.0	-62.433	63.978	1038
334	668.0	-62.433	63.978	1039
335	670.0	-62.433	63.978	1037
336	672.0	-62.433	63.933	1039
337	674.0	-62.433	63.933	1038
338	676.0	-62.433	63.888	1037
339	678.0	-62.433	63.888	1037
340	680.0	-62.433	63.888	1037
341	682.0	-62.433	63.843	1037
342	684.0	-62.433	63.843	1036
343	686.0	-62.505	63.843	1036
344	688.0	-62.505	63.798	1036
345	690.0	-62.505	63.798	1036
346	692.0	-62.505	63.798	1035
347	694.0	-62.433	63.753	1035
348	696.0	-62.433	63.753	1036
349	698.0	-62.505	63.753	1034
350	700.0	-62.505	63.707	1035
351	702.0	-62.505	63.707	1035
352	704.0	-62.505	63.707	1035
353	706.0	-62.505	63.662	1034
354	708.0	-62.505	63.662	1033
355	710.0	-62.505	63.662	1032
356	712.0	-62.505	63.617	1033

Number of Readings 400 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-47-59

	Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
357	714.0	-62.505	63.617	1034
358	716.0	-62.505	63.617	1032
359	718.0	-62.505	63.572	1032
360	720.0	-62.505	63.572	1032
361	722.0	-62.505	63.572	1032
362	724.0	-62.505	63.527	1032
363	726.0	-62.505	63.527	1032
364	728.0	-62.505	63.527	1031
365	730.0	-62.505	63.482	1031
366	732.0	-62.505	63.482	1031
367	734.0	-62.505	63.482	1030
368	736.0	-62.505	63.482	1030
369	738.0	-62.505	63.437	1029
370	740.0	-62.505	63.437	1029
371	742.0	-62.505	63.437	1029
372	744.0	-62.505	63.437	1029
373	746.0	-62.505	63.392	1029
374	748.0	-62.505	63.392	1029
375	750.0	-62.505	63.392	1028
376	752.0	-62.505	63.347	1029
377	754.0	-62.505	63.347	1029
378	756.0	-62.505	63.347	1029
379	758.0	-62.505	63.302	1028
380	760.0	-62.505	63.302	1028
381	762.0	-62.505	63.302	1028
382	764.0	-62.577	63.302	1027
383	766.0	-62.505	63.257	1027
384	768.0	-62.505	63.257	1027
385	770.0	-62.505	63.257	1027
386	772.0	-62.505	63.212	1026
387	774.0	-62.505	63.212	1027
388	776.0	-62.577	63.212	1026
389	778.0	-62.577	63.212	1025
390	780.0	-62.577	63.167	1026
391	782.0	-62.505	63.167	1026
392	784.0	-62.505	63.167	1026
393	786.0	-62.577	63.167	1025
394	788.0	-62.505	63.122	1025
395	790.0	-62.577	63.122	1025
396	792.0	-62.577	63.122	1025
397	794.0	-62.505	63.122	1025
398	796.0	-62.505	63.077	1025
399	798.0	-62.577	63.077	1024
400	800.0	-62.505	63.077	1025

CALCULATION SHEET FOR TEST Triax 8

Peak axial load difference [kN] = 82.68
Axial compression [mm] = 34.5
Cross - sectional area [m²] = 0.566
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 1461
Cell pressure, σ_3 [kN/m²] = 400
Angle of friction, $\phi = 40.27^\circ$

} combination
giving
peak stress

Axial load difference at 10% axial strain [kN] = 76.73
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 1363
Cell pressure, σ_3 [kN/m²] = 400
Angle of friction, $\phi = 39.0^\circ$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

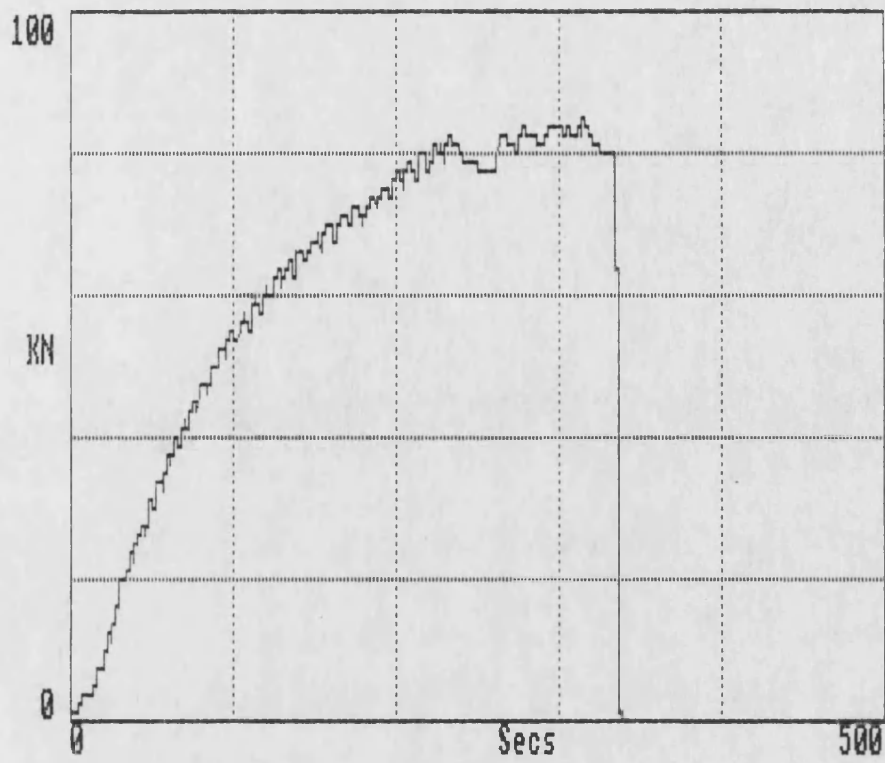
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

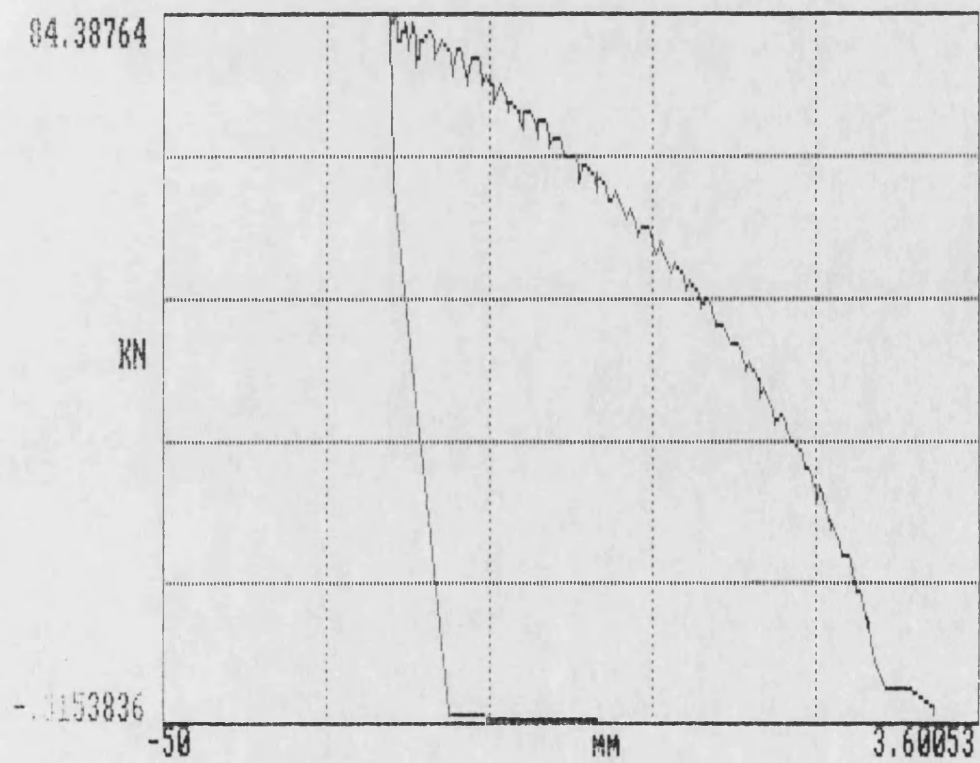
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v displ7



Number of Readings 267 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 01-03-29

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
0	0.0	0.720	1.712 28
1	2.0	0.720	1.712 28
2	4.0	0.720	1.712 28
3	6.0	0.720	1.757 28
4	8.0	0.720	1.712 27
5	10.0	0.720	1.712 28
6	12.0	0.720	1.712 27
7	14.0	0.792	1.712 27
8	16.0	0.792	1.712 27
9	18.0	-0.792	3.694 62
10	20.0	-1.152	3.649 60
11	22.0	-1.728	4.235 71
12	24.0	-2.376	4.100 67
13	26.0	-2.808	4.956 84
14	28.0	-3.168	7.479 126
15	30.0	-3.168	7.119 118
16	32.0	-3.384	9.191 156
17	34.0	-3.673	12.030 203
18	36.0	-3.817	11.759 195
19	38.0	-4.249	15.949 268
20	40.0	-4.537	15.679 260
21	42.0	-5.041	19.779 329
22	44.0	-5.329	19.644 324
23	46.0	-5.617	21.807 362
24	48.0	-6.049	24.059 398
25	50.0	-6.265	23.023 378
26	52.0	-6.409	25.276 418
27	54.0	-6.913	27.754 459
28	56.0	-7.057	26.627 438
29	58.0	-7.129	27.393 455
30	60.0	-7.633	30.682 509
31	62.0	-7.921	30.052 496
32	64.0	-8.137	32.485 539
33	66.0	-8.641	34.016 563
34	68.0	-8.857	33.070 546
35	70.0	-9.217	36.359 604
36	72.0	-9.793	36.134 597
37	74.0	-9.937	37.711 627
38	76.0	-10.514	40.369 670
39	78.0	-10.802	38.657 639
40	80.0	-10.874	40.820 680
41	82.0	-11.522	42.892 710
42	84.0	-11.738	42.487 706
43	86.0	-12.170	45.145 750
44	88.0	-12.602	45.010 745
45	90.0	-12.746	45.686 760
46	92.0	-13.178	47.578 789
47	94.0	-13.610	47.533 786
48	96.0	-13.754	47.443 787
49	98.0	-14.258	50.416 835
50	100.0	-14.690	49.425 817

IRIMABE TEST/IRIMAB.DAT

Number of Readings 267 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 01-03-29

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
51	102.0	-14.906	51.903 862
52	104.0	-15.410	52.714 873
53	106.0	-15.698	51.633 853
54	108.0	-15.842	53.795 894
55	110.0	-16.274	54.606 904
56	112.0	-16.634	54.201 896
57	114.0	-16.706	54.201 898
58	116.0	-16.994	56.183 931
59	118.0	-17.427	56.994 943
60	120.0	-17.643	55.913 924
61	122.0	-17.787	57.084 948
62	124.0	-18.147	59.022 978
63	126.0	-18.579	58.751 970
64	128.0	-18.723	57.940 959
65	130.0	-18.939	60.509 1003
66	132.0	-19.371	61.365 1017
67	134.0	-19.659	59.833 990
68	136.0	-19.803	60.644 1006
69	138.0	-20.163	63.257 1048
70	140.0	-20.595	62.311 1029
71	142.0	-20.667	62.086 1029
72	144.0	-20.955	64.113 1063
73	146.0	-21.387	65.014 1076
74	148.0	-21.603	63.392 1049
75	150.0	-21.747	65.104 1082
76	152.0	-22.179	66.366 1098
77	154.0	-22.539	65.870 1089
78	156.0	-22.683	64.564 1067
79	158.0	-22.755	66.186 1097
80	160.0	-22.971	67.132 1112
81	162.0	-23.259	67.807 1121
82	164.0	-23.475	67.267 1111
83	166.0	-23.619	67.672 1120
84	168.0	-23.836	69.204 1144
85	170.0	-24.196	69.520 1148
86	172.0	-24.412	69.114 1140
87	174.0	-24.556	67.988 1122
88	176.0	-24.628	69.429 1148
89	178.0	-24.916	71.322 1179
90	180.0	-25.276	71.322 1175
91	182.0	-25.492	69.745 1149
92	184.0	-25.636	71.367 1179
93	186.0	-25.924	72.763 1200
94	188.0	-26.284	72.403 1192
95	190.0	-26.428	70.916 1167
96	192.0	-26.500	71.592 1181
97	194.0	-26.716	72.944 1202
98	196.0	-27.004	73.845 1215
99	198.0	-27.220	73.665 1210
100	200.0	-27.364	72.403 1190
101	202.0	-27.580	75.061 1235

Number of Readings 267 No. Columns 4
Date Task Started 01/01/83 Time Task Started 01-03-29

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
102	204.0	-27.868	75.422 1238
103	206.0	-28.084	74.791 1226
104	208.0	-28.228	73.710 1206
105	210.0	-28.372	76.143 1249
106	212.0	-28.732	77.314 1265
107	214.0	-29.020	76.188 1242
108	216.0	-29.092	76.323 1248
109	218.0	-29.308	78.576 1286
110	220.0	-29.740	78.936 1290
111	222.0	-29.956	77.179 1260
112	224.0	-30.100	76.368 1247
113	226.0	-30.388	80.333 1317
114	228.0	-30.821	79.792 1305
115	230.0	-31.037	78.035 1275
116	232.0	-31.109	77.179 1263
117	234.0	-31.253	80.107 1313
118	236.0	-31.613	80.828 1324
119	238.0	-31.973	80.648 1320
120	240.0	-32.117	79.161 1297
121	242.0	-32.261	81.099 1331
122	244.0	-32.549	82.315 1350
123	246.0	-32.837	81.639 1339
124	248.0	-33.125	80.873 1325
125	250.0	-33.197	79.927 1309
126	252.0	-33.269	79.387 1300
127	254.0	-33.269	78.981 1294
128	256.0	-33.341	78.666 1289
129	258.0	-33.341	78.440 1284
130	260.0	-33.341	78.215 1281
131	262.0	-33.341	78.080 1277
132	264.0	-33.413	77.900 1275
133	266.0	-33.413	77.810 1272
134	268.0	-33.413	77.674 1271
135	270.0	-33.413	77.539 1269
136	272.0	-33.413	78.621 1288
137	274.0	-33.485	80.783 1323
138	276.0	-33.557	82.811 1358
139	278.0	-33.701	82.540 1352
140	280.0	-33.845	81.549 1335
141	282.0	-33.917	80.963 1325
142	284.0	-33.917	80.558 1317
143	286.0	-33.989	80.288 1312
144	288.0	-34.061	82.901 1358
145	290.0	-34.205	83.261 1364
146	292.0	-34.277	82.631 1352
147	294.0	-34.421	82.585 1350
148	296.0	-34.421	81.910 1338
149	298.0	-34.493	81.459 1331
150	300.0	-34.493	81.099 1324
151	302.0	-34.565	81.594 1333
152	304.0	-34.565	82.901 1355

IRIRAHIL TEST/IRIRAHIL.VMT

Number of Readings 267 No. Columns 4
Date Task Started 01/01/83 Time Task Started 01-03-29

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
153	306.0	-34.637	83.577 1367
154	308.0	-34.709	83.667 1370
155	310.0	-34.853	84.117 1379
156	312.0	-34.853	83.351 1366
157	314.0	-34.925	82.901 1359
158	316.0	-34.997	83.712 1373
159	318.0	-35.069	82.901 1359
160	320.0	-35.069	82.405 1350
161	322.0	-35.141	82.045 1345
162	324.0	-35.141	83.261 1366
163	326.0	-35.213	84.388 1389
164	328.0	-35.213	84.027 1385
165	330.0	-35.213	82.856 1366
166	332.0	-35.141	82.000 1351
167	334.0	-35.141	81.189 1341
168	336.0	-35.141	80.738 1335
169	338.0	-35.069	80.423 1331
170	340.0	-35.141	80.198 1328
171	342.0	-35.069	80.017 1326
172	344.0	-35.141	79.837 1324
173	346.0	-34.853	63.347 979
174	348.0	-31.325	0.856 10
175	350.0	-11.810	0.180 6
176	352.0	3.529	-0.315 -1
177	354.0	3.529	-0.270 0
178	356.0	3.529	-0.270 -1
179	358.0	3.529	-0.270 -1
180	360.0	3.529	-0.270 -1
181	362.0	3.529	-0.225 1
182	364.0	3.529	-0.225 -1
183	366.0	3.529	-0.225 1
184	368.0	3.529	-0.225 1
185	370.0	3.529	-0.225 0
186	372.0	3.601	-0.225 -0
187	374.0	3.529	-0.225 -0
188	376.0	3.529	-0.180 0
189	378.0	3.529	-0.180 0
190	380.0	3.529	-0.180 1
191	382.0	3.529	-0.180 -1
192	384.0	3.529	-0.180 -0
193	386.0	3.529	-0.180 0
194	388.0	3.529	-0.180 0
195	390.0	3.529	-0.180 -0
196	392.0	3.529	-0.180 0
197	394.0	3.529	-0.135 -1
198	396.0	3.529	-0.180 -2
199	398.0	3.529	-0.180 -3
200	400.0	3.601	-0.135 -5
201	402.0	3.529	-0.180 -6
202	404.0	3.529	-0.180 -5
203	406.0	3.529	-0.135 -4

=====

Number of Readings 267 No. Columns 4

Date Task Started 01/01/83 Time Task Started 01-03-29

=====

Time	disp17	LOAD18	LOAD	
Secs	mm	KN	NON	
1	2	3	4	
204	408.0	3.529	-0.135	-5
205	410.0	3.529	-0.180	-5
206	412.0	3.529	-0.135	-5
207	414.0	3.529	-0.135	-5
208	416.0	3.529	-0.135	-4
209	418.0	3.529	-0.135	-5
210	420.0	3.529	-0.135	-4
211	422.0	3.529	-0.135	-4
212	424.0	3.529	-0.135	-5
213	426.0	3.529	-0.135	-5
214	428.0	3.529	-0.135	-5
215	430.0	3.529	-0.135	-5
216	432.0	3.601	-0.135	-5
217	434.0	-5.761	-0.135	-5
218	436.0	-13.034	-0.135	-4
219	438.0	-24.700	-0.135	-5
220	440.0	-43.926	-0.135	-5
221	442.0	-589.911	-0.135	-4
222	444.0	-589.911	-0.135	-5
223	446.0	-589.911	-0.090	-5
224	448.0	-589.911	-0.090	-6
225	450.0	-589.911	-0.135	-5
226	452.0	-589.911	-0.135	-5
227	454.0	-589.911	-0.135	-4
228	456.0	-589.911	-0.135	-4
229	458.0	-589.911	-0.135	-4
230	460.0	-589.911	-0.135	-4
231	462.0	-589.911	-0.135	-5
232	464.0	-589.911	-0.135	-5
233	466.0	-589.911	-0.090	-5
234	468.0	-31.325	-0.135	-4
235	470.0	-589.911	-0.135	-5
236	472.0	-589.911	-0.090	-5
237	474.0	-589.911	-0.135	-5
238	476.0	-56.024	-0.090	-4
239	478.0	-37.518	-0.135	-5
240	480.0	-37.590	-0.135	-4
241	482.0	-37.590	-0.090	-5
242	484.0	-48.031	-0.090	-4
243	486.0	-589.911	-0.090	-5
244	488.0	-58.041	-0.090	-5
245	490.0	-63.369	-0.090	-5
246	492.0	-67.258	-0.135	-5
247	494.0	-67.258	-0.090	-4
248	496.0	-57.032	-0.090	-4
249	498.0	-20.523	-0.135	-5
250	500.0	-41.046	-0.090	-4
251	502.0	-37.374	-0.090	-5
252	504.0	-42.486	-0.090	-5
253	506.0	-37.806	-0.090	-5
254	508.0	-13.034	-0.090	-5

=====

Number of Readings 267 No. Columns 4

Date Task Started 01/01/83 Time Task Started 01-03-29

=====

Time	disp17	LOAD18	LOAD	
Secs	mm	KN	NON	
1	2	3	4	
255	510.0	-65.890	-0.090	-4
256	512.0	-31.613	-0.090	-5
257	514.0	-45.943	-0.090	-5
258	516.0	-56.240	-0.090	-6
259	518.0	-56.240	-0.090	-5
260	520.0	-56.096	-0.090	-6
261	522.0	-55.880	-0.090	-5
262	524.0	-55.808	-0.090	-5
263	526.0	-55.808	-0.090	-5
264	528.0	-55.808	-0.090	-5
265	530.0	-55.736	-0.090	-5
266	532.0	-55.664	-0.090	-6
267	534.0	-55.592	-0.090	-6

CALCULATION SHEET FOR TEST Triax 9.

Cell pressure increase during test. Peak void.

Peak axial load difference [kN] =
Axial compression [mm] =

} combination
giving
peak stress

Cross - sectional area [m²] =

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] =

Cell pressure, σ_3 [kN/m²] =

Angle of friction, ϕ =

Axial load difference at 10% axial strain [kN] = 34.69

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 616

Cell pressure, σ_3 [kN/m²] = 100

Angle of friction, ϕ = 49.0°

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height - axial comp.}}$

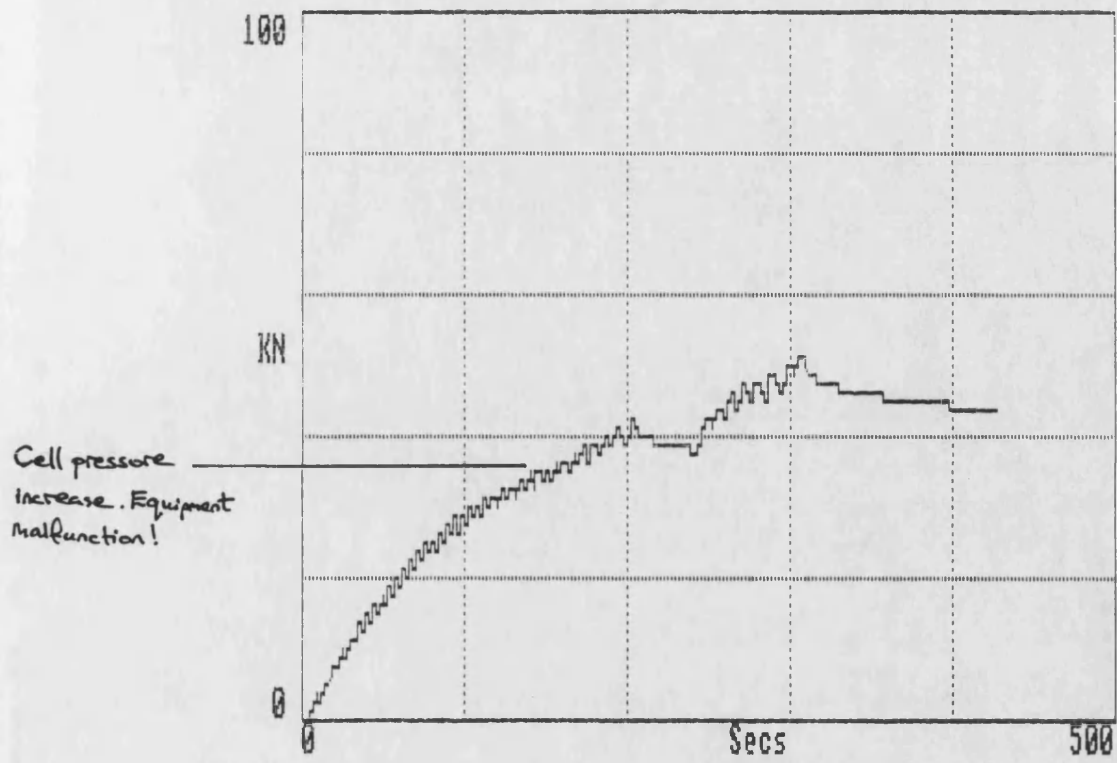
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

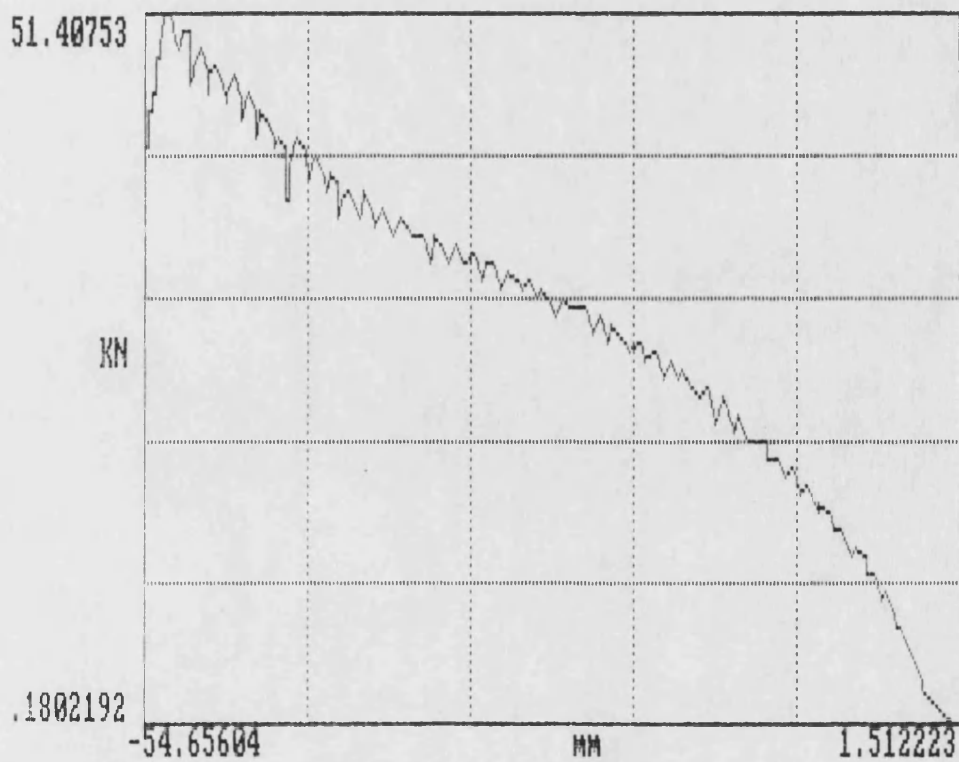
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v disp17



Number of Readings 324 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-13-43

	Time Secs 1	disp17 mm 2	LOAD18 KN 3	LOAD NON 4
0	0.0	1.440	0.180	2
1	2.0	1.368	0.180	3
2	4.0	1.440	0.180	3
3	6.0	1.440	0.225	2
4	8.0	1.440	0.180	3
5	10.0	1.440	0.180	2
6	12.0	1.440	0.180	2
7	14.0	1.440	0.180	2
8	16.0	1.440	0.180	3
9	18.0	1.440	0.225	3
10	20.0	1.440	0.180	3
11	22.0	1.440	0.180	3
12	24.0	1.440	0.180	3
13	26.0	1.440	0.180	4
14	28.0	1.440	0.180	4
15	30.0	1.440	0.225	3
16	32.0	1.440	0.180	2
17	34.0	1.440	0.180	3
18	36.0	1.512	0.180	2
19	38.0	1.440	0.225	3
20	40.0	1.440	0.180	2
21	42.0	1.440	0.180	2
22	44.0	1.440	0.180	3
23	46.0	1.440	0.180	2
24	48.0	1.440	0.225	3
25	50.0	1.440	0.180	3
26	52.0	1.512	0.180	3
27	54.0	1.512	0.180	1
28	56.0	1.512	0.180	3
29	58.0	1.512	0.225	2
30	60.0	1.512	0.180	3
31	62.0	1.512	0.225	4
32	64.0	0.576	0.721	12
33	66.0	-0.936	2.793	46
34	68.0	-1.080	3.154	52
35	70.0	-1.152	3.649	61
36	72.0	-1.440	4.911	81
37	74.0	-1.800	5.452	90
38	76.0	-1.944	6.037	101
39	78.0	-2.520	7.434	124
40	80.0	-2.808	7.389	121
41	82.0	-3.096	8.786	146
42	84.0	-3.673	9.597	159
43	86.0	-3.817	9.146	152
44	88.0	-4.249	10.993	183
45	90.0	-4.825	11.129	183
46	92.0	-4.969	12.075	203
47	94.0	-5.761	13.156	220
48	96.0	-5.977	12.525	208
49	98.0	-6.553	14.508	243
50	100.0	-7.057	14.192	234

Number of Readings 324 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-13-43

	Time Secs 1	disp17 mm 2	LOAD18 KN 3	LOAD NON 4
51	102.0	-7.345	15.724	263
52	104.0	-8.137	16.040	266
53	106.0	-8.209	15.229	253
54	108.0	-8.929	17.616	293
55	110.0	-9.361	16.760	277
56	112.0	-9.865	18.788	313
57	114.0	-10.514	18.112	299
58	116.0	-10.874	19.644	327
59	118.0	-11.666	19.554	322
60	120.0	-11.882	20.635	344
61	122.0	-12.818	20.905	346
62	124.0	-12.962	20.905	347
63	126.0	-13.898	22.753	376
64	128.0	-14.114	21.266	351
65	130.0	-14.834	23.609	391
66	132.0	-15.266	22.257	366
67	134.0	-15.770	24.420	405
68	136.0	-16.418	23.609	388
69	138.0	-16.778	24.645	408
70	140.0	-17.571	25.501	419
71	142.0	-17.787	24.870	411
72	144.0	-18.507	26.357	434
73	146.0	-19.011	25.050	413
74	148.0	-19.515	27.078	445
75	150.0	-20.091	26.267	430
76	152.0	-20.451	27.754	457
77	154.0	-21.315	27.393	448
78	156.0	-21.531	27.889	459
79	158.0	-22.467	29.196	479
80	160.0	-22.683	27.438	451
81	162.0	-23.331	29.691	488
82	164.0	-23.836	28.430	464
83	166.0	-24.196	30.187	496
84	168.0	-24.988	30.007	490
85	170.0	-25.204	30.232	496
86	172.0	-25.924	31.133	510
87	174.0	-26.284	29.691	484
88	176.0	-26.716	31.178	510
89	178.0	-27.436	31.628	515
90	180.0	-27.652	31.088	510
91	182.0	-28.228	32.394	530
92	184.0	-28.732	31.313	510
93	186.0	-28.948	32.079	525
94	188.0	-29.668	33.025	539
95	190.0	-30.028	31.538	513
96	192.0	-30.532	33.431	546
97	194.0	-31.181	33.296	541
98	196.0	-31.325	32.034	522
99	198.0	-31.973	34.242	558
100	200.0	-32.477	33.474	547
101	202.0	-32.693	33.292	547

Number of Readings 324 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-13-43

Time Secs	disp17 mm	LOAD18 KN	LOAD NON	
1	2	3	4	
102	204.0	-33.269	34.872	567
103	206.0	-33.773	33.476	543
104	208.0	-34.133	34.917	569
105	210.0	-34.853	35.503	579
106	212.0	-35.069	33.701	549
107	214.0	-35.573	35.548	579
108	216.0	-36.149	35.458	574
109	218.0	-36.437	35.774	583
110	220.0	-37.085	36.584	594
111	222.0	-37.518	35.413	574
112	224.0	-37.734	35.954	586
113	226.0	-38.310	37.486	609
114	228.0	-38.742	36.044	583
115	230.0	-38.958	37.260	607
116	232.0	-39.606	38.477	625
117	234.0	-39.966	36.449	591
118	236.0	-40.038	37.215	607
119	238.0	-40.614	38.702	628
120	240.0	-41.118	38.206	617
121	242.0	-41.262	36.945	598
122	244.0	-41.478	39.063	633
123	246.0	-41.910	40.099	650
124	248.0	-42.270	38.657	625
125	250.0	-42.414	39.828	646
126	252.0	-42.846	41.135	666
127	254.0	-43.278	40.775	659
128	256.0	-43.422	39.513	637
129	258.0	-43.494	39.108	633
130	260.0	-43.710	41.676	675
131	262.0	-44.143	42.171	681
132	264.0	-44.503	41.495	670
133	266.0	-44.575	40.549	656
134	268.0	-44.647	40.099	649
135	270.0	-44.647	39.783	643
136	272.0	-44.719	39.558	640
137	274.0	-44.719	39.378	637
138	276.0	-44.719	39.198	635
139	278.0	-44.719	39.063	632
140	280.0	-44.791	38.927	630
141	282.0	-44.791	38.792	627
142	284.0	-44.791	38.702	626
143	286.0	-44.791	38.567	625
144	288.0	-44.791	38.477	623
145	290.0	-44.791	38.387	622
146	292.0	-44.791	38.297	621
147	294.0	-44.791	38.252	620
148	296.0	-44.863	38.206	619
149	298.0	-44.791	38.116	617
150	300.0	-44.863	38.026	616
151	302.0	-44.863	37.981	615
152	304.0	-44.935	40.054	650

10%

Number of Readings 324 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-13-43

Time Secs	disp17 mm	LOAD18 KN	LOAD NON	
1	2	3	4	
153	306.0	-45.079	41.991	680
154	308.0	-45.439	42.487	687
155	310.0	-45.727	41.541	670
156	312.0	-45.799	42.216	684
157	314.0	-46.231	43.793	708
158	316.0	-46.663	44.064	711
159	318.0	-46.879	42.622	687
160	320.0	-46.951	44.109	714
161	322.0	-47.455	45.686	738
162	324.0	-47.887	44.920	723
163	326.0	-48.031	43.793	706
164	328.0	-48.103	44.875	727
165	330.0	-48.607	47.037	760
166	332.0	-48.967	45.821	738
167	334.0	-49.111	44.920	725
168	336.0	-49.471	47.217	763
169	338.0	-49.903	47.758	771
170	340.0	-50.191	46.677	752
171	342.0	-50.335	45.821	738
172	344.0	-50.407	46.992	761
173	346.0	-50.839	48.930	789
174	348.0	-51.200	48.524	780
175	350.0	-51.344	47.308	763
176	352.0	-51.416	46.677	753
177	354.0	-51.488	46.316	747
178	356.0	-51.632	49.831	807
179	358.0	-52.136	50.056	807
180	360.0	-52.424	48.659	784
181	362.0	-52.568	49.380	797
182	364.0	-52.928	51.408	830
183	366.0	-53.432	51.092	823
184	368.0	-53.648	49.515	798
185	370.0	-53.720	48.884	787
186	372.0	-53.720	48.434	781
187	374.0	-53.792	48.073	776
188	376.0	-53.792	47.848	772
189	378.0	-53.864	47.623	768
190	380.0	-53.864	47.443	765
191	382.0	-53.864	47.262	762
192	384.0	-53.864	47.172	760
193	386.0	-53.936	47.037	758
194	388.0	-53.936	46.902	756
195	390.0	-53.936	46.767	754
196	392.0	-53.936	46.677	753
197	394.0	-53.936	46.587	751
198	396.0	-53.936	46.452	750
199	398.0	-54.008	46.361	748
200	400.0	-54.008	46.271	746
201	402.0	-54.008	46.181	745
202	404.0	-54.008	46.091	743
203	406.0	-54.008	45.956	741

Number of Readings 324 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-13-43

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
204	408.0	-54.080	45.911
205	410.0	-54.080	45.776
206	412.0	-54.080	45.731
207	414.0	-54.080	45.641
208	416.0	-54.080	45.550
209	418.0	-54.080	45.505
210	420.0	-54.080	45.415
211	422.0	-54.080	45.325
212	424.0	-54.080	45.280
213	426.0	-54.152	45.190
214	428.0	-54.152	45.145
215	430.0	-54.152	45.100
216	432.0	-54.152	45.010
217	434.0	-54.152	44.965
218	436.0	-54.152	44.875
219	438.0	-54.152	44.830
220	440.0	-54.152	44.784
221	442.0	-54.152	44.739
222	444.0	-54.152	44.694
223	446.0	-54.152	44.604
224	448.0	-54.224	44.559
225	450.0	-54.224	44.514
226	452.0	-54.224	44.469
227	454.0	-54.224	44.424
228	456.0	-54.224	44.379
229	458.0	-54.224	44.289
230	460.0	-54.224	44.244
231	462.0	-54.224	44.199
232	464.0	-54.224	44.154
233	466.0	-54.224	44.064
234	468.0	-54.296	44.064
235	470.0	-54.296	43.973
236	472.0	-54.296	43.928
237	474.0	-54.296	43.883
238	476.0	-54.296	43.838
239	478.0	-54.296	43.793
240	480.0	-54.296	43.748
241	482.0	-54.296	43.703
242	484.0	-54.296	43.613
243	486.0	-54.296	43.568
244	488.0	-54.296	43.523
245	490.0	-54.296	43.523
246	492.0	-54.296	43.433
247	494.0	-54.368	43.388
248	496.0	-54.368	43.343
249	498.0	-54.368	43.298
250	500.0	-54.368	43.253
251	502.0	-54.368	43.208
252	504.0	-54.368	43.162
253	506.0	-54.368	43.117
254	508.0	-54.368	43.072

Number of Readings 324 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-13-43

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
255	510.0	-54.368	43.027
256	512.0	-54.368	42.982
257	514.0	-54.368	42.937
258	516.0	-54.368	42.892
259	518.0	-54.368	42.847
260	520.0	-54.440	42.802
261	522.0	-54.440	42.757
262	524.0	-54.440	42.712
263	526.0	-54.440	42.667
264	528.0	-54.440	42.577
265	530.0	-54.440	42.577
266	532.0	-54.440	42.532
267	534.0	-54.440	42.487
268	536.0	-54.440	42.442
269	538.0	-54.440	42.397
270	540.0	-54.440	42.352
271	542.0	-54.440	42.306
272	544.0	-54.440	42.261
273	546.0	-54.440	42.216
274	548.0	-54.512	42.171
275	550.0	-54.440	42.171
276	552.0	-54.512	42.081
277	554.0	-54.440	42.081
278	556.0	-54.512	42.036
279	558.0	-54.512	41.991
280	560.0	-54.512	41.946
281	562.0	-54.512	41.946
282	564.0	-54.512	41.901
283	566.0	-54.512	41.856
284	568.0	-54.512	41.811
285	570.0	-54.512	41.766
286	572.0	-54.512	41.766
287	574.0	-54.512	41.721
288	576.0	-54.512	41.676
289	578.0	-54.512	41.631
290	580.0	-54.584	41.586
291	582.0	-54.584	41.541
292	584.0	-54.584	41.541
293	586.0	-54.584	41.495
294	588.0	-54.584	41.450
295	590.0	-54.584	41.405
296	592.0	-54.584	41.360
297	594.0	-54.584	41.360
298	596.0	-54.584	41.315
299	598.0	-54.584	41.270
300	600.0	-54.584	41.270
301	602.0	-54.584	41.225
302	604.0	-54.584	41.180
303	606.0	-54.584	41.135
304	608.0	-54.584	41.135
305	610.0	-54.584	41.090

Number of Readings 324 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-13-43

	Time		displ7		LOAD18		LOAD	
	Secs	mm	KN	NON				
	1	2	3	4				
306	612.0	-54.584	41.045	66.4				
307	614.0	-54.584	41.000	66.3				
308	616.0	-54.584	41.000	66.2				
309	618.0	-54.584	40.955	66.2				
310	620.0	-54.656	40.910	66.1				
311	622.0	-54.656	40.865	66.0				
312	624.0	-54.656	40.820	66.0				
313	626.0	-54.656	40.820	65.9				
314	628.0	-54.656	40.775	65.9				
315	630.0	-54.656	40.775	65.9				
316	632.0	-54.656	40.730	65.8				
317	634.0	-54.656	40.730	65.8				
318	636.0	-54.656	40.684	65.7				
319	638.0	-54.656	40.639	65.7				
320	640.0	-54.656	40.639	65.6				
321	642.0	-54.656	40.594	65.6				
322	644.0	-54.656	40.549	65.5				
323	646.0	-54.656	40.504	65.4				
324	648.0	-54.656	40.459	65.4				

CALCULATION SHEET FOR TEST Triax 10

With membrane protection.

Peak axial load difference [kN] = 30.2
Axial compression [mm] = 48.25

} combination
giving
peak stress

Cross - sectional area [m²] = 0.0593

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 509

Cell pressure, σ_3 [kN/m²] = 100

Angle of friction, $\phi = 45.88^\circ$

Axial load difference at 10% axial strain [kN] = 25.68

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 456

Cell pressure, σ_3 [kN/m²] = 100

Angle of friction, $\phi = 44.1^\circ$

Cross - sectional area =
$$\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$$

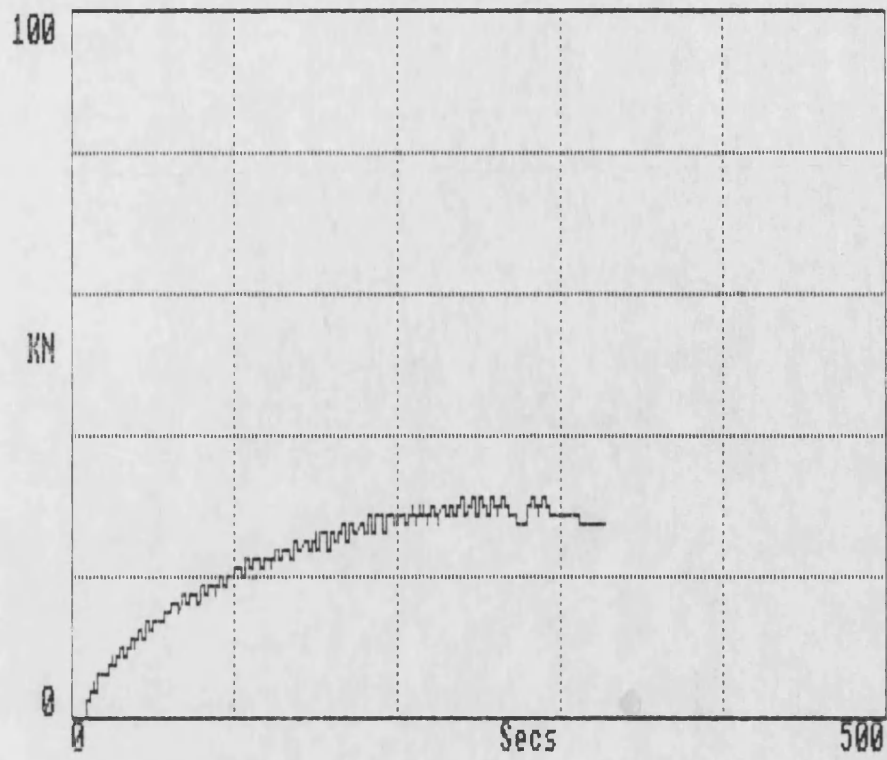
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

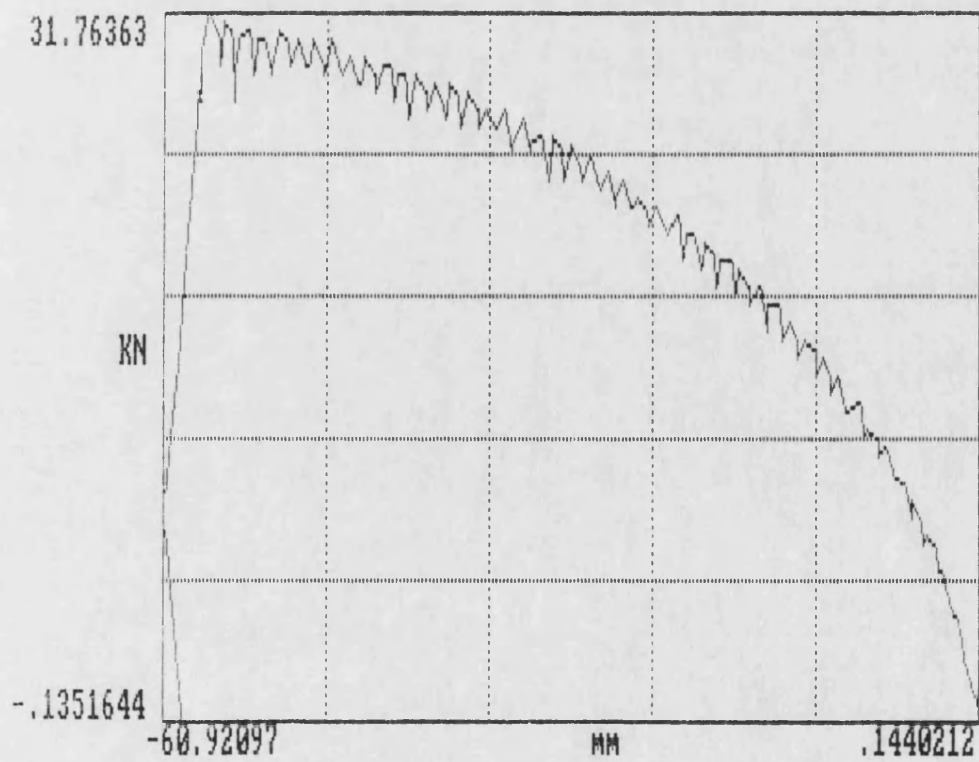
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v displ7



Number of Readings 286 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-16-27

Time Secs	1	displ mm	2	LOAD KN	3	LOAD NON	4
0	0.0	0.144	0.225	5			
1	2.0	0.144	0.225	4			
2	4.0	0.144	0.225	4			
3	6.0	0.144	0.225	4			
4	8.0	0.144	0.225	4			
5	10.0	0.144	0.225	4			
6	12.0	0.144	0.225	4			
7	14.0	0.144	0.225	3			
8	16.0	0.144	0.225	4			
9	18.0	0.144	0.225	5			
10	20.0	0.144	0.360	7			
11	22.0	-1.080	2.568	43			
12	24.0	-1.296	3.785	64			
13	26.0	-1.656	4.641	78			
14	28.0	-1.728	4.641	78			
15	30.0	-2.232	6.037	101			
16	32.0	-2.664	6.578	110			
17	34.0	-2.808	6.758	114			
18	36.0	-3.312	7.794	130			
19	38.0	-3.817	8.290	138			
20	40.0	-3.889	7.840	130			
21	42.0	-4.249	9.191	154			
22	44.0	-4.825	9.642	161			
23	46.0	-4.969	9.056	152			
24	48.0	-5.401	10.543	177			
25	50.0	-6.121	10.858	181			
26	52.0	-6.193	10.948	184			
27	54.0	-6.841	12.030	201			
28	56.0	-7.345	11.714	195			
29	58.0	-7.489	12.796	215			
30	60.0	-8.209	13.201	221			
31	62.0	-8.497	12.660	211			
32	64.0	-8.857	14.147	237			
33	66.0	-9.649	13.922	231			
34	68.0	-9.793	13.742	230			
35	70.0	-10.370	15.274	255			
36	72.0	-10.874	14.778	246			
37	74.0	-11.018	15.409	258			
38	76.0	-11.594	16.130	270			
39	78.0	-12.026	15.589	260			
40	80.0	-12.170	16.445	276			
41	82.0	-12.818	16.896	281			
42	84.0	-13.322	16.355	272			
43	86.0	-13.394	16.941	285			
44	88.0	-14.114	17.707	297			
45	90.0	-14.546	16.941	282			
46	92.0	-14.762	18.427	310			
47	94.0	-15.554	18.698	312			
48	96.0	-15.770	17.571	293			
49	98.0	-16.346	19.374	324			
50	100.0	-17.067	18.427	306			

Number of Readings 286 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-16-27

Time Secs	1	displ mm	2	LOAD KN	3	LOAD NON	4
51	102.0	-17.139	19.329	324			
52	104.0	-17.931	20.004	334			
53	106.0	-18.291	18.923	314			
54	108.0	-18.507	20.500	344			
55	110.0	-19.227	20.545	343			
56	112.0	-19.515	19.554	325			
57	114.0	-19.803	21.176	353			
58	116.0	-20.523	21.446	356			
59	118.0	-20.811	20.139	334			
60	120.0	-21.171	21.897	364			
61	122.0	-21.891	21.761	361			
62	124.0	-22.035	20.635	342			
63	126.0	-22.467	22.663	377			
64	128.0	-23.259	21.761	360			
65	130.0	-23.403	22.347	372			
66	132.0	-24.052	23.023	383			
67	134.0	-24.484	22.122	366			
68	136.0	-24.700	23.113	385			
69	138.0	-25.276	23.564	390			
70	140.0	-25.780	22.798	377			
71	142.0	-25.924	23.113	384			
72	144.0	-26.572	24.014	397			
73	146.0	-27.004	23.203	383			
74	148.0	-27.148	22.978	382			
75	150.0	-27.724	24.735	409			
76	152.0	-28.300	23.699	391			
77	154.0	-28.444	24.059	399			
78	156.0	-29.092	25.186	417			
79	158.0	-29.596	24.104	398			
80	160.0	-29.812	25.096	417			
81	162.0	-30.460	25.636	423			
82	164.0	-30.893	24.239	400			
83	166.0	-31.109	25.726	426			
84	168.0	-31.901	26.087	432			
85	170.0	-32.189	24.375	402			
86	172.0	-32.693	26.357	435			
87	174.0	-33.341	25.907	426			
88	176.0	-33.485	25.411	420			
89	178.0	-34.133	27.033	446			
90	180.0	-34.709	25.907	425			
91	182.0	-34.781	25.456	421			
92	184.0	-35.357	27.213	448			
93	186.0	-35.933	26.672	437			
94	188.0	-36.077	26.627	439			
95	190.0	-36.725	27.574	453			
96	192.0	-37.229	26.672	437			
97	194.0	-37.374	27.213	451			
98	196.0	-38.166	28.204	464			
99	198.0	-38.598	26.672	438			
100	200.0	-38.886	28.294	467			
101	202.0	-39.534	28.430	467			

10% strain

Number of Readings 286 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-16-27

Time Secs	displ7 mm	LOAD18 KN	LOAD NON
102	204.0	-39.894	26.853 440
103	206.0	-39.966	27.709 457
104	208.0	-40.686	28.610 470
105	210.0	-41.118	27.438 449
106	212.0	-41.334	28.249 467
107	214.0	-41.982	28.880 474
108	216.0	-42.414	28.159 460
109	218.0	-42.558	27.168 445
110	220.0	-42.846	29.060 477
111	222.0	-43.494	29.150 477
112	224.0	-43.782	27.664 453
113	226.0	-44.070	29.376 482
114	228.0	-44.647	29.376 481
115	230.0	-45.007	28.024 457
116	232.0	-45.223	29.060 478
117	234.0	-45.799	29.601 485
118	236.0	-46.303	28.520 465
119	238.0	-46.447	28.610 471
120	240.0	-46.951	29.961 490
121	242.0	-47.527	28.970 472
122	244.0	-47.743	29.286 481
123	246.0	-48.391	30.412 497
124	248.0	-48.895	29.060 474
125	250.0	-49.039	29.376 481
126	252.0	-49.687	30.727 502
127	254.0	-50.191	29.241 477
128	256.0	-50.335	29.556 484
129	258.0	-50.984	30.727 500
130	260.0	-51.488	29.466 480
131	262.0	-51.704	30.412 497
132	264.0	-52.424	30.863 503
133	266.0	-52.856	29.511 480
134	268.0	-52.928	29.015 473
135	270.0	-53.360	30.727 501
136	272.0	-54.008	30.592 496
137	274.0	-54.224	29.060 473
138	276.0	-54.512	30.772 502
139	278.0	-55.232	30.727 499
140	280.0	-55.520	29.331 476
141	282.0	-55.592	28.655 467
142	284.0	-55.664	28.339 462
143	286.0	-55.664	28.114 459
144	288.0	-55.664	27.979 455
145	290.0	-55.664	27.869 455
146	292.0	-55.726	27.799 454
147	294.0	-55.860	27.998 503
148	296.0	-56.454	27.178 481
149	298.0	-56.774	27.506 481
150	300.0	-56.888	30.502 499
151	302.0	-57.680	31.764 516
152	304.0	-58.041	30.052 488

peak

Number of Readings 286 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-16-27

Time Secs	displ7 mm	LOAD18 KN	LOAD NON
153	306.0	-58.113	29.331 476
154	308.0	-58.185	28.925 471
155	310.0	-58.185	28.745 467
156	312.0	-58.185	28.565 465
157	314.0	-58.185	28.430 465
158	316.0	-58.257	28.385 462
159	318.0	-58.257	28.294 462
160	320.0	-58.257	28.249 460
161	322.0	-58.257	28.159 460
162	324.0	-58.257	28.159 459
163	326.0	-58.257	28.114 458
164	328.0	-58.257	28.069 457
165	330.0	-58.329	28.024 457
166	332.0	-58.257	27.979 456
167	334.0	-58.329	27.934 456
168	336.0	-58.257	27.934 456
169	338.0	-58.257	27.889 454
170	340.0	-58.329	27.889 454
171	342.0	-58.329	27.889 454
172	344.0	-58.329	27.844 455
173	346.0	-58.329	27.799 454
174	348.0	-58.329	27.799 454
175	350.0	-58.329	27.799 453
176	352.0	-58.329	27.754 453
177	354.0	-58.329	27.754 453
178	356.0	-58.329	27.709 451
179	358.0	-58.329	27.709 451
180	360.0	-58.329	27.709 452
181	362.0	-58.329	27.664 451
182	364.0	-58.329	27.664 450
183	366.0	-58.329	27.664 450
184	368.0	-58.329	27.619 451
185	370.0	-58.329	27.619 451
186	372.0	-58.329	27.619 450
187	374.0	-58.329	27.574 450
188	376.0	-58.329	27.574 449
189	378.0	-58.329	27.574 449
190	380.0	-58.329	27.528 449
191	382.0	-58.329	27.528 449
192	384.0	-58.329	27.528 449
193	386.0	-58.401	27.483 449
194	388.0	-58.329	27.483 449
195	390.0	-58.329	27.483 448
196	392.0	-58.329	27.483 447
197	394.0	-58.329	27.438 448
198	396.0	-58.329	27.438 447
199	398.0	-58.329	27.438 447
200	400.0	-58.401	27.388 447
201	402.0	-59.193	20.950 337
202	404.0	-59.841	16.220 264
203	406.0	-60.273	13.066 214

Number of Readings 286 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-16-27

	Time		displ7	LOAD18		LOAD
	Secs	mm		KN	NON	
	1	2		3	4	
204	408.0	-60.633	10.948	180		
205	410.0	-60.921	9.326	154		
206	412.0	-59.409	-0.135	-1		
207	414.0	-59.409	-0.090	1		
208	416.0	-59.409	-0.090	1		
209	418.0	-59.409	-0.090	0		
210	420.0	-59.409	-0.090	1		
211	422.0	-59.409	-0.090	1		
212	424.0	-59.409	-0.090	0		
213	426.0	-59.409	-0.045	0		
214	428.0	-59.409	-0.090	1		
215	430.0	-59.337	-0.090	0		
216	432.0	-59.337	-0.090	1		
217	434.0	-59.337	-0.090	0		
218	436.0	-59.409	-0.045	1		
219	438.0	-59.409	-0.045	1		
220	440.0	-59.409	-0.045	0		
221	442.0	-59.409	-0.045	0		
222	444.0	-59.409	-0.045	1		
223	446.0	-59.409	-0.045	1		
224	448.0	-59.409	-0.045	-0		
225	450.0	-59.337	-0.045	1		
226	452.0	-59.337	-0.045	0		
227	454.0	-59.337	-0.090	-0		
228	456.0	-59.409	-0.045	0		
229	458.0	-59.337	-0.045	0		
230	460.0	-59.337	-0.045	-0		
231	462.0	-59.337	-0.045	-0		
232	464.0	-59.337	-0.045	1		
233	466.0	-59.337	-0.045	1		
234	468.0	-59.337	-0.045	0		
235	470.0	-59.337	-0.045	0		
236	472.0	-59.337	-0.045	1		
237	474.0	-59.337	-0.045	1		
238	476.0	-59.337	-0.045	1		
239	478.0	-59.337	-0.045	1		
240	480.0	-59.337	-0.045	1		
241	482.0	-59.337	-0.045	1		
242	484.0	-59.337	-0.045	1		
243	486.0	-59.337	-0.045	-0		
244	488.0	-59.337	-0.045	1		
245	490.0	-59.337	-0.045	1		
246	492.0	-59.409	-0.045	0		
247	494.0	-59.337	-0.045	1		
248	496.0	-59.409	-0.045	1		
249	498.0	-59.337	-0.045	1		
250	500.0	-59.337	-0.045	1		
251	502.0	-59.337	-0.045	1		
252	504.0	-59.337	-0.045	1		
253	506.0	-59.337	-0.045	0		
254	508.0	-59.337	-0.045	1		

Number of Readings 286 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-16-27

	Time		displ7	LOAD18		LOAD
	Secs	mm		KN	NON	
	1	2		3	4	
255	510.0	-59.337	-0.045	0		
256	512.0	-59.337	-0.045	0		
257	514.0	-59.337	-0.045	1		
258	516.0	-59.337	-0.045	1		
259	518.0	-59.337	-0.045	0		
260	520.0	-59.337	-0.045	1		
261	522.0	-59.337	-0.045	2		
262	524.0	-59.337	-0.045	1		
263	526.0	-59.337	-0.045	1		
264	528.0	-59.337	-0.045	1		
265	530.0	-59.337	-0.045	1		
266	532.0	-59.337	-0.045	0		
267	534.0	-59.337	-0.045	1		
268	536.0	-59.337	-0.045	0		
269	538.0	-59.337	-0.045	0		
270	540.0	-59.337	-0.045	1		
271	542.0	-59.337	-0.045	0		
272	544.0	-59.337	-0.045	1		
273	546.0	-59.337	-0.045	0		
274	548.0	-59.337	-0.045	1		
275	550.0	-59.337	-0.045	1		
276	552.0	-59.337	-0.045	1		
277	554.0	-59.337	-0.045	0		
278	556.0	-59.337	-0.045	1		
279	558.0	-59.337	-0.045	0		
280	560.0	-59.337	-0.045	1		
281	562.0	-59.337	-0.045	1		
282	564.0	-59.337	-0.045	1		
283	566.0	-59.337	-0.045	0		
284	568.0	-59.337	-0.000	0		
285	570.0	-59.337	-0.045	1		
286	572.0	-59.337	-0.045	1		

CALCULATION SHEET FOR TEST Triax 11

Peak axial load difference [kN] = 36.22
Axial compression [mm] = 43.0
Cross - sectional area [m²] = 0.0582
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 622
Cell pressure, σ_3 [kN/m²] = 100
Angle of friction, $\phi = 49.20^\circ$

} combination
giving
peak stress

Axial load difference at 10% axial strain [kN] = 33.0
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 586
Cell pressure, σ_3 [kN/m²] = 100
Angle of friction, $\phi = 48.27^\circ$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

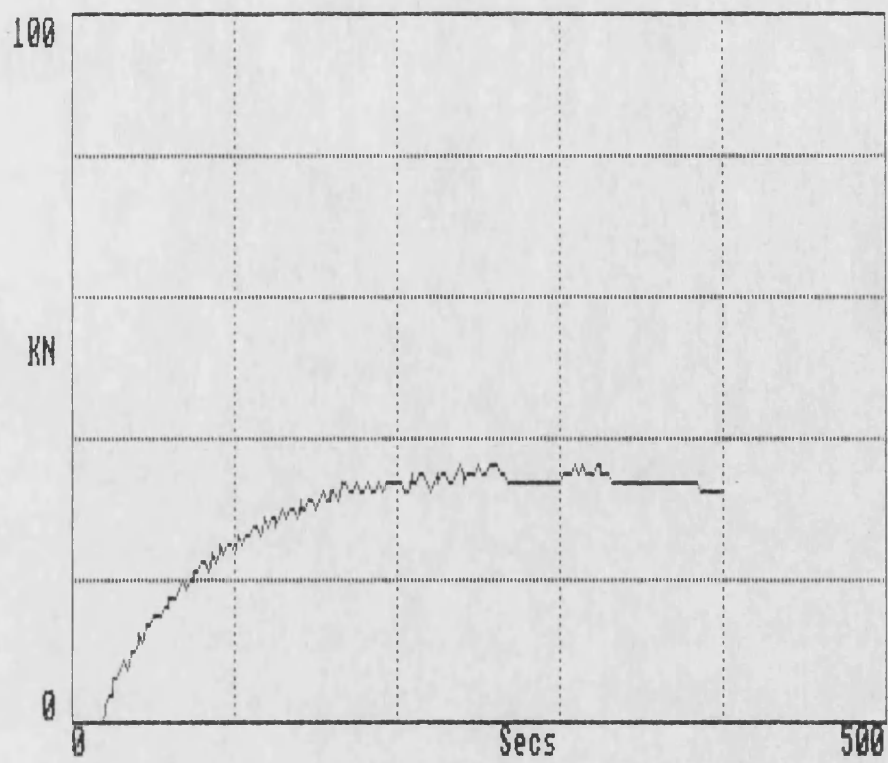
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

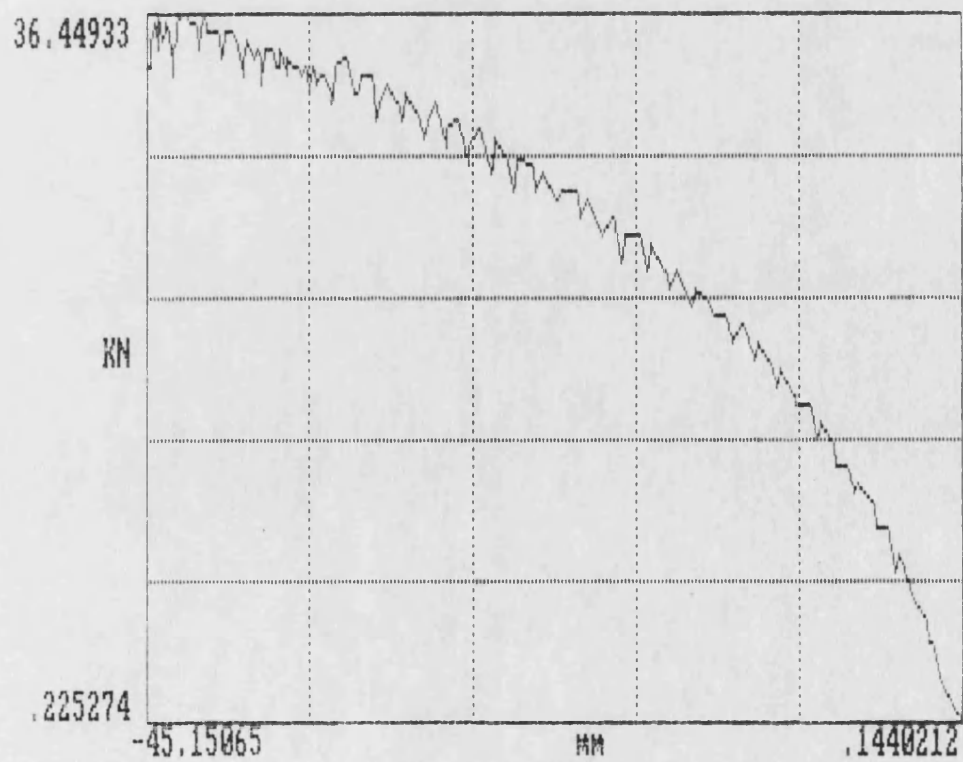
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v displ7



Number of Readings 200 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-04-40

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
0	0.0	0.144	0.225
1	2.0	0.144	0.225
2	4.0	0.144	0.225
3	6.0	0.144	0.225
4	8.0	0.144	0.225
5	10.0	0.144	0.225
6	12.0	0.144	0.225
7	14.0	0.144	0.225
8	16.0	0.144	0.225
9	18.0	0.144	0.315
10	20.0	-1.008	2.613
11	22.0	-1.440	4.235
12	24.0	-1.728	4.370
13	26.0	-1.944	5.677
14	28.0	-2.592	6.578
15	30.0	-2.736	7.119
16	32.0	-3.457	8.696
17	34.0	-3.601	8.110
18	36.0	-4.033	10.047
19	38.0	-4.537	10.047
20	40.0	-4.825	11.354
21	42.0	-5.617	12.255
22	44.0	-5.761	11.804
23	46.0	-6.337	13.561
24	48.0	-6.769	13.381
25	50.0	-6.985	14.598
26	52.0	-7.633	15.409
27	54.0	-7.921	14.598
28	56.0	-8.353	16.445
29	58.0	-9.073	16.535
30	60.0	-9.217	17.166
31	62.0	-9.937	18.112
32	64.0	-10.226	17.301
33	66.0	-10.514	18.788
34	68.0	-11.234	19.554
35	70.0	-11.450	18.608
36	72.0	-12.098	20.410
37	74.0	-12.602	19.599
38	76.0	-12.962	21.176
39	78.0	-13.682	20.905
40	80.0	-13.970	21.897
41	82.0	-14.690	22.437
42	84.0	-14.978	21.356
43	86.0	-15.698	23.519
44	88.0	-16.202	22.572
45	90.0	-16.346	23.519
46	92.0	-17.067	24.555
47	94.0	-17.427	23.248
48	96.0	-17.715	25.005
49	98.0	-18.507	25.276
50	100.0	-18.723	23.879

Number of Readings 200 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-04-40

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
51	102.0	-19.227	25.952
52	104.0	-19.803	25.276
53	106.0	-19.947	25.771
54	108.0	-20.595	26.718
55	110.0	-21.027	25.861
56	112.0	-21.315	27.303
57	114.0	-22.179	27.483
58	116.0	-22.323	26.853
59	118.0	-23.043	28.159
60	120.0	-23.475	27.258
61	122.0	-23.764	28.565
62	124.0	-24.484	29.060
63	126.0	-24.772	27.619
64	128.0	-25.132	29.331
65	130.0	-25.852	30.097
66	132.0	-25.996	28.294
67	134.0	-26.644	30.457
68	136.0	-27.148	29.826
69	138.0	-27.292	28.655
70	140.0	-27.868	31.133
71	142.0	-28.444	30.502
72	144.0	-28.516	29.466
73	146.0	-29.020	31.764
74	148.0	-29.596	31.223
75	150.0	-29.740	30.007
76	152.0	-30.028	31.674
77	154.0	-30.677	32.575
78	156.0	-30.965	30.817
79	158.0	-31.181	31.854
80	160.0	-31.685	32.665
81	162.0	-32.189	32.124
82	164.0	-32.333	31.043
83	166.0	-32.693	33.205
84	168.0	-33.125	33.250
85	170.0	-33.485	32.259
86	172.0	-33.629	32.530
87	174.0	-34.133	33.971
88	176.0	-34.709	33.611
89	178.0	-34.853	32.124
90	180.0	-34.925	32.485
91	182.0	-35.213	33.431
92	184.0	-35.501	33.115
93	186.0	-35.645	32.800
94	188.0	-35.861	33.566
95	190.0	-36.005	32.800
96	192.0	-36.149	32.169
97	194.0	-36.293	33.521
98	196.0	-36.437	33.431
99	198.0	-36.653	33.836
100	200.0	-36.869	33.881
101	202.0	-37.229	34.106

10% strain

Number of Readings 200 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-04-40

	Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
102	204.0	-37.301	33.070	540
103	206.0	-37.374	33.070	541
104	208.0	-37.662	34.467	562
105	210.0	-37.806	33.926	554
106	212.0	-37.950	33.836	554
107	214.0	-38.166	34.602	565
108	216.0	-38.526	34.647	564
109	218.0	-38.670	33.476	546
110	220.0	-38.670	33.025	538
111	222.0	-38.742	34.061	557
112	224.0	-38.886	34.422	562
113	226.0	-39.102	34.061	554
114	228.0	-39.318	34.782	566
115	230.0	-39.534	35.143	574
116	232.0	-39.822	34.061	554
117	234.0	-39.894	33.431	545
118	236.0	-40.110	35.278	575
119	238.0	-40.470	35.638	581
120	240.0	-40.902	35.593	579
121	242.0	-41.046	34.242	557
122	244.0	-41.190	35.503	580
123	246.0	-41.478	35.458	577
124	248.0	-41.766	35.413	577
125	250.0	-41.982	36.224	591
126	252.0	-42.270	35.008	568
127	254.0	-42.342	34.647	564
128	256.0	-42.486	35.999	586
129	258.0	-42.774	36.044	587
130	260.0	-43.134	36.449	593
131	262.0	-43.422	35.954	584
132	264.0	-43.566	35.008	570
133	266.0	-43.638	34.557	562
134	268.0	-43.638	34.332	559
135	270.0	-43.638	34.106	555
136	272.0	-43.710	33.971	552
137	274.0	-43.710	33.836	552
138	276.0	-43.710	33.746	550
139	278.0	-43.710	33.701	549
140	280.0	-43.710	33.611	547
141	282.0	-43.710	33.566	547
142	284.0	-43.710	33.476	546
143	286.0	-43.782	33.431	545
144	288.0	-43.782	33.386	545
145	290.0	-43.782	33.341	544
146	292.0	-43.782	33.296	543
147	294.0	-43.782	33.250	542
148	296.0	-43.782	33.250	542
149	298.0	-43.782	33.205	541
150	300.0	-43.782	33.926	555
151	302.0	-43.854	35.143	572
152	304.0	-43.854	35.008	571

Number of Readings 200 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-04-40

	Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
153	306.0	-43.926	35.548	578
154	308.0	-44.070	35.999	586
155	310.0	-44.215	35.728	581
156	312.0	-44.287	35.098	571
157	314.0	-44.431	36.224	590
158	316.0	-44.503	35.368	575
159	318.0	-44.575	34.917	568
160	320.0	-44.575	34.647	564
161	322.0	-44.647	36.044	587
162	324.0	-44.791	35.774	581
163	326.0	-44.863	35.098	571
164	328.0	-44.935	34.737	565
165	330.0	-44.935	34.512	562
166	332.0	-44.935	34.332	559
167	334.0	-44.935	34.197	557
168	336.0	-45.007	34.106	555
169	338.0	-45.007	34.016	553
170	340.0	-45.007	33.926	552
171	342.0	-45.007	33.836	551
172	344.0	-45.007	33.791	550
173	346.0	-45.007	33.701	550
174	348.0	-45.007	33.701	549
175	350.0	-45.079	33.611	548
176	352.0	-45.079	33.566	546
177	354.0	-45.079	33.566	547
178	356.0	-45.079	33.521	546
179	358.0	-45.079	33.476	544
180	360.0	-45.079	33.431	545
181	362.0	-45.079	33.386	544
182	364.0	-45.079	33.386	545
183	366.0	-45.079	33.341	544
184	368.0	-45.079	33.341	542
185	370.0	-45.079	33.341	543
186	372.0	-45.079	33.296	543
187	374.0	-45.079	33.250	542
188	376.0	-45.079	33.250	542
189	378.0	-45.079	33.205	541
190	380.0	-45.079	33.205	542
191	382.0	-45.151	33.160	541
192	384.0	-45.079	33.160	540
193	386.0	-45.151	33.115	540
194	388.0	-45.151	33.115	539
195	390.0	-45.079	33.070	539
196	392.0	-45.079	33.070	538
197	394.0	-45.079	33.070	540
198	396.0	-45.151	33.025	539
199	398.0	-45.151	33.025	538
200	400.0	-45.079	33.025	537

CALCULATION SHEET FOR TEST Triax 12

Load response jump. Adjusted peak used.

Peak axial load difference [kN] = 30
Axial compression [mm] = 35

} combination
giving
peak stress

Cross - sectional area [m²] = 0.0567

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 529

Cell pressure, σ_3 [kN/m²] = 100

Angle of friction, $\phi = 46.53^\circ$

Axial load difference at 10% axial strain [kN] = 27.44

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 487

Cell pressure, σ_3 [kN/m²] = 100

Angle of friction, $\phi = 45.19^\circ$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height - axial comp.}}$

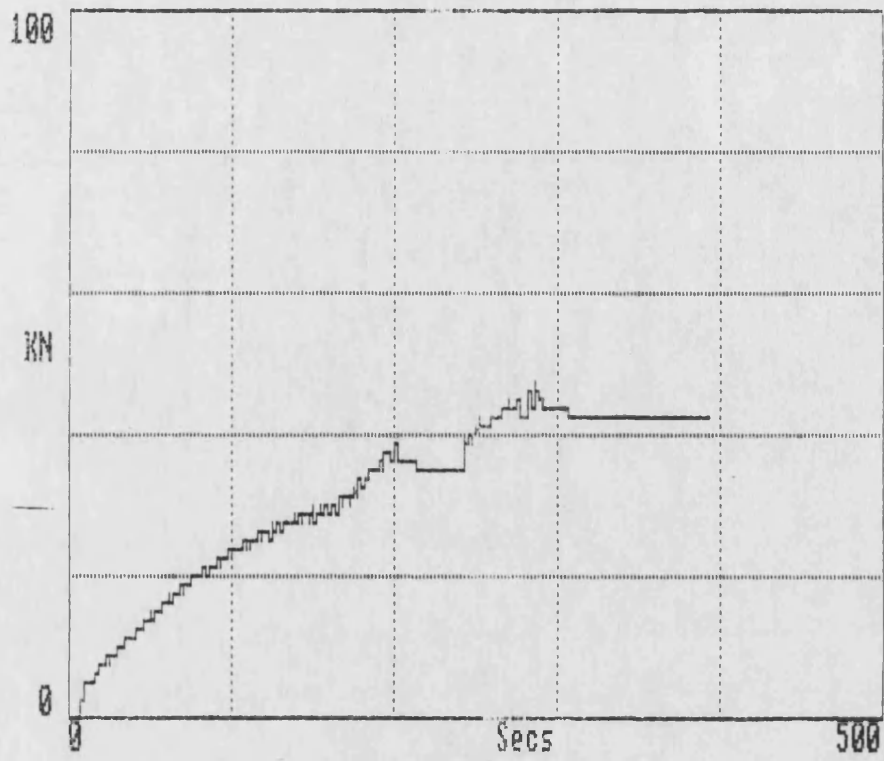
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

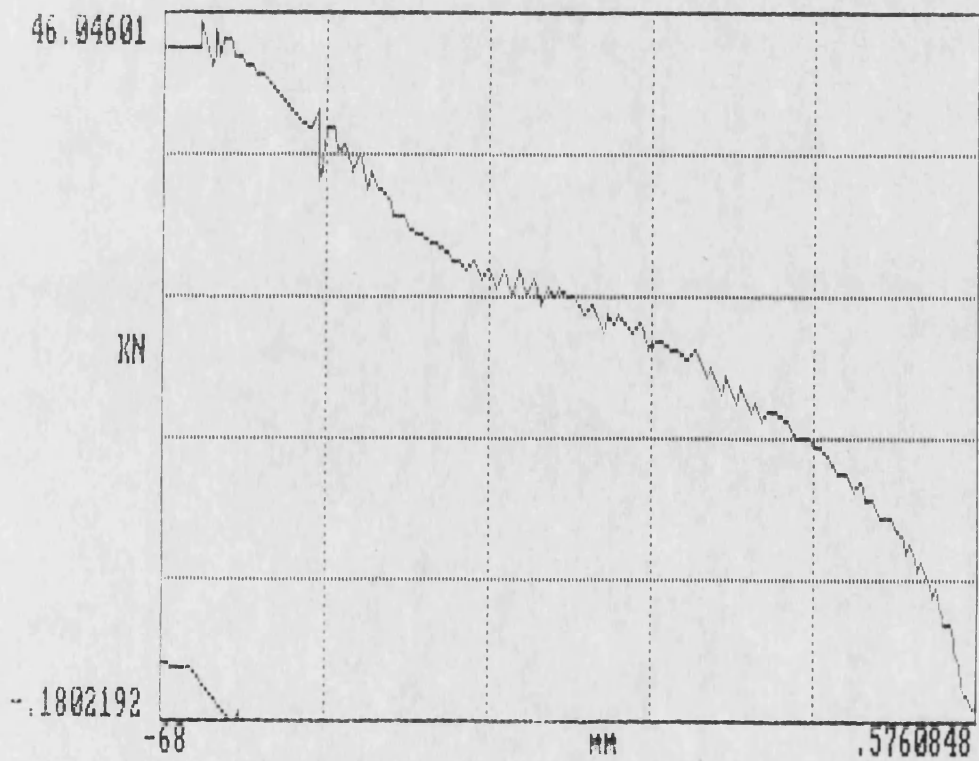
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v disp17



Number of Readings 278 No. Columns 4
Date Task Started 01/01/83 Time Task Started 03-16-28

Time Secs 1	displ mm 2	LOAD18 KN 3	LOAD NON 4	
0	0.0	0.576	0.225	4
1	2.0	0.576	0.225	4
2	4.0	0.576	0.225	3
3	6.0	0.576	0.225	4
4	8.0	0.576	0.225	3
5	10.0	0.576	0.225	5
6	12.0	0.576	0.225	4
7	14.0	0.504	0.315	5
8	16.0	-0.648	2.298	41
9	18.0	-1.080	3.920	67
10	20.0	-1.224	4.460	74
11	22.0	-1.368	5.226	90
12	24.0	-1.728	6.082	104
13	26.0	-2.088	6.263	104
14	28.0	-2.160	6.983	118
15	30.0	-2.448	7.524	126
16	32.0	-2.952	8.380	140
17	34.0	-3.168	7.975	133
18	36.0	-3.384	9.056	151
19	38.0	-3.745	9.597	160
20	40.0	-4.105	10.137	168
21	42.0	-4.321	9.777	163
22	44.0	-4.609	10.813	180
23	46.0	-5.113	11.264	187
24	48.0	-5.401	10.903	181
25	50.0	-5.689	12.075	201
26	52.0	-6.409	12.390	206
27	54.0	-6.697	13.111	219
28	56.0	-7.633	13.381	222
29	58.0	-8.065	14.147	235
30	60.0	-8.641	14.057	232
31	62.0	-9.145	15.229	253
32	64.0	-9.793	14.913	247
33	66.0	-10.442	16.130	267
34	68.0	-11.018	15.769	260
35	70.0	-11.882	17.166	285
36	72.0	-12.386	17.481	290
37	74.0	-12.962	17.661	292
38	76.0	-13.394	18.067	290
39	78.0	-14.042	18.292	303
40	80.0	-14.402	18.518	306
41	82.0	-14.978	18.833	310
42	84.0	-15.410	19.374	320
43	86.0	-16.202	19.869	328
44	88.0	-16.778	20.185	334
45	90.0	-17.355	19.644	324
46	92.0	-18.003	20.770	344
47	94.0	-18.507	20.185	332
48	96.0	-19.227	21.716	358
49	98.0	-19.803	20.815	342
50	100.0	-20.595	22.212	366

Number of Readings 278 No. Columns 4
Date Task Started 01/01/83 Time Task Started 03-16-28

Time Secs 1	displ mm 2	LOAD18 KN 3	LOAD NON 4	
51	102.0	-21.027	21.266	349
52	104.0	-21.891	22.843	376
53	106.0	-22.323	22.257	365
54	108.0	-23.259	23.879	392
55	110.0	-23.691	23.609	389
56	112.0	-24.556	24.194	396
57	114.0	-24.916	23.969	393
58	116.0	-25.852	24.825	407
59	118.0	-26.284	24.690	405
60	120.0	-27.220	24.285	397
61	122.0	-27.796	25.591	420
62	124.0	-28.444	25.231	412
63	126.0	-29.524	26.312	430
64	128.0	-29.884	25.771	422
65	130.0	-30.532	26.492	433
66	132.0	-31.037	25.186	410
67	134.0	-31.757	26.898	439
68	136.0	-32.333	26.402	432
69	138.0	-33.269	27.664	451
70	140.0	-33.773	27.528	449
71	142.0	-34.565	28.024	457
72	144.0	-34.925	27.619	451
73	146.0	-35.645	27.979	454
74	148.0	-36.077	27.078	439
75	150.0	-36.797	28.655	466
76	152.0	-37.374	27.438	444
77	154.0	-38.166	29.015	471
78	156.0	-38.670	27.664	448
79	158.0	-39.390	29.286	476
80	160.0	-39.966	27.934	452
81	162.0	-40.470	29.286	476
82	164.0	-41.190	28.655	464
83	166.0	-41.838	29.601	480
84	168.0	-42.558	29.060	469
85	170.0	-42.918	29.826	484
86	172.0	-43.710	29.916	483
87	174.0	-44.070	30.277	492
88	176.0	-44.863	30.953	502
89	178.0	-45.295	31.088	504
90	180.0	-46.087	31.583	512
91	182.0	-46.519	31.809	516
92	184.0	-47.383	32.124	518
93	186.0	-47.743	32.530	526
94	188.0	-48.607	32.890	530
95	190.0	-49.039	33.836	549
96	192.0	-49.687	34.287	554
97	194.0	-49.975	34.692	561
98	196.0	-50.623	35.368	571
99	198.0	-50.984	34.737	561
100	200.0	-51.632	36.675	593
101	202.0	-52.208	35.458	570

Number of Readings 278 No. Columns 4
Date Task Started 01/01/83 Time Task Started 03-16-28

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4	
102	204.0	-52.784	37.531	607
103	206.0	-53.360	36.630	589
104	208.0	-53.720	38.252	618
105	210.0	-54.440	38.342	616
106	212.0	-54.656	36.765	591
107	214.0	-54.656	36.269	585
108	216.0	-54.728	36.044	581
109	218.0	-54.728	35.864	578
110	220.0	-54.728	35.728	576
111	222.0	-54.728	35.593	574
112	224.0	-54.728	35.503	573
113	226.0	-54.800	35.458	572
114	228.0	-54.728	35.368	571
115	230.0	-54.800	35.323	570
116	232.0	-54.800	35.278	570
117	234.0	-54.800	35.233	568
118	236.0	-54.800	35.188	568
119	238.0	-54.800	35.143	568
120	240.0	-54.800	35.143	566
121	242.0	-54.800	35.098	566
122	244.0	-54.800	35.053	565
123	246.0	-54.800	35.053	566
124	248.0	-54.800	35.008	565
125	250.0	-54.800	35.008	564
126	252.0	-54.944	39.603	645
127	254.0	-55.664	38.252	615
128	256.0	-56.312	39.108	629
129	258.0	-57.104	39.468	634
130	260.0	-57.752	40.009	643
131	262.0	-58.401	40.775	656
132	264.0	-58.977	41.405	666
133	266.0	-59.625	41.766	672
134	268.0	-60.057	42.081	676
135	270.0	-60.489	42.352	680
136	272.0	-60.993	42.667	686
137	274.0	-61.497	43.253	695
138	276.0	-62.001	43.253	695
139	278.0	-62.505	44.289	713
140	280.0	-63.009	44.199	710
141	282.0	-63.297	43.162	692
142	284.0	-63.585	44.875	721
143	286.0	-63.729	43.253	694
144	288.0	-63.801	42.712	686
145	290.0	-63.873	42.442	682
146	292.0	-64.810	45.460	729
147	294.0	-65.026	43.973	706
148	296.0	-589.911	46.046	736
149	298.0	-589.911	45.190	727
150	300.0	-589.911	44.334	712
151	302.0	-589.911	43.883	705
152	304.0	-589.911	43.748	702

Number of Readings 278 No. Columns 4
Date Task Started 01/01/83 Time Task Started 03-16-28

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4	
153	306.0	-589.911	43.568	699
154	308.0	-589.911	43.433	698
155	310.0	-589.911	43.343	696
156	312.0	-589.911	43.208	695
157	314.0	-589.911	43.162	693
158	316.0	-589.911	43.072	693
159	318.0	-589.911	43.027	691
160	320.0	-589.911	42.982	690
161	322.0	-589.911	42.892	690
162	324.0	-589.911	42.892	689
163	326.0	-589.911	42.847	688
164	328.0	-589.911	42.802	687
165	330.0	-589.911	42.757	687
166	332.0	-589.911	42.712	686
167	334.0	-589.911	42.712	685
168	336.0	-589.911	42.667	686
169	338.0	-589.911	42.667	685
170	340.0	-589.911	42.622	685
171	342.0	-589.911	42.622	685
172	344.0	-589.911	42.577	684
173	346.0	-589.911	42.532	684
174	348.0	-589.911	42.532	683
175	350.0	-589.911	42.532	684
176	352.0	-589.911	42.487	682
177	354.0	-589.911	42.442	683
178	356.0	-589.911	42.442	682
179	358.0	-589.911	42.442	682
180	360.0	-589.911	42.397	681
181	362.0	-589.911	42.397	681
182	364.0	-589.911	42.352	681
183	366.0	-589.911	42.352	680
184	368.0	-589.911	42.352	679
185	370.0	-589.911	42.306	680
186	372.0	-589.911	42.306	679
187	374.0	-589.911	42.306	680
188	376.0	-589.911	42.261	680
189	378.0	-589.911	42.261	680
190	380.0	-589.911	42.216	678
191	382.0	-589.911	42.216	678
192	384.0	-589.911	42.216	679
193	386.0	-589.911	42.171	678
194	388.0	-589.911	42.171	679
195	390.0	-589.911	42.171	677
196	392.0	-589.911	42.126	678
197	394.0	-589.911	42.126	677
198	396.0	-589.911	42.126	677
199	398.0	-589.911	42.081	677
200	400.0	-589.911	42.081	677
201	402.0	-589.911	42.081	676
202	404.0	-589.911	42.081	676
203	406.0	-589.911	42.036	677

Number of Readings 278 No. Columns 4
Date Task Started 01/01/83 Time Task Started 03-16-28

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
204	408.0	-589.911	42.036 675
205	410.0	-589.911	42.036 675
206	412.0	-589.911	42.036 675
207	414.0	-589.911	42.036 674
208	416.0	-589.911	41.991 675
209	418.0	-589.911	41.991 675
210	420.0	-589.911	41.991 675
211	422.0	-589.911	41.946 675
212	424.0	-589.911	41.946 675
213	426.0	-589.911	41.946 674
214	428.0	-589.911	41.946 674
215	430.0	-589.911	41.901 675
216	432.0	-589.911	41.901 674
217	434.0	-589.911	41.901 673
218	436.0	-589.911	41.901 674
219	438.0	-589.911	41.901 674
220	440.0	-589.911	41.856 674
221	442.0	-589.911	41.856 673
222	444.0	-589.911	41.856 673
223	446.0	-589.911	41.856 674
224	448.0	-589.911	41.856 672
225	450.0	-589.911	41.856 673
226	452.0	-589.911	41.811 672
227	454.0	-589.911	41.811 672
228	456.0	-589.911	41.811 672
229	458.0	-589.911	41.811 672
230	460.0	-589.911	41.766 672
231	462.0	-589.911	41.766 672
232	464.0	-589.911	41.766 673
233	466.0	-589.911	41.766 671
234	468.0	-589.911	41.766 672
235	470.0	-589.911	41.766 671
236	472.0	-589.911	41.721 672
237	474.0	-589.911	41.721 671
238	476.0	-589.911	41.721 671
239	478.0	-589.911	41.721 671
240	480.0	-589.911	41.721 671
241	482.0	-589.911	41.721 -0
242	484.0	-589.911	41.676 1
243	486.0	-65.386	3.379 2
244	488.0	-62.001	0.000 2
245	490.0	-61.497	0.090 2
246	492.0	-61.569	0.225 3
247	494.0	-61.569	0.180 3
248	496.0	-61.641	-0.090 3
249	498.0	-64.017	-0.180 2
250	500.0	-64.089	-0.180 2
251	502.0	-64.089	-0.135 2
252	504.0	-64.017	-0.135 2
253	506.0	-64.017	-0.135 2
254	508.0	-64.017	-0.135 2

Number of Readings 278 No. Columns 4
Date Task Started 01/01/83 Time Task Started 03-16-28

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
255	510.0	-63.945	-0.135 2
256	512.0	-64.017	-0.135 2
257	514.0	-63.945	-0.135 3
258	516.0	-64.017	-0.135 3
259	518.0	-64.017	-0.135 3
260	520.0	-64.017	-0.135 3
261	522.0	-64.017	-0.135 3
262	524.0	-64.017	-0.135 3
263	526.0	-64.017	-0.135 3
264	528.0	-64.017	-0.135 3
265	530.0	-64.017	-0.135 3
266	532.0	-64.017	-0.090 3
267	534.0	-64.017	-0.135 3
268	536.0	-64.017	-0.090 3
269	538.0	-64.017	-0.090 3
270	540.0	-64.017	-0.090 3
271	542.0	-64.017	-0.090 3
272	544.0	-64.017	-0.090 3
273	546.0	-64.017	-0.090 3
274	548.0	-64.017	-0.090 3
275	550.0	-64.017	-0.090 4
276	552.0	-63.945	-0.090 3
277	554.0	-64.017	-0.135 3
278	556.0	-63.945	-0.090 3

CALCULATION SHEET FOR TEST Triax 13

Peak axial load difference [kN] = 43.52
Axial compression [mm] = 51.56
Cross - sectional area [m²] = 0.060
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 726
Cell pressure, σ_3 [kN/m²] = 100
Angle of friction, $\phi = 51.62^\circ$

} combination
giving
peak stress

Axial load difference at 10% axial strain [kN] = 26.94
Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 479
Cell pressure, σ_3 [kN/m²] = 100
Angle of friction, $\phi = 44.85^\circ$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

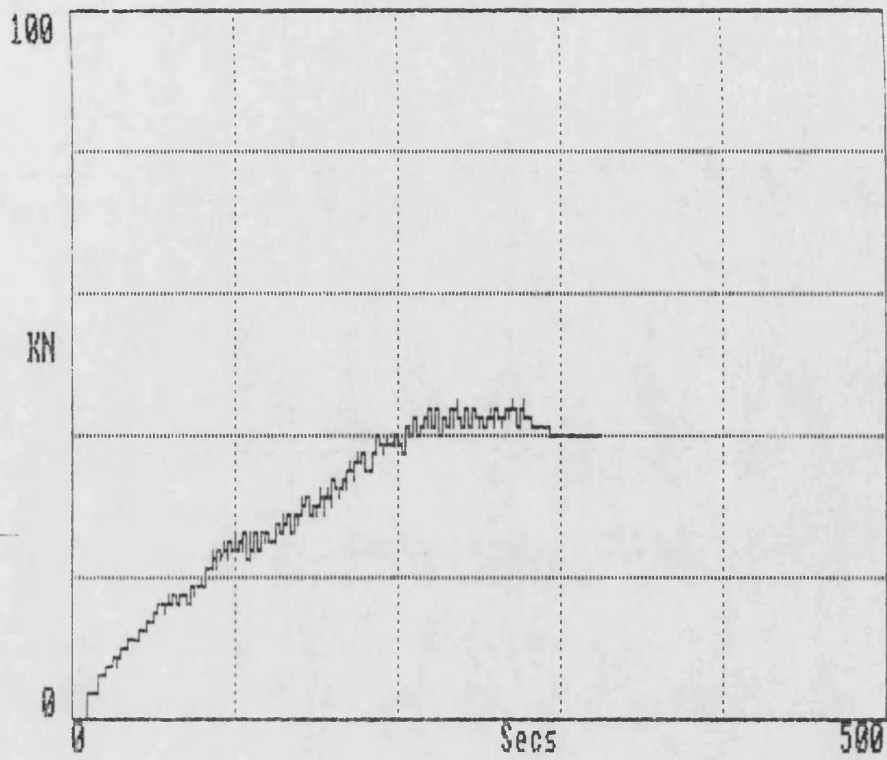
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

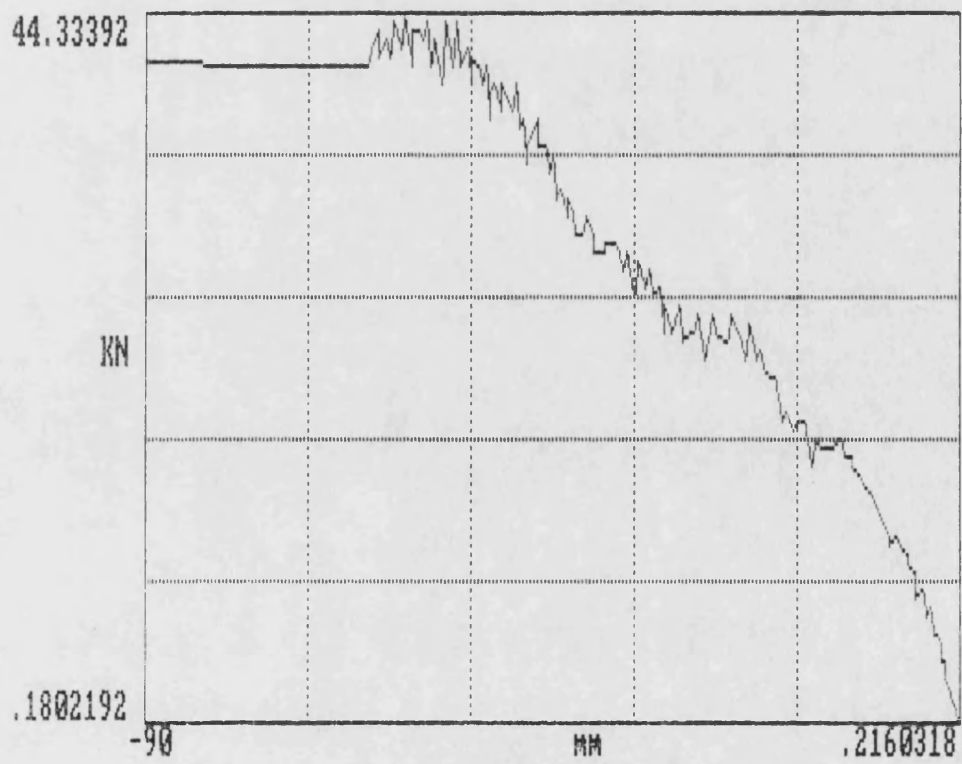
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v disp17



Number of Readings 260 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-22-42

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
0	0.0	0.216	0.225 3
1	2.0	0.216	0.180 2
2	4.0	0.216	0.180 3
3	6.0	0.216	0.180 2
4	8.0	0.216	0.225 2
5	10.0	0.216	0.180 2
6	12.0	0.216	0.180 3
7	14.0	0.216	0.180 2
8	16.0	0.216	0.180 3
9	18.0	0.216	0.180 3
10	20.0	0.144	0.451 7
11	22.0	-1.296	2.884 47
12	24.0	-1.584	3.785 61
13	26.0	-1.800	4.010 64
14	28.0	-1.944	4.956 82
15	30.0	-2.448	5.947 98
16	32.0	-2.592	5.722 94
17	34.0	-3.096	7.209 120
18	36.0	-3.529	7.074 116
19	38.0	-3.961	8.290 137
20	40.0	-4.537	8.065 132
21	42.0	-4.897	9.462 157
22	44.0	-5.329	10.002 166
23	46.0	-5.545	10.588 177
24	48.0	-6.337	10.993 181
25	50.0	-6.553	11.714 197
26	52.0	-7.561	11.038 181
27	54.0	-7.705	12.030 203
28	56.0	-8.497	12.660 209
29	58.0	-8.569	12.841 214
30	60.0	-9.505	14.282 237
31	62.0	-9.577	14.463 241
32	64.0	-10.514	15.229 253
33	66.0	-10.730	15.724 263
34	68.0	-11.594	15.949 264
35	70.0	-11.738	16.715 280
36	72.0	-12.674	16.760 277
37	74.0	-12.890	17.661 295
38	76.0	-13.898	17.031 281
39	78.0	-14.186	17.301 288
40	80.0	-14.978	17.121 282
41	82.0	-15.626	17.932 298
42	84.0	-16.274	16.220 268
43	86.0	-17.067	18.878 314
44	88.0	-17.355	18.923 316
45	90.0	-18.291	18.608 307
46	92.0	-19.155	19.509 320
47	94.0	-19.515	18.788 310
48	96.0	-20.379	21.446 355
49	98.0	-20.595	21.716 360
50	100.0	-21.387	22.167 364

Number of Readings 260 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-22-42

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
51	102.0	-21.891	23.474 388
52	104.0	-22.395	22.798 376
53	106.0	-23.187	24.870 411
54	108.0	-23.547	23.068 380
55	110.0	-24.556	24.285 398
56	112.0	-25.132	25.636 423
57	114.0	-25.780	23.969 394
58	116.0	-26.860	24.600 403
59	118.0	-27.220	25.681 423
60	120.0	-28.228	22.798 375
61	122.0	-29.020	25.771 426
62	124.0	-29.452	24.375 402
63	126.0	-30.460	24.059 393
64	128.0	-30.893	25.952 428
65	130.0	-31.613	24.600 400
66	132.0	-32.405	26.177 430
67	134.0	-32.693	25.186 412
68	136.0	-32.765	24.555 402
69	138.0	-33.125	27.168 448
70	140.0	-33.629	26.627 435
71	142.0	-34.205	28.294 463
72	144.0	-34.637	27.213 444
73	146.0	-35.501	28.610 467
74	148.0	-35.933	26.627 434
75	150.0	-36.797	29.331 480
76	152.0	-36.941	28.385 464
77	154.0	-37.806	30.142 490
78	156.0	-37.950	29.826 487
79	158.0	-39.030	30.097 489
80	160.0	-39.246	29.241 479
81	162.0	-40.254	29.286 475
82	164.0	-40.470	30.457 498
83	166.0	-41.118	31.628 514
84	168.0	-41.622	30.682 501
85	170.0	-42.414	30.637 497
86	172.0	-42.702	31.989 523
87	174.0	-43.350	32.710 531
88	176.0	-43.422	31.583 513
89	178.0	-44.215	33.025 537
90	180.0	-44.503	32.755 531
91	182.0	-44.863	35.323 575
92	184.0	-45.439	34.647 560
93	186.0	-45.727	36.314 581
94	188.0	-46.375	36.224 578
95	190.0	-46.663	37.486 593
96	192.0	-47.599	35.819 578
97	194.0	-47.571	34.872 564
98	196.0	-48.031	37.801 614
99	198.0	-48.607	37.170 600
100	200.0	-49.039	39.874 646
101	202.0	-49.543	38.477 621

Number of Readings 260 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-22-42

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
102	204.0	-49.831	39.017
103	206.0	-50.551	40.054
104	208.0	-50.695	38.522
105	210.0	-51.344	40.054
106	212.0	-51.776	39.243
107	214.0	-51.848	38.297
108	216.0	-51.920	37.846
109	218.0	-52.424	41.180
110	220.0	-52.784	40.189
111	222.0	-52.928	41.225
112	224.0	-53.792	41.360
113	226.0	-53.936	39.964
114	228.0	-54.512	42.352
115	230.0	-54.944	41.180
116	232.0	-55.520	43.748
117	234.0	-56.096	41.225
118	236.0	-56.600	43.523
119	238.0	-57.392	39.964
120	240.0	-57.897	42.712
121	242.0	-58.401	41.225
122	244.0	-58.833	43.928
123	246.0	-59.337	42.442
124	248.0	-59.553	43.433
125	250.0	-60.417	43.117
126	252.0	-60.633	41.225
127	254.0	-61.281	44.244
128	256.0	-61.641	42.261
129	258.0	-61.785	42.216
130	260.0	-62.577	44.019
131	262.0	-62.937	41.315
132	264.0	-63.513	42.442
133	266.0	-64.017	41.631
134	268.0	-64.233	43.072
135	270.0	-65.098	42.352
136	272.0	-65.242	41.000
137	274.0	-589.911	44.199
138	276.0	-589.911	41.586
139	278.0	-589.911	44.064
140	280.0	-589.911	42.892
141	282.0	-589.911	44.334
142	284.0	-589.911	43.253
143	286.0	-589.911	41.631
144	288.0	-589.911	43.928
145	290.0	-589.911	43.208
146	292.0	-589.911	42.306
147	294.0	-589.911	41.766
148	296.0	-589.911	41.450
149	298.0	-589.911	41.180
150	300.0	-589.911	41.000
151	302.0	-589.911	40.820
152	304.0	-589.911	40.730

peak

TRANSDUCER READINGS OUT
OF RANGE.

Number of Readings 260 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-22-42

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
153	306.0	-589.911	40.639
154	308.0	-589.911	40.504
155	310.0	-589.911	40.414
156	312.0	-589.911	40.324
157	314.0	-589.911	40.234
158	316.0	-589.911	40.144
159	318.0	-589.911	40.099
160	320.0	-589.911	40.009
161	322.0	-589.911	39.964
162	324.0	-589.911	39.874
163	326.0	-589.911	39.828
164	328.0	-589.911	39.783
165	330.0	-589.911	39.738
166	332.0	-589.911	39.693
167	334.0	-589.911	39.648
168	336.0	-589.911	39.603
169	338.0	-589.911	39.558
170	340.0	-589.911	39.513
171	342.0	-589.911	39.513
172	344.0	-589.911	39.468
173	346.0	-589.911	39.378
174	348.0	-589.911	39.333
175	350.0	-589.911	39.333
176	352.0	-589.911	39.288
177	354.0	-589.911	39.288
178	356.0	-589.911	39.243
179	358.0	-589.911	39.198
180	360.0	-589.911	39.198
181	362.0	-589.911	39.153
182	364.0	-589.911	39.153
183	366.0	-589.911	39.108
184	368.0	-589.911	39.108
185	370.0	-589.911	39.063
186	372.0	-589.911	39.017
187	374.0	-589.911	39.017
188	376.0	-589.911	38.972
189	378.0	-589.911	38.927
190	380.0	-589.911	38.927
191	382.0	-589.911	38.927
192	384.0	-589.911	38.882
193	386.0	-589.911	38.882
194	388.0	-589.911	38.882
195	390.0	-589.911	38.837
196	392.0	-589.911	38.837
197	394.0	-589.911	38.792
198	396.0	-589.911	38.792
199	398.0	-589.911	38.792
200	400.0	-589.911	38.747
201	402.0	-589.911	38.747
202	404.0	-589.911	38.747
203	406.0	-589.911	38.702

=====				
Number of Readings 260 No. Columns 4				
Date Task Started 01/01/83 Time Task Started 00-22-42				
=====				
	Time	displ7	LOAD18	LOAD
	Secs	mm	KN	NON
	1	2	3	4

204	408.0	-589.911	38.702	622
205	410.0	-589.911	38.702	621
206	412.0	-589.911	38.657	621
207	414.0	-589.911	38.657	621
208	416.0	-589.911	38.657	621
209	418.0	-589.911	38.612	621
210	420.0	-589.911	38.612	620
211	422.0	-589.911	38.612	620
212	424.0	-589.911	38.612	620
213	426.0	-589.911	38.612	620
214	428.0	-589.911	38.567	620
215	430.0	-589.911	38.567	620
216	432.0	-589.911	38.567	619
217	434.0	-589.911	38.567	619
218	436.0	-589.911	38.522	619
219	438.0	-589.911	38.522	619
220	440.0	-589.911	38.522	619
221	442.0	-589.911	38.522	619
222	444.0	-589.911	38.522	618
223	446.0	-589.911	38.477	618
224	448.0	-589.911	38.477	619
225	450.0	-589.911	38.477	618
226	452.0	-589.911	38.477	619
227	454.0	-589.911	38.477	618
228	456.0	-589.911	38.477	618
229	458.0	-589.911	38.477	617
230	460.0	-589.911	38.477	617
231	462.0	-589.911	38.432	618
232	464.0	-589.911	38.432	617
233	466.0	-589.911	38.432	618
234	468.0	-589.911	38.432	617
235	470.0	-589.911	38.432	617
236	472.0	-589.911	38.387	616
237	474.0	-589.911	38.387	617
238	476.0	-589.911	38.387	618
239	478.0	-589.911	38.342	616
240	480.0	-589.911	38.342	617
241	482.0	-589.911	38.342	616
242	484.0	-589.911	38.297	616
243	486.0	-589.911	38.342	616
244	488.0	-589.911	38.297	616
245	490.0	-589.911	38.297	617
246	492.0	-589.911	38.297	616
247	494.0	-589.911	38.297	614
248	496.0	-589.911	38.252	615
249	498.0	-589.911	38.252	615
250	500.0	-589.911	38.252	616
251	502.0	-589.911	38.252	614
252	504.0	-589.911	38.252	615
253	506.0	-589.911	38.252	614
254	508.0	-589.911	38.252	614

=====				
Number of Readings 260 No. Columns 4				
Date Task Started 01/01/83 Time Task Started 00-22-42				
=====				
	Time	displ7	LOAD18	LOAD
	Secs	mm	KN	NON
	1	2	3	4

255	510.0	-589.911	38.206	614
256	512.0	-589.911	38.206	615
257	514.0	-589.911	38.206	614
258	516.0	-589.911	38.206	614
259	518.0	-589.911	38.206	614
260	520.0	-589.911	38.206	613

CALCULATION SHEET FOR TEST Triax 14

Peak axial load difference [kN] = 61.45
Axial compression [mm] = 62.30

} combination
giving
peak stress

Cross - sectional area [m²] = 0.0623

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 985

Cell pressure, σ_3 [kN/m²] = 200

Angle of friction, $\phi = 45.38^\circ$

Axial load difference at 10% axial strain [kN] = 39.33

Stress difference, $\sigma_1 - \sigma_3$ [kN/m²] = 699

Cell pressure, σ_3 [kN/m²] = 200

Angle of friction, $\phi = 39.50^\circ$

Cross - sectional area = $\frac{\text{Sample volume}}{\text{Orig. height} - \text{axial comp.}}$

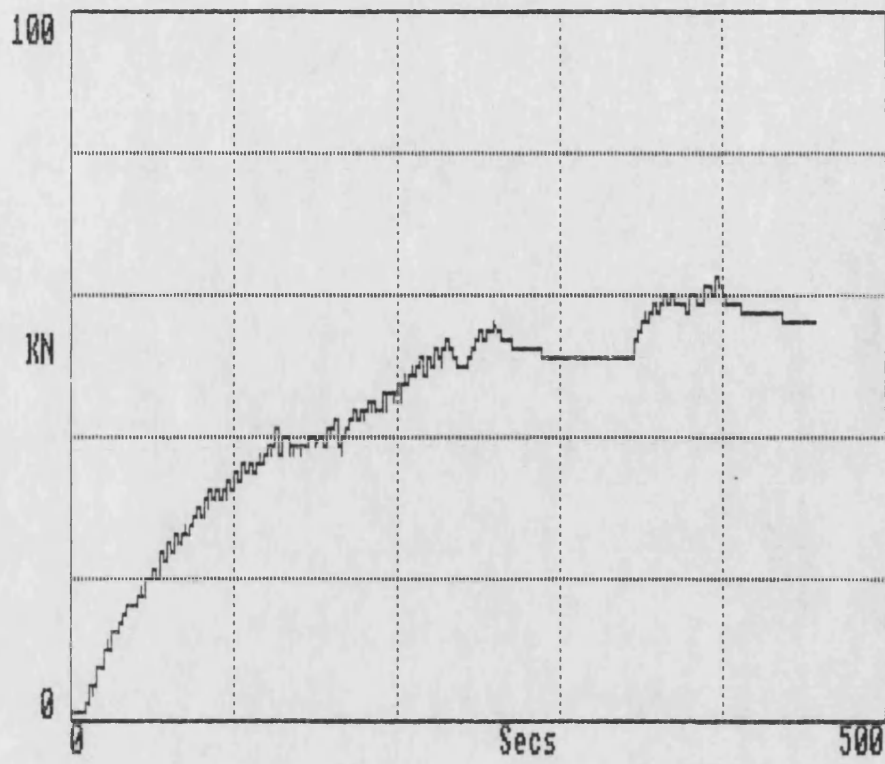
Angle of friction determined from; $\sin \phi = \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3}$

Sample volume = 0.017 m³,

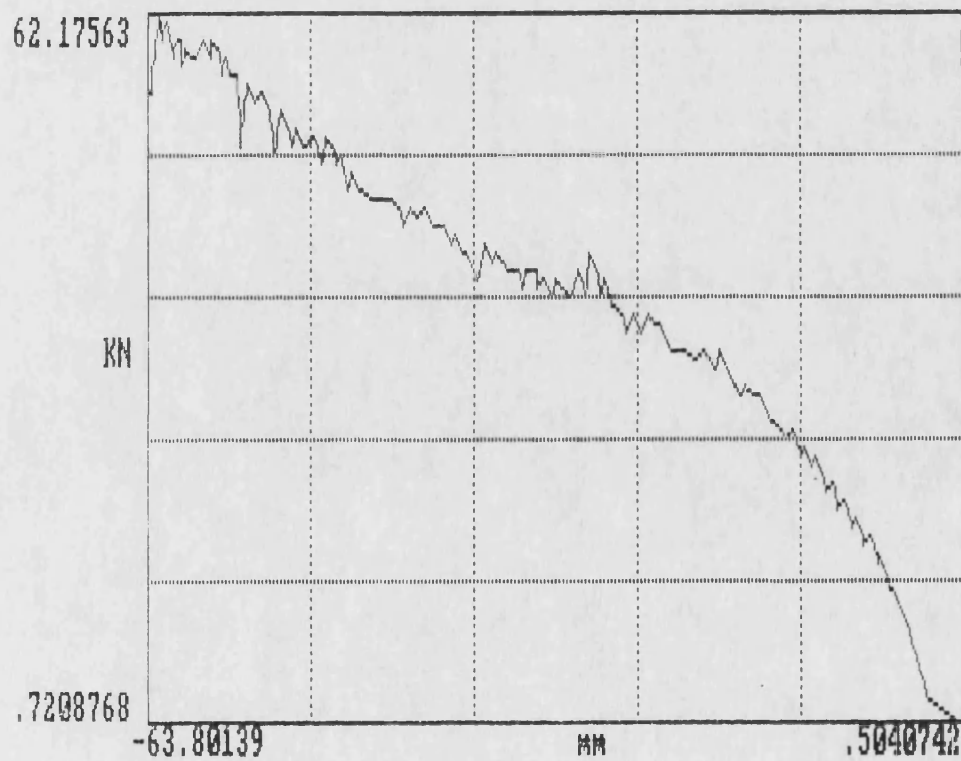
Original sample height = 335mm,

Cross - sectional area at 10% strain = 0.0563 m²

LOAD18 v Time



LOAD18 v disp17



Number of Readings 317 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-31-19

Time Secs 1	disp17 mm 2	LOAD18 KN 3	LOAD NON 4
0	0.0	0.504	0.721 11
1	2.0	0.504	0.721 10
2	4.0	0.504	0.721 11
3	6.0	0.504	0.721 10
4	8.0	0.504	0.721 11
5	10.0	0.504	0.721 10
6	12.0	0.504	0.721 10
7	14.0	0.504	0.721 11
8	16.0	0.504	0.721 10
9	18.0	0.072	1.081 18
10	20.0	-2.304	3.064 52
11	22.0	-2.736	4.505 74
12	24.0	-2.880	5.362 90
13	26.0	-3.312	7.164 121
14	28.0	-3.529	7.344 122
15	30.0	-3.817	9.146 152
16	32.0	-4.321	10.678 179
17	34.0	-4.465	10.678 179
18	36.0	-4.969	12.480 210
19	38.0	-5.329	12.165 203
20	40.0	-5.617	14.147 239
21	42.0	-6.193	15.093 252
22	44.0	-6.337	14.868 248
23	46.0	-6.697	16.580 279
24	48.0	-7.273	16.265 271
25	50.0	-7.489	16.851 283
26	52.0	-7.993	18.112 303
27	54.0	-8.281	17.842 297
28	56.0	-8.785	20.139 337
29	58.0	-9.289	19.419 323
30	60.0	-9.721	21.176 353
31	62.0	-10.442	21.041 350
32	64.0	-10.586	22.077 369
33	66.0	-11.234	23.744 395
34	68.0	-11.450	22.843 380
35	70.0	-11.954	24.780 411
36	72.0	-12.458	24.014 397
37	74.0	-12.818	25.997 432
38	76.0	-13.538	25.501 421
39	78.0	-13.826	26.177 432
40	80.0	-14.546	27.168 449
41	82.0	-14.834	27.168 450
42	84.0	-15.626	28.880 476
43	86.0	-15.842	29.015 480
44	88.0	-16.634	30.052 495
45	90.0	-17.067	29.241 482
46	92.0	-17.787	30.682 504
47	94.0	-18.003	30.637 505
48	96.0	-18.723	32.710 537
49	98.0	-19.083	31.538 519
50	100.0	-19.947	33.115 545

Number of Readings 317 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-31-19

Time Secs 1	disp17 mm 2	LOAD18 KN 3	LOAD NON 4
51	102.0	-20.451	32.079 529
52	104.0	-21.315	32.800 537
53	106.0	-21.531	33.296 548
54	108.0	-22.539	33.025 539
55	110.0	-22.755	33.836 557
56	112.0	-23.547	35.053 571
57	114.0	-23.764	35.368 580
58	116.0	-24.484	35.909 586
59	118.0	-24.844	34.422 563
60	120.0	-25.492	36.089 592
61	122.0	-25.996	34.872 569
62	124.0	-26.356	36.359 595
63	126.0	-27.004	36.810 600
64	128.0	-27.220	37.170 606
65	130.0	-27.724	38.792 633
66	132.0	-28.084	37.801 615
67	134.0	-28.372	39.648 647
68	136.0	-28.876	41.360 674
69	138.0	-29.380	37.756 614
70	140.0	-29.524	39.423 643
71	142.0	-30.028	39.874 648
72	144.0	-30.605	37.711 614
73	146.0	-30.965	37.981 618
74	148.0	-31.541	38.882 634
75	150.0	-31.829	37.846 616
76	152.0	-32.405	39.288 640
77	154.0	-32.981	38.567 626
78	156.0	-33.197	39.648 647
79	158.0	-33.845	40.054 651
80	160.0	-34.277	38.297 623
81	162.0	-34.637	40.234 655
82	164.0	-35.357	39.964 649
83	166.0	-35.717	40.549 662
84	168.0	-36.221	41.721 679
85	170.0	-36.581	40.639 661
86	172.0	-37.085	42.442 690
87	174.0	-37.806	38.747 626
88	176.0	-38.094	40.009 654
89	178.0	-38.742	41.676 678
90	180.0	-38.958	41.090 669
91	182.0	-39.534	43.027 700
92	184.0	-39.966	42.306 685
93	186.0	-40.470	43.568 707
94	188.0	-40.974	43.478 704
95	190.0	-41.262	44.109 716
96	192.0	-41.982	45.325 735
97	194.0	-42.342	44.424 722
98	196.0	-43.134	44.920 727
99	198.0	-43.494	43.568 704
100	200.0	-43.782	45.190 733
101	202.0	-44.431	45.866 741

10%

Number of Readings 317 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-31-19

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
102	204.0	-44.935	45.821 743
103	206.0	-45.583	46.406 750
104	208.0	-46.015	46.091 747
105	210.0	-46.735	46.902 758
106	212.0	-47.023	46.542 754
107	214.0	-47.599	48.434 783
108	216.0	-47.959	47.037 759
109	218.0	-48.463	49.786 806
110	220.0	-48.967	48.975 790
111	222.0	-49.183	50.506 815
112	224.0	-49.759	51.408 830
113	226.0	-50.119	49.155 792
114	228.0	-50.695	51.498 832
115	230.0	-51.200	50.912 820
116	232.0	-51.416	50.867 823
117	234.0	-52.064	52.173 841
118	236.0	-52.424	50.551 815
119	238.0	-52.712	52.399 846
120	240.0	-53.144	53.525 863
121	242.0	-53.432	52.173 839
122	244.0	-53.504	51.362 828
123	246.0	-53.576	50.732 818
124	248.0	-53.648	50.281 812
125	250.0	-53.720	49.876 805
126	252.0	-53.720	49.560 800
127	254.0	-53.792	51.498 833
128	256.0	-54.080	53.075 857
129	258.0	-54.296	53.345 859
130	260.0	-54.656	55.282 890
131	262.0	-55.232	54.111 870
132	264.0	-55.448	54.516 880
133	266.0	-55.592	55.192 891
134	268.0	-55.808	56.003 904
135	270.0	-56.024	55.462 893
136	272.0	-56.096	54.787 882
137	274.0	-56.168	54.111 872
138	276.0	-56.240	53.660 865
139	278.0	-56.240	53.345 859
140	280.0	-56.312	53.075 856
141	282.0	-56.312	52.894 852
142	284.0	-56.312	52.624 848
143	286.0	-56.384	52.444 845
144	288.0	-56.384	52.309 844
145	290.0	-56.384	52.219 842
146	292.0	-56.384	52.083 840
147	294.0	-56.384	51.993 839
148	296.0	-56.384	51.948 838
149	298.0	-56.384	51.858 836
150	300.0	-56.384	51.768 836
151	302.0	-56.384	51.768 834
152	304.0	-56.456	51.678 834

Number of Readings 317 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-31-19

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
153	306.0	-56.456	51.633 833
154	308.0	-56.456	51.588 833
155	310.0	-56.456	51.543 831
156	312.0	-56.456	51.498 830
157	314.0	-56.456	51.408 829
158	316.0	-56.456	51.362 829
159	318.0	-56.456	51.362 829
160	320.0	-56.456	51.317 827
161	322.0	-56.456	51.272 826
162	324.0	-56.456	51.227 827
163	326.0	-56.456	51.182 826
164	328.0	-56.456	51.137 825
165	330.0	-56.528	51.092 825
166	332.0	-56.528	51.047 824
167	334.0	-56.528	51.047 824
168	336.0	-56.528	51.002 823
169	338.0	-56.528	51.002 823
170	340.0	-56.528	50.957 822
171	342.0	-56.528	50.912 822
172	344.0	-56.528	50.867 821
173	346.0	-56.528	50.867 821
174	348.0	-56.528	50.822 820
175	350.0	-56.528	50.822 820
176	352.0	-56.528	50.777 820
177	354.0	-56.528	50.777 820
178	356.0	-56.528	53.210 861
179	358.0	-56.672	55.462 895
180	360.0	-56.888	56.499 911
181	362.0	-57.104	56.499 909
182	364.0	-57.320	56.904 918
183	366.0	-57.680	57.986 934
184	368.0	-57.897	57.805 930
185	370.0	-57.969	56.994 917
186	372.0	-58.185	59.157 956
187	374.0	-58.689	59.833 964
188	376.0	-58.977	58.346 938
189	378.0	-59.481	60.013 965
190	380.0	-59.913	58.436 940
191	382.0	-60.417	58.706 946
192	384.0	-60.777	58.977 950
193	386.0	-60.993	58.256 936
194	388.0	-61.065	57.535 926
195	390.0	-61.281	60.148 965
196	392.0	-61.569	60.058 963
197	394.0	-61.785	59.157 952
198	396.0	-61.857	58.481 941
199	398.0	-62.073	59.788 963
200	400.0	-62.361	61.139 984
201	402.0	-62.577	60.644 975
202	404.0	-62.649	59.833 962
203	406.0	-62.793	62.176 1001

peak

Number of Readings 317 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-31-19

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
204	408.0	-62.937	61.229 983
205	410.0	-63.081	60.193 967
206	412.0	-63.153	59.517 957
207	414.0	-63.225	59.022 949
208	416.0	-63.225	58.751 944
209	418.0	-63.297	58.481 941
210	420.0	-63.297	58.346 938
211	422.0	-63.297	58.031 933
212	424.0	-63.369	57.850 931
213	426.0	-63.369	57.715 928
214	428.0	-63.369	57.535 926
215	430.0	-63.369	57.445 924
216	432.0	-63.441	57.355 923
217	434.0	-63.441	57.310 922
218	436.0	-63.441	57.220 921
219	438.0	-63.441	57.129 920
220	440.0	-63.441	57.084 918
221	442.0	-63.441	56.994 918
222	444.0	-63.441	56.949 917
223	446.0	-63.441	56.904 916
224	448.0	-63.513	56.724 913
225	450.0	-63.513	56.634 912
226	452.0	-63.513	56.544 910
227	454.0	-63.513	56.454 908
228	456.0	-63.513	56.409 908
229	458.0	-63.513	56.319 907
230	460.0	-63.513	56.273 906
231	462.0	-63.513	56.183 905
232	464.0	-63.585	56.138 903
233	466.0	-63.585	56.093 902
234	468.0	-63.585	56.048 902
235	470.0	-63.585	56.003 901
236	472.0	-63.585	55.958 899
237	474.0	-63.585	55.913 900
238	476.0	-63.585	55.868 899
239	478.0	-63.585	55.823 898
240	480.0	-63.585	55.778 898
241	482.0	-63.585	55.733 898
242	484.0	-63.585	55.733 897
243	486.0	-63.585	55.688 896
244	488.0	-63.585	55.643 896
245	490.0	-63.657	55.598 896
246	492.0	-63.657	55.553 895
247	494.0	-63.585	55.553 895
248	496.0	-63.585	55.508 895
249	498.0	-63.657	55.462 893
250	500.0	-63.657	55.417 892
251	502.0	-63.657	55.417 892
252	504.0	-63.657	55.372 892
253	506.0	-63.657	55.372 892
254	508.0	-63.657	55.327 891

Number of Readings 317 No. Columns 4
 Date Task Started 01/01/83 Time Task Started 00-31-19

Time Secs 1	displ7 mm 2	LOAD18 KN 3	LOAD NON 4
255	510.0	-63.657	55.282 891
256	512.0	-63.657	55.282 891
257	514.0	-63.657	55.237 890
258	516.0	-63.657	55.237 888
259	518.0	-63.657	55.192 889
260	520.0	-63.657	55.147 888
261	522.0	-63.657	55.147 888
262	524.0	-63.657	55.102 888
263	526.0	-63.657	55.102 888
264	528.0	-63.657	55.057 887
265	530.0	-63.657	55.057 886
266	532.0	-63.729	55.012 887
267	534.0	-63.729	55.012 886
268	536.0	-63.657	54.967 885
269	538.0	-63.729	54.922 886
270	540.0	-63.657	54.922 885
271	542.0	-63.657	54.877 884
272	544.0	-63.729	54.877 885
273	546.0	-63.729	54.877 885
274	548.0	-63.729	54.832 883
275	550.0	-63.729	54.832 882
276	552.0	-63.729	54.787 881
277	554.0	-63.729	54.787 882
278	556.0	-63.729	54.742 882
279	558.0	-63.729	54.742 881
280	560.0	-63.729	54.742 882
281	562.0	-63.729	54.697 881
282	564.0	-63.729	54.697 881
283	566.0	-63.729	54.697 880
284	568.0	-63.729	54.651 880
285	570.0	-63.729	54.651 880
286	572.0	-63.729	54.606 880
287	574.0	-63.729	54.606 879
288	576.0	-63.729	54.561 879
289	578.0	-63.729	54.561 878
290	580.0	-63.729	54.561 879
291	582.0	-63.729	54.516 879
292	584.0	-63.729	54.516 879
293	586.0	-63.729	54.471 876
294	588.0	-63.729	54.471 877
295	590.0	-63.729	54.471 877
296	592.0	-63.729	54.471 876
297	594.0	-63.729	54.426 876
298	596.0	-63.729	54.426 875
299	598.0	-63.729	54.426 877
300	600.0	-63.729	54.381 875
301	602.0	-63.729	54.381 876
302	604.0	-63.729	54.381 875
303	606.0	-63.729	54.336 876
304	608.0	-63.729	54.336 874
305	610.0	-63.801	54.336 874

Number of Readings 317 No. Columns 4
Date Task Started 01/01/83 Time Task Started 00-31-19

=====				
	Time	displ7	LOAD18	LOAD
	Secs	mm	KN	NON
	1	2	3	4

306	612.0	-63.801	54.291	875
307	614.0	-63.801	54.291	875
308	616.0	-63.729	54.291	875
309	618.0	-63.729	54.246	874
310	620.0	-63.729	54.246	874
311	622.0	-63.801	54.246	873
312	624.0	-63.801	54.201	873
313	626.0	-63.801	54.201	874
314	628.0	-63.801	54.201	873
315	630.0	-63.801	54.156	873
316	632.0	-63.801	54.156	873
317	634.0	-63.801	54.156	873

Data from dynamic verification tests. 254mm samples only.

Data output from ADU, utilising DIALOG software and screen dumps.

Includes;

1. Print out of logged data. Strain rings [mm or ADU units], internal LVDT [ADU units], external actuator load cell [kN], and actuator LVDT [mm].
2. Load vs stroke plot from external sensors. XY plot from Dialog software (Screen dump).
3. Selected strain ring vs stroke plots.
4. Selected strain ring vs load plots.

Number of Readings 200 No. Columns 8
Task Started 01/01/83 Time Task Started 00-11-00

Time Secs	STRING5/16STRING3/12STRING3/125STRING3/16LVDT					LOAD KN	STROKE MM
	1	2	3	4	5		
0.0	-175	164	162.0	-175.0	-1423	-1.8555	125.0742
0.0	-174	162	158.0	-176.0	-1429	-9.1552	123.5164
0.0	-172	168	145.0	-179.0	-1427	-12.3535	123.3680
0.0	-165	163	105.0	-182.0	-1425	-20.7519	122.9229
0.0	-168	173	153.0	-180.0	-1422	-11.3769	123.2196
0.0	-174	157	155.0	-172.0	-1422	-1.2451	124.1098
1.0	-174	175	150.0	-173.0	-1430	-1.1475	125.0000
1.0	-174	153	170.0	-175.0	-1425	-1.1475	125.4451
1.0	-175	173	151.0	-174.0	-1422	-1.8311	125.0742
1.0	-172	160	154.0	-174.0	-1424	-1.9775	124.2582
2.0	-172	164	140.0	-178.0	-1426	-12.1338	123.2938
2.0	-166	161	166.0	-183.0	-1422	-20.6542	122.9229
2.0	-168	159	137.0	-180.0	-1422	-11.1815	123.2196
2.0	-173	174	162.0	-172.0	-1427	-1.2451	124.1098
3.0	-174	161	142.0	-174.0	-1426	-1.1719	125.0000
3.0	-176	173	161.0	-175.0	-1422	-1.1475	125.4451
3.0	-176	162	141.0	-174.0	-1424	-1.8311	125.0742
3.0	-176	167	159.0	-175.0	-1427	-1.9775	124.2582
4.0	-173	160	150.0	-179.0	-1423	-11.9873	123.2680
4.0	-166	168	139.0	-182.0	-1422	-20.5322	122.9229
4.0	-168	160	160.0	-180.0	-1426	-11.0351	123.2196
4.0	-174	165	131.0	-174.0	-1430	-1.2645	124.1098
5.0	-174	160	159.0	-176.0	-1429	-1.1475	125.0000
5.0	-175	169	138.0	-175.0	-1423	-1.1475	125.4451
5.0	-176	162	162.0	-175.0	-1423	-1.8311	125.0742
5.0	-176	170	148.0	-175.0	-1427	-1.9775	124.2582
6.0	-173	164	159.0	-179.0	-1423	-11.8408	123.3680
6.0	-166	155	127.0	-181.0	-1425	-20.3613	122.9229
6.0	-169	165	134.0	-181.0	-1429	-10.4980	123.2938
7.0	-174	164	127.0	-179.0	-1422	-1.2645	124.1098
7.0	-176	172	133.0	-176.0	-1423	-1.1475	125.0742
7.0	-176	166	164.0	-176.0	-1423	-1.1475	125.4451
7.0	-176	171	142.0	-174.0	-1426	-1.8555	125.0742
7.0	-176	164	150.0	-175.0	-1428	-1.9531	124.2582
8.0	-174	163	153.0	-178.0	-1429	-11.8652	123.3680
8.0	-167	163	132.0	-182.0	-1425	-20.2636	122.9229
8.0	-170	163	146.0	-181.0	-1426	-10.5469	123.2938
8.0	-176	163	141.0	-178.0	-1426	-1.2451	124.1098
9.0	-177	169	147.0	-177.0	-1422	-1.1475	125.0742
9.0	-176	164	127.0	-177.0	-1428	-1.1475	125.4451
9.0	-177	168	145.0	-175.0	-1423	-1.8311	125.0742
9.0	-177	163	144.0	-175.0	-1422	-1.9775	124.1098
10.0	-174	163	148.0	-179.0	-1423	-11.8652	123.3680
10.0	-169	170	158.0	-182.0	-1423	-20.1171	122.9229
10.0	-170	162	153.0	-180.0	-1422	-10.5469	123.2938
10.0	-178	161	147.0	-179.0	-1423	-1.2451	124.1098
11.0	-177	163	132.0	-177.0	-1429	-1.1475	125.0742
11.0	-176	163	120.0	-176.0	-1423	-1.1475	125.4451
11.0	-177	171	139.0	-175.0	-1425	-1.8311	125.0742
11.0	-177	164	146.0	-175.0	-1424	-1.9775	124.2582
12.0	-173	163	153.0	-180.0	-1423	-11.5967	123.2938

Number of Readings 200 No. Columns 8
Task Started 01/01/83 Time Task Started 00-11-00

Time Secs	STRING5/16STRING3/12STRING3/125STRING3/16LVDT					LOAD KN	STROKE MM
	1	2	3	4	5		
12.0	-169	169	134.0	-184.0	-1423	-19.9951	122.9229
12.0	-170	165	135.0	-182.0	-1426	-10.4980	123.2938
12.0	-178	168	142.0	-179.0	-1422	-1.2451	124.1098
13.0	-178	176	163.0	-177.0	-1425	-1.1475	125.0742
13.0	-177	164	139.0	-177.0	-1423	-1.1475	125.4451
13.0	-178	166	140.0	-176.0	-1426	-1.8311	125.0742
14.0	-170	160	152.0	-185.0	-1423	-2.0019	124.1098
14.0	-174	172	138.0	-180.0	-1425	-11.5722	123.3680
14.0	-170	162	145.0	-182.0	-1430	-19.8242	122.9229
15.0	-171	162	145.0	-182.0	-1426	-10.3759	123.2196
15.0	-178	170	164.0	-177.0	-1426	-1.2451	124.1098
15.0	-177	167	169.0	-178.0	-1423	-1.1475	125.4451
15.0	-177	167	151.0	-177.0	-1426	-1.8311	125.0742
16.0	-173	171	143.0	-181.0	-1423	-1.9775	124.2582
16.0	-169	167	133.0	-177.0	-1423	-11.7431	123.2938
16.0	-173	171	150.0	-184.0	-1425	-19.7753	122.9229
16.0	-171	167	153.0	-183.0	-1423	-10.2783	123.2938
17.0	-180	169	140.0	-177.0	-1423	-1.2451	124.1098
17.0	-179	183	126.0	-176.0	-1424	-1.1475	125.0742
17.0	-176	160	145.0	-178.0	-1423	-1.1475	125.4451
17.0	-177	166	163.0	-178.0	-1427	-1.8311	125.1484
18.0	-178	167	147.0	-177.0	-1428	-1.9775	124.1098
18.0	-173	166	131.0	-181.0	-1429	-11.6211	123.3680
18.0	-169	171	150.0	-184.0	-1425	-19.7021	122.9229
18.0	-171	164	150.0	-183.0	-1424	-10.2051	123.2938
19.0	-181	172	165.0	-177.0	-1423	-1.1475	125.0742
19.0	-178	175	147.0	-178.0	-1427	-1.1475	125.4451
19.0	-177	174	141.0	-178.0	-1423	-1.1475	125.4451
20.0	-178	172	146.0	-178.0	-1426	-1.8311	125.0742
20.0	-178	168	156.0	-177.0	-1424	-1.9775	124.1098
20.0	-173	172	137.0	-180.0	-1423	-11.4013	123.2938
20.0	-172	164	154.0	-185.0	-1426	-19.6044	122.9229
20.0	-172	167	135.0	-182.0	-1425	-9.7656	123.2938
20.0	-180	167	127.0	-174.0	-1423	-1.2451	124.1098
21.0	-179	173	147.0	-178.0	-1429	-1.1475	125.0742
21.0	-177	164	151.0	-178.0	-1423	-1.1475	125.4451
21.0	-178	169	149.0	-178.0	-1427	-1.8311	125.0742
21.0	-178	166	158.0	-177.0	-1428	-1.9775	124.2582
22.0	-174	161	152.0	-180.0	-1428	-11.5722	123.3680
22.0	-169	161	163.0	-185.0	-1428	-19.5068	122.9229
22.0	-172	167	135.0	-182.0	-1427	-9.7908	123.2938
23.0	-179	178	154.0	-173.0	-1429	-1.2207	124.1098
23.0	-178	168	134.0	-177.0	-1423	-1.1475	125.0742
23.0	-178	164	150.0	-178.0	-1423	-1.1475	125.4451
23.0	-179	169	151.0	-177.0	-1423	-1.8311	125.0742
24.0	-179	171	148.0	-177.0	-1430	-1.9775	124.1098
24.0	-174	170	143.0	-181.0	-1423	-11.5769	123.3680
24.0	-169	172	132.0	-184.0	-1428	-19.4091	122.9229
24.0	-172	172	143.0	-183.0	-1425	-9.7412	123.2938
24.0	-179	183	150.0	-175.0	-1424	-1.2451	124.1098

Number of Readings 200 No. Columns 8
Task Started 01/01/83 Time Task Started 00-11-00

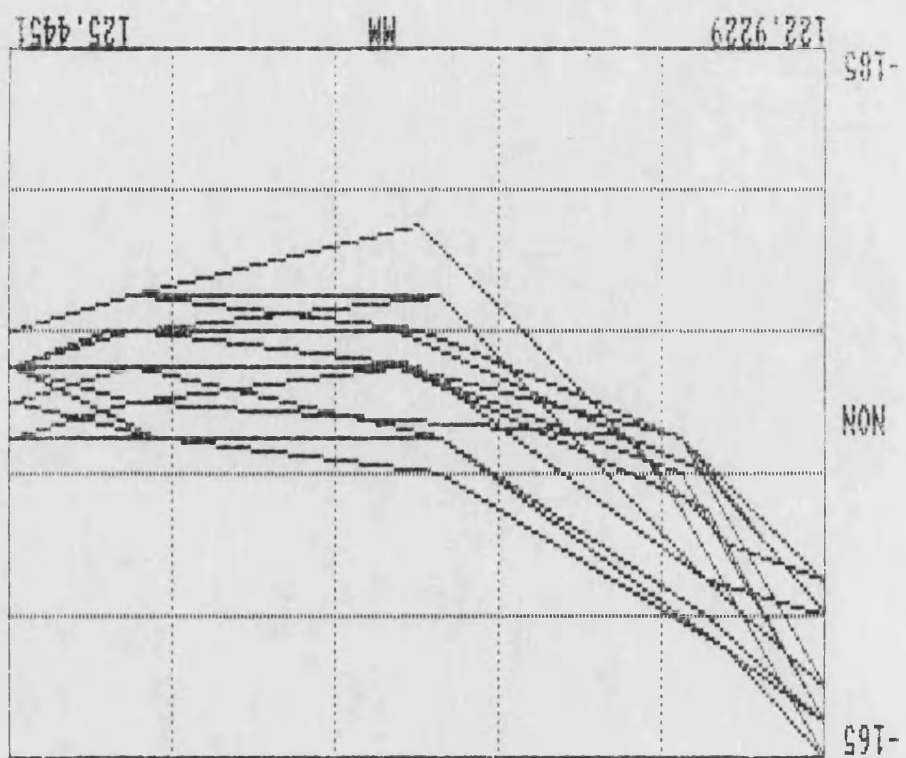
Number of Readings 200 No. Columns 8
Task Started 01/01/83 Time Task Started 00-11-00

Time STRINGS/16STRINGS/125STRINGS/125STRINGS/16LVD1
Secs 1 NON 2 NON 3 NON 4 NON 5 NON 6 NON 7 LOAD STROKE
MM

Time STRINGS/16STRINGS/125STRINGS/125STRINGS/16LVD1
Secs 1 NON 2 NON 3 NON 4 NON 5 NON 6 NON 7 LOAD STROKE
MM

02	25.0	-178	167	125.0	-177.0	-1430	-1.1475	125.0742
03	25.0	-179	164	164.0	-180.0	-1473	-1.1230	125.4431
04	25.0	-179	173	146.0	-179.0	-1475	-1.8311	125.0742
05	25.0	-180	172	147.0	-178.0	-1472	-2.0019	124.1840
06	26.0	-175	169	153.0	-180.0	-1477	-1.1816	123.2938
07	26.0	-173	183	163.0	-185.0	-1425	-19.4091	122.9229
08	26.0	-173	173	142.0	-183.0	-1424	-9.4726	123.2938
09	26.0	-177	178	133.0	-177.0	-1423	-1.2431	124.1840
10	27.0	-179	179	137.0	-177.0	-1426	-1.1475	125.0742
11	27.0	-179	170	142.0	-178.0	-1423	-1.1475	125.4431
12	27.0	-179	172	151.0	-178.0	-1423	-1.8311	125.0742
13	27.0	-179	169	142.0	-177.0	-1424	-1.9775	124.1840
14	28.0	-175	166	145.0	-181.0	-1424	-11.3037	123.2938
15	28.0	-170	158	160.0	-183.0	-1426	-9.7168	123.2938
16	28.0	-173	168	151.0	-183.0	-1426	-9.7168	123.2938
17	28.0	-178	176	146.0	-175.0	-1424	-1.2207	124.1840
18	29.0	-180	167	162.0	-179.0	-1427	-1.1475	125.0742
19	29.0	-179	165	155.0	-179.0	-1425	-1.1475	125.4431
20	29.0	-180	178	150.0	-179.0	-1424	-1.8311	125.0742
21	29.0	-179	170	153.0	-178.0	-1424	-1.9775	124.2582
22	30.0	-176	170	152.0	-180.0	-1427	-10.9619	123.3680
23	30.0	-171	158	161.0	-185.0	-1423	-19.1650	122.9971
24	30.0	-174	172	148.0	-184.0	-1427	-9.5215	123.2938
25	30.0	-175	177	154.0	-180.0	-1425	-1.2207	124.1840
26	31.0	-179	184	154.0	-178.0	-1423	-1.1475	125.0742
27	31.0	-179	178	137.0	-179.0	-1429	-1.1475	125.4431
28	31.0	-180	167	162.0	-179.0	-1425	-1.8311	125.0742
29	31.0	-180	167	156.0	-178.0	-1425	-1.9775	124.1840
30	32.0	-177	174	148.0	-179.0	-1425	-11.0107	123.2938
31	32.0	-173	177	157.0	-185.0	-1429	-19.1162	122.9971
32	32.0	-174	170	159.0	-182.0	-1428	-9.2529	123.2938
33	32.0	-175	165	156.0	-181.0	-1423	-1.2451	124.1840
34	33.0	-178	164	168.0	-177.0	-1423	-1.1475	125.0742
35	33.0	-180	168	172.0	-179.0	-1430	-1.1230	125.4431
36	33.0	-180	174	149.0	-178.0	-1429	-1.8311	125.0742
37	33.0	-180	172	150.0	-177.0	-1423	-1.9751	124.1840
38	34.0	-177	175	149.0	-179.0	-1428	-11.1328	123.2938
39	34.0	-171	161	163.0	-185.0	-1423	-19.0185	122.9229
40	34.0	-174	170	145.0	-183.0	-1430	-9.4482	123.2938
41	34.0	-176	166	167.0	-178.0	-1423	-1.2451	124.1840
42	35.0	-179	166	167.0	-179.0	-1426	-1.1475	125.0742
43	35.0	-179	178	141.0	-179.0	-1425	-1.1475	125.4431
44	35.0	-180	170	142.0	-179.0	-1423	-1.8311	125.0742
45	35.0	-181	176	146.0	-178.0	-1428	-1.9775	124.1840
46	36.0	-176	171	151.0	-180.0	-1426	-10.8154	123.3680
47	36.0	-172	163	124.0	-185.0	-1424	-18.9697	122.9229
48	36.0	-174	173	147.0	-183.0	-1424	-9.2773	123.2938
49	36.0	-178	172	147.0	-184.0	-1426	-1.2451	124.1840
50	37.0	-179	166	141.0	-180.0	-1423	-1.1475	125.0742
51	37.0	-179	167	166.0	-179.0	-1426	-1.1475	125.4431
52	37.0	-180	168	158.0	-179.0	-1430	-1.8311	125.0742

153	37.0	-180	176	147.0	-178.0	-1427	-1.9775	124.1840
154	38.0	-177	174	148.0	-180.0	-1430	-10.8642	123.3680
155	38.0	-172	153	145.0	-185.0	-1424	-18.8964	122.9229
156	38.0	-174	171	136.0	-183.0	-1423	-8.9355	123.2938
157	38.0	-178	181	161.0	-182.0	-1423	-1.2451	124.1840
158	39.0	-179	144	162.0	-180.0	-1428	-1.1475	125.0742
159	39.0	-180	165	166.0	-179.0	-1428	-1.1475	125.4431
160	39.0	-180	169	162.0	-179.0	-1423	-1.8311	125.0742
161	39.0	-181	178	147.0	-178.0	-1430	-1.9775	124.1840
162	40.0	-177	172	138.0	-180.0	-1427	-11.0107	123.2938
163	40.0	-173	159	172.0	-185.0	-1428	-18.8964	122.9229
164	40.0	-175	172	147.0	-183.0	-1425	-9.2041	123.2938
165	40.0	-179	170	145.0	-184.0	-1424	-1.2451	124.1840
166	41.0	-179	166	169.0	-181.0	-1423	-1.1475	125.0742
167	41.0	-180	168	160.0	-179.0	-1423	-1.1475	125.4431
168	41.0	-180	177	136.0	-179.0	-1424	-1.8555	125.0742
169	41.0	-182	175	150.0	-178.0	-1423	-1.9775	124.1840
170	42.0	-178	171	154.0	-180.0	-1428	-10.7177	123.3680
171	42.0	-175	180	166.0	-186.0	-1428	-18.8232	122.9229
172	42.0	-174	171	150.0	-184.0	-1429	-9.0820	123.2938
173	42.0	-182	164	131.0	-183.0	-1426	-1.2451	124.1840
174	43.0	-179	180	135.0	-179.0	-1429	-1.1475	125.0742
175	43.0	-180	173	146.0	-178.0	-1426	-1.1230	125.4431
176	43.0	-180	168	161.0	-179.0	-1429	-1.8311	125.0742
177	43.0	-182	174	163.0	-179.0	-1423	-1.9775	124.2582
178	44.0	-178	172	155.0	-180.0	-1426	-10.7422	122.9229
179	44.0	-172	169	121.0	-185.0	-1423	-18.7744	122.9229
180	44.0	-174	164	140.0	-183.0	-1423	-8.7402	123.2938
181	44.0	-184	171	152.0	-184.0	-1423	-1.2451	124.1840
182	45.0	-180	166	165.0	-179.0	-1428	-1.1475	125.0742
183	45.0	-181	168	167.0	-180.0	-1426	-1.1475	125.4431
184	45.0	-180	178	147.0	-178.0	-1428	-1.8311	125.0742
185	45.0	-181	177	152.0	-178.0	-1423	-1.9775	124.1840
186	46.0	-178	176	149.0	-180.0	-1429	-10.8866	123.2938
187	46.0	-174	161	138.0	-186.0	-1423	-18.7255	122.9229
188	46.0	-175	171	150.0	-182.0	-1424	-8.8867	123.2938
189	46.0	-185	175	144.0	-183.0	-1429	-1.2207	124.1840
190	47.0	-182	179	143.0	-179.0	-1430	-1.1475	125.0742
191	47.0	-181	169	133.0	-178.0	-1425	-1.1475	125.4431
192	47.0	-181	170	142.0	-178.0	-1423	-1.8311	125.0742
193	47.0	-181	170	160.0	-177.0	-1427	-1.9775	124.1840
194	48.0	-179	171	156.0	-180.0	-1424	-10.6445	123.2938
195	48.0	-175	174	169.0	-185.0	-1426	-8.8135	122.9229
196	48.0	-176	170	158.0	-182.0	-1425	-18.8135	123.2938
197	48.0	-184	178	154.0	-178.0	-1426	-1.2451	124.1098
198	49.0	-181	167	145.0	-180.0	-1423	-1.1475	125.0742
199	49.0	-180	168	167.0	-178.0	-1427	-1.1475	125.4431
200	49.0	-181	175	150.0	-177.0	-1425	-1.8311	125.1484

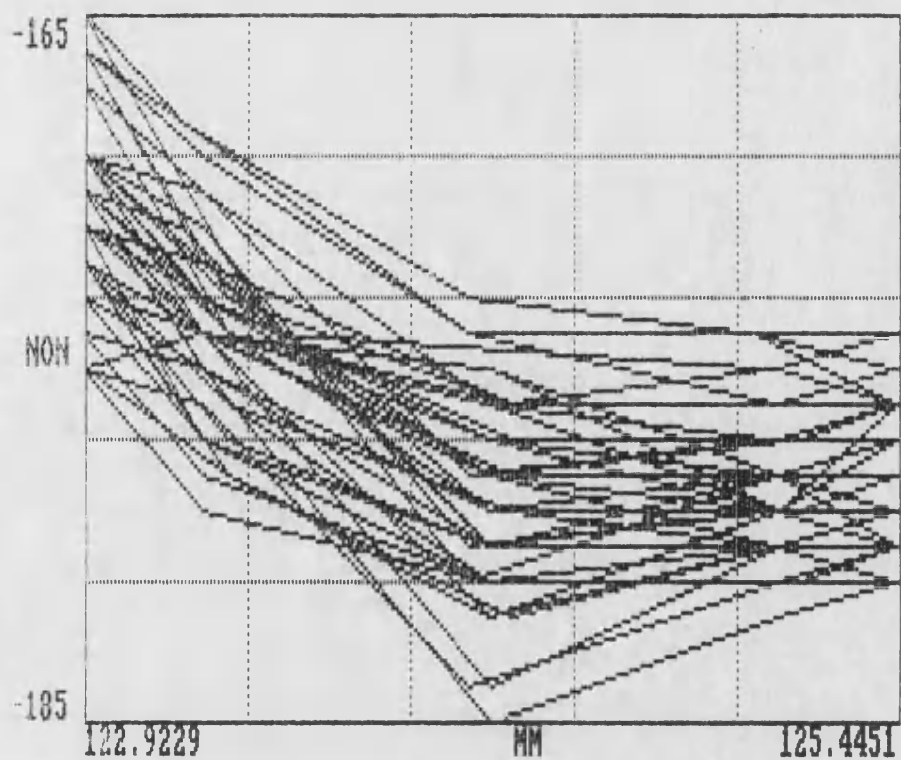


STRINGS/16 v STROKE

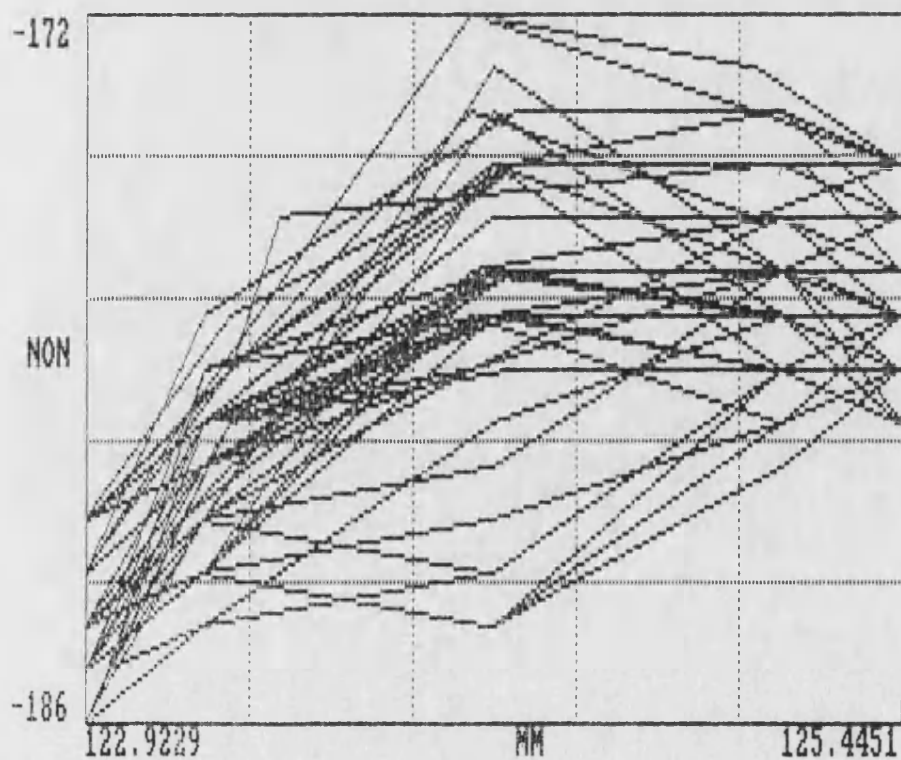


STRINGS/16 v STROKE

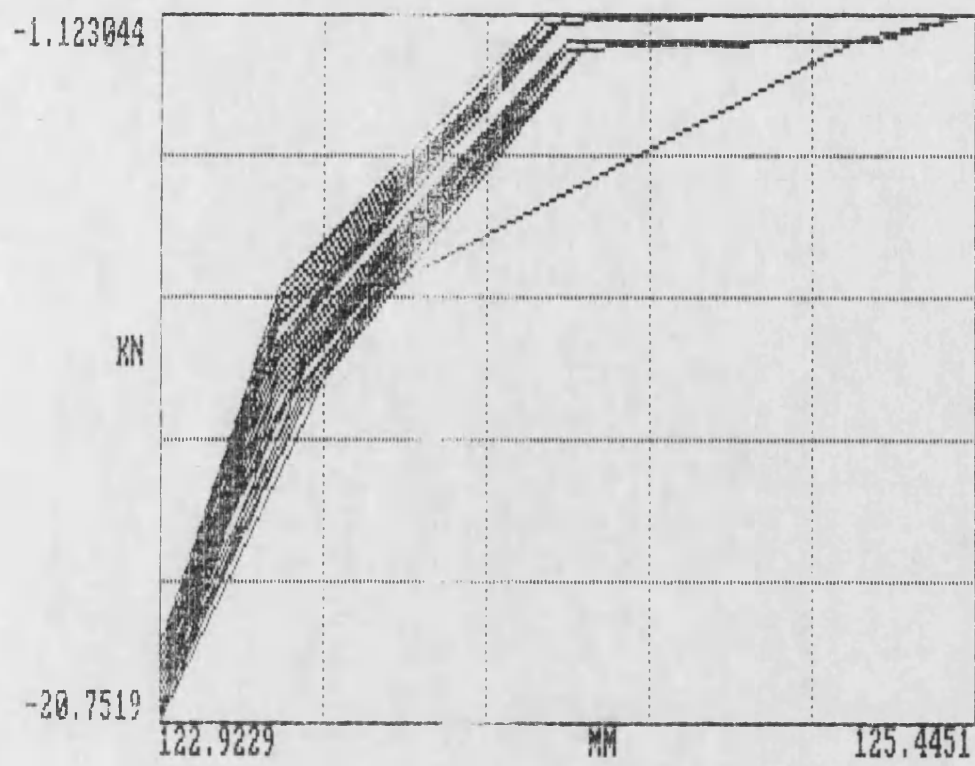
STRING5/16 v STROKE



STRING3/16 v STROKE



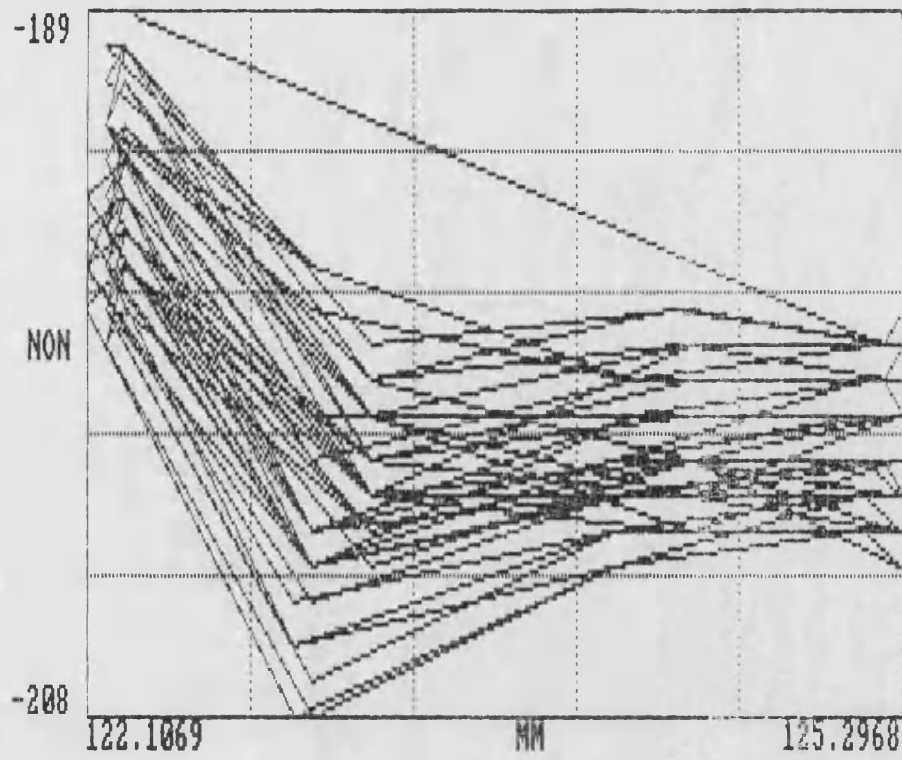
LOAD v STROKE



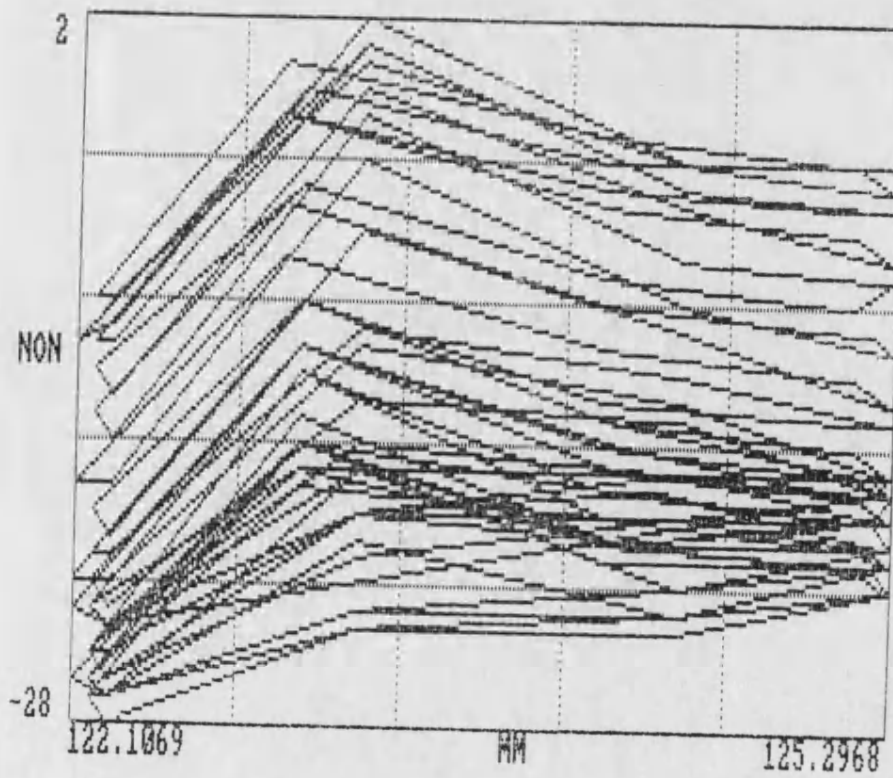
Number of Readings: 200 No. Columns: 8 Time Task Started 00-23-19									
at Task Started 01/01/83									
Time	Secs	1	2	3	4	5	6	7	8
STRINGS/16STRINGS/12STRINGS/12STRINGS/16ALVT	MIN	MIN	MIN	MIN	MIN	MIN	MIN	LOAD	STROKE
102	25.0	-195	201	166.0	-25.0	-1430	19.3068	122.1810	122.1810
103	25.0	-194	200	165.0	-24.0	-1428	19.7705	122.2552	122.2552
104	25.0	-207	205	173.0	-17.0	-1426	-1.2695	123.2196	123.2196
105	25.0	-203	202	173.0	-17.0	-1426	-1.1719	124.4808	124.4808
106	26.0	-201	202	171.0	-19.0	-1430	-1.1230	125.2968	125.2968
107	26.0	-201	201	168.0	-18.0	-1432	-1.6846	125.1484	125.1484
108	26.0	-202	203	167.0	-16.0	-1432	-1.9531	124.2582	124.2582
109	26.0	-204	203	171.0	-12.0	-1428	-2.9785	122.1049	122.1049
110	27.0	-195	200	161.0	-23.0	-1426	-19.4335	122.2552	122.2552
111	27.0	-194	199	161.0	-24.0	-1427	-14.6240	122.2552	122.2552
112	27.0	-202	205	168.0	-16.0	-1430	-1.2451	123.2196	123.2196
113	27.0	-202	202	168.0	-17.0	-1431	-1.1719	124.4808	124.4808
114	28.0	-202	202	166.0	-18.0	-1426	-1.0986	125.2968	125.2968
115	28.0	-201	201	168.0	-17.0	-1426	-1.7090	125.1484	125.1484
116	28.0	-202	206	169.0	-16.0	-1426	-1.9531	124.1840	124.1840
117	28.0	-203	204	173.0	-12.0	-1429	-2.9297	122.9971	122.9971
118	29.0	-195	203	165.0	-22.0	-1432	-19.4335	122.1810	122.1810
119	29.0	-193	199	163.0	-23.0	-1428	-14.6240	122.2552	122.2552
120	29.0	-202	207	171.0	-14.0	-1426	-1.2695	123.2196	123.2196
121	29.0	-202	205	171.0	-15.0	-1426	-1.1475	124.4808	124.4808
122	30.0	-202	205	171.0	-18.0	-1431	-1.0986	125.2968	125.2968
123	30.0	-201	201	171.0	-17.0	-1431	-1.7090	125.2226	125.2226
124	30.0	-202	203	169.0	-15.0	-1428	-1.9287	124.1840	124.1840
125	30.0	-205	202	173.0	-10.0	-1426	-2.9053	122.9971	122.9971
126	31.0	-196	200	165.0	-22.0	-1428	-19.3115	122.1049	122.1049
127	31.0	-193	197	165.0	-22.0	-1432	-14.4043	122.2552	122.2552
128	31.0	-202	204	170.0	-12.0	-1433	-1.2695	123.2196	123.2196
129	31.0	-202	203	167.0	-14.0	-1433	-1.1475	124.4808	124.4808
130	32.0	-202	202	166.0	-17.0	-1429	-1.1230	125.2968	125.2968
131	32.0	-201	201	168.0	-16.0	-1428	-1.7090	125.1484	125.1484
132	32.0	-202	205	167.0	-15.0	-1429	-1.9287	124.1840	124.1840
133	32.0	-204	203	172.0	-10.0	-1430	-2.8809	122.9971	122.9971
134	33.0	-196	204	163.0	-21.0	-1430	-19.2626	122.1810	122.1810
135	33.0	-194	200	163.0	-21.0	-1429	-1.2695	123.2196	123.2196
136	33.0	-202	206	168.0	-11.0	-1428	-1.6846	125.2968	125.2968
137	33.0	-201	206	170.0	-13.0	-1429	-1.1475	124.4808	124.4808
138	34.0	-202	205	171.0	-15.0	-1432	-1.0986	125.2968	125.2968
139	34.0	-201	202	172.0	-15.0	-1433	-1.7090	125.2226	125.2226
140	34.0	-202	203	169.0	-14.0	-1433	-1.9287	124.1840	124.1840
141	34.0	-204	202	174.0	-10.0	-1426	-2.8809	122.9971	122.9971
142	35.0	-197	202	160.0	-19.0	-1426	-19.2138	122.1810	122.1810
143	35.0	-196	199	166.0	-20.0	-1431	-14.3310	122.2552	122.2552
144	35.0	-201	205	171.0	-7.0	-1431	-1.2451	123.2196	123.2196
145	35.0	-202	204	168.0	-11.0	-1431	-1.1719	124.4808	124.4808
146	36.0	-203	203	167.0	-14.0	-1427	-1.0986	125.2968	125.2968
147	36.0	-202	203	168.0	-13.0	-1427	-1.7090	125.1484	125.1484
148	36.0	-203	204	167.0	-12.0	-1432	-1.9531	124.1840	124.1840
149	36.0	-205	203	171.0	-8.0	-1433	-2.8809	122.9971	122.9971
150	37.0	-196	203	162.0	-18.0	-1431	-19.2138	122.1049	122.1049
151	37.0	-195	200	163.0	-18.0	-1430	-14.3066	122.2552	122.2552
152	37.0	-201	208	168.0	-4.0	-1429	-1.2451	123.2196	123.2196

Number of Readings: 200 No. Columns: 8 Time Task Started 00-23-19									
at Task Started 01/01/83									
Time	Secs	1	2	3	4	5	6	7	8
STRINGS/16STRINGS/12STRINGS/12STRINGS/16ALVT	MIN	MIN	MIN	MIN	MIN	MIN	MIN	LOAD	STROKE
153	37.0	-202	206	168.0	-10.0	-1430	-1.1475	124.4808	124.4808
154	38.0	-203	206	171.0	-12.0	-1431	-1.0986	125.2968	125.2968
155	38.0	-203	202	171.0	-11.0	-1430	-1.7090	125.2226	125.2226
156	38.0	-203	204	170.0	-10.0	-1429	-1.9287	124.1840	124.1840
157	38.0	-206	202	173.0	-6.0	-1429	-2.8544	122.9971	122.9971
158	39.0	-196	199	166.0	-15.0	-1426	-19.1162	122.1810	122.1810
159	39.0	-195	200	165.0	-16.0	-1434	-14.1601	122.2552	122.2552
160	39.0	-201	206	167.0	-2.0	-1433	-1.2451	123.2196	123.2196
161	39.0	-202	204	169.0	-8.0	-1432	-1.9475	124.4808	124.4808
162	40.0	-202	204	167.0	-9.0	-1428	-1.1230	125.2968	125.2968
163	40.0	-202	202	169.0	-10.0	-1427	-1.7090	125.1484	125.1484
164	40.0	-203	205	168.0	-9.0	-1432	-1.9287	124.1840	124.1840
165	40.0	-207	204	171.0	-5.0	-1432	-2.8320	122.9971	122.9971
166	41.0	-197	204	164.0	-13.0	-1430	-19.0429	122.1810	122.1810
167	41.0	-195	201	161.0	-14.0	-1429	-14.1601	122.2552	122.2552
168	41.0	-203	207	166.0	-1.0	-1430	-1.2451	123.2196	123.2196
169	41.0	-201	206	169.0	-6.0	-1431	-1.1719	124.4808	124.4808
170	42.0	-203	205	171.0	-8.0	-1434	-1.0986	125.2968	125.2968
171	42.0	-203	203	172.0	-7.0	-1433	-1.6846	125.1484	125.1484
172	42.0	-204	204	169.0	-6.0	-1432	-1.9531	124.1840	124.1840
173	43.0	-206	206	173.0	-2.0	-1429	-2.9053	122.9971	122.9971
174	43.0	-196	201	163.0	-12.0	-1431	-14.0136	122.2552	122.2552
175	43.0	-204	205	166.0	-1.0	-1431	-1.2451	123.2196	123.2196
176	43.0	-201	204	166.0	-5.0	-1430	-1.1719	124.4808	124.4808
177	44.0	-203	204	167.0	-6.0	-1427	-1.0986	125.2968	125.2968
178	44.0	-203	203	168.0	-6.0	-1434	-1.6846	125.1484	125.1484
179	44.0	-208	206	168.0	-5.0	-1434	-1.9531	124.1840	124.1840
180	44.0	-204	204	171.0	-2.0	-1433	-2.8076	122.9971	122.9971
181	45.0	-202	205	165.0	-12.0	-1429	-18.9941	122.1049	122.1049
182	45.0	-197	205	165.0	-12.0	-1429	-13.9648	122.2552	122.2552
183	45.0	-196	201	164.0	-11.0	-1428	-1.9287	124.1840	124.1840
184	45.0	-201	207	161.0	-2.0	-1432	-1.2451	123.2196	123.2196
185	46.0	-202	205	169.0	-4.0	-1432	-1.1475	124.4808	124.4808
186	46.0	-202	205	172.0	-5.0	-1429	-1.0986	125.2968	125.2968
187	46.0	-202	203	172.0	-4.0	-1428	-1.7090	125.1484	125.1484
188	46.0	-204	204	169.0	-9.0	-1427	-1.9287	124.1840	124.1840
189	46.0	-208	203	172.0	-1.0	-1433	-2.8076	122.9971	122.9971
190	47.0	-198	201	161.0	-11.0	-1428	-18.9209	122.1810	122.1810
191	47.0	-196	199	164.0	-10.0	-1432	-13.8427	122.2552	122.2552
192	47.0	-202	204	161.0	-0.0	-1430	-1.2695	123.2196	123.2196
193	47.0	-201	203	165.0	-4.0	-1429	-1.1475	124.4808	124.4808
194	48.0	-204	205	167.0	-4.0	-1427	-1.0986	125.2968	125.2968
195	48.0	-203	205	171.0	-4.0	-1428	-1.7090	125.1484	125.1484
196	48.0	-204	206	169.0	-3.0	-1430	-2.8076	124.1840	124.1840
197	48.0	-208	204	172.0	-0.0	-1429	-2.8076	122.9971	122.9971
198	49.0	-198	205	168.0	-10.0	-1427	-18.8964	122.1810	122.1810
199	49.0	-197	204	167.0	-10.0	-1427	-13.8427	122.2552	122.2552
200	49.0	-203	207	163.0	-1.0	-1434	-1.2695	123.2196	123.2196

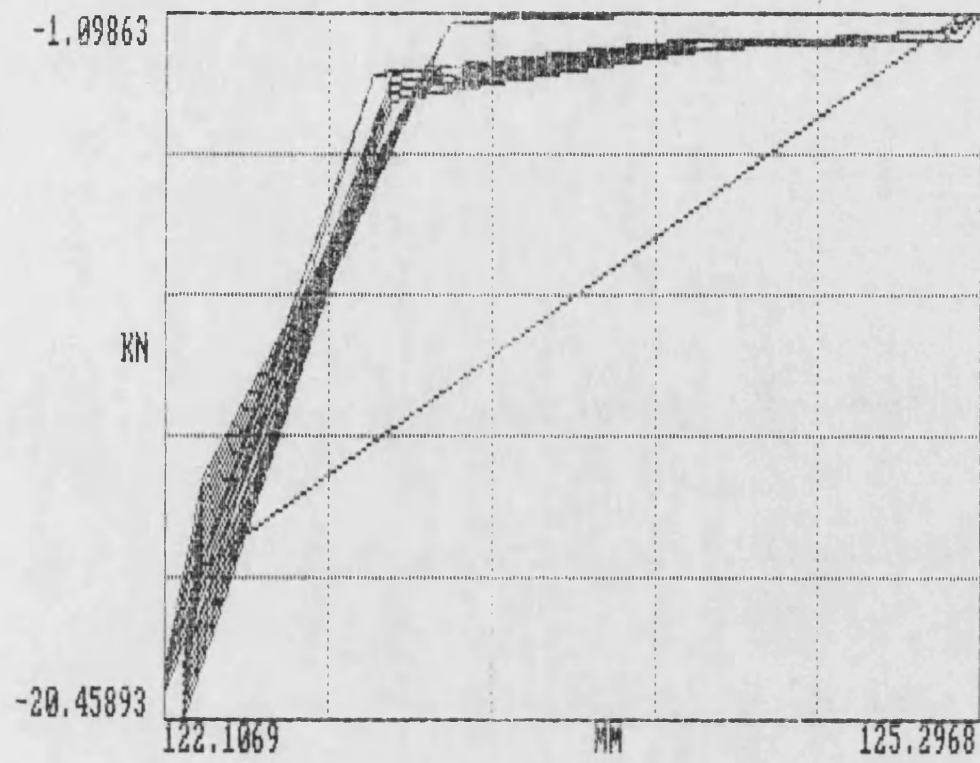
STRING5/16 v STROKE



STRING3/16 v STROKE



LOAD v STROKE



Number of Readings 200		No. Columns 8		Time Task Started 00-00-25					
Date Task Started 01/01/83									
Time	Secs	1	2	3	4	5	6	7	8
		NON	NON	NON	NON	NON	NON	NON	NON
0	0.0	0	0	16	7.0	-2.0	-1436	-1.1719	122.9229
1	0.0	2	2	-14	13.0	2.0	-1438	-1.1719	123.2938
2	0.0	1	1	-15	14.0	0.0	-1437	-1.1230	123.4422
3	0.0	1	1	-16	13.0	1.0	-1436	-1.0986	123.5905
4	0.0	0	0	-16	13.0	1.0	-1434	-1.5381	123.5905
5	0.0	0	0	-16	13.0	0.0	-1435	-1.7334	123.4422
6	0.0	1	1	-16	13.0	1.0	-1434	-1.8799	123.1454
7	0.0	0	0	-17	15.0	1.0	-1436	-1.9287	122.8487
8	0.0	0	0	-15	17.0	1.0	-1437	-1.9775	122.4778
9	1.0	-2	-2	-17	16.0	-2.0	-1439	-2.3193	122.0327
10	1.0	5	5	-15	12.0	1.0	-1437	-8.6181	121.7359
11	1.0	4	4	-17	13.0	-3.0	-1436	-15.1123	121.4392
12	1.0	7	7	-19	8.0	-7.0	-1435	-20.1904	121.1425
13	1.0	9	9	-20	5.0	-7.0	-1434	-22.5097	121.0683
14	1.0	9	9	-20	4.0	-7.0	-1435	-21.2402	121.0683
15	1.0	9	9	-20	6.0	-6.0	-1435	-16.4550	121.2167
16	1.0	7	7	-18	10.0	-5.0	-1438	-10.5957	121.4392
17	1.0	4	4	-17	12.0	-2.0	-1438	-4.6875	121.8101
18	1.0	-1	-1	-14	14.0	1.0	-1441	-1.2695	122.1810
19	2.0	3	3	-11	11.0	6.0	-1438	-1.2207	122.5520
20	2.0	0	0	-17	12.0	-1.0	-1441	-1.1475	122.9971
21	2.0	2	2	-15	12.0	2.0	-1435	-1.1475	123.2196
22	2.0	0	0	-17	13.0	0.0	-1434	-1.1475	123.4422
23	2.0	1	1	-16	12.0	1.0	-1436	-1.1230	123.5905
24	2.0	0	0	-16	14.0	1.0	-1436	-1.5625	123.5905
25	2.0	0	0	-16	14.0	1.0	-1440	-1.7578	123.4422
26	2.0	0	0	-16	16.0	1.0	-1439	-1.9043	123.1454
27	2.0	0	0	-15	15.0	1.0	-1441	-1.9287	122.8487
28	2.0	-1	-1	-14	15.0	2.0	-1439	-1.9775	122.4778
29	3.0	-1	-1	-18	13.0	-2.0	-1435	-2.3193	122.0327
30	3.0	4	4	-16	9.0	2.0	-1439	-8.5449	121.6618
31	3.0	0	0	-18	10.0	-5.0	-1440	-15.0390	121.3650
32	3.0	8	8	-20	9.0	-6.0	-1437	-20.1171	121.2167
33	3.0	9	9	-20	7.0	-7.0	-1437	-22.5097	121.0683
34	3.0	9	9	-20	8.0	-7.0	-1440	-21.0205	121.1425
35	3.0	10	10	-19	10.0	-7.0	-1439	-16.4550	121.2167
36	3.0	7	7	-19	10.0	-5.0	-1441	-10.5469	121.4392
37	3.0	4	4	-17	11.0	-1.0	-1439	-4.6147	121.8101
38	3.0	-2	-2	-15	11.0	1.0	-1438	-1.2695	122.1810
39	4.0	4	4	-12	9.0	4.0	-1439	-1.2207	122.5520
40	4.0	-1	-1	-18	14.0	-1.0	-1438	-1.1475	122.9229
41	4.0	1	1	-17	14.0	1.0	-1436	-1.1475	123.2938
42	4.0	-1	-1	-17	16.0	1.0	-1435	-1.1230	123.4422
43	4.0	1	1	-16	15.0	0.0	-1435	-1.1230	123.5905
44	4.0	-1	-1	-15	15.0	0.0	-1434	-1.5381	123.5905
45	4.0	0	0	-15	15.0	1.0	-1435	-1.7334	123.4422
46	4.0	0	0	-15	14.0	1.0	-1436	-1.8799	123.1454
47	4.0	0	0	-16	13.0	1.0	-1439	-1.9287	122.8487
48	4.0	-1	-1	-16	13.0	2.0	-1439	-1.9775	122.4778
49	5.0	0	0	-19	12.0	-2.0	-1436	-2.3193	122.0327
50	5.0	5	5	-16	11.0	1.0	-1435	-8.5449	121.6618

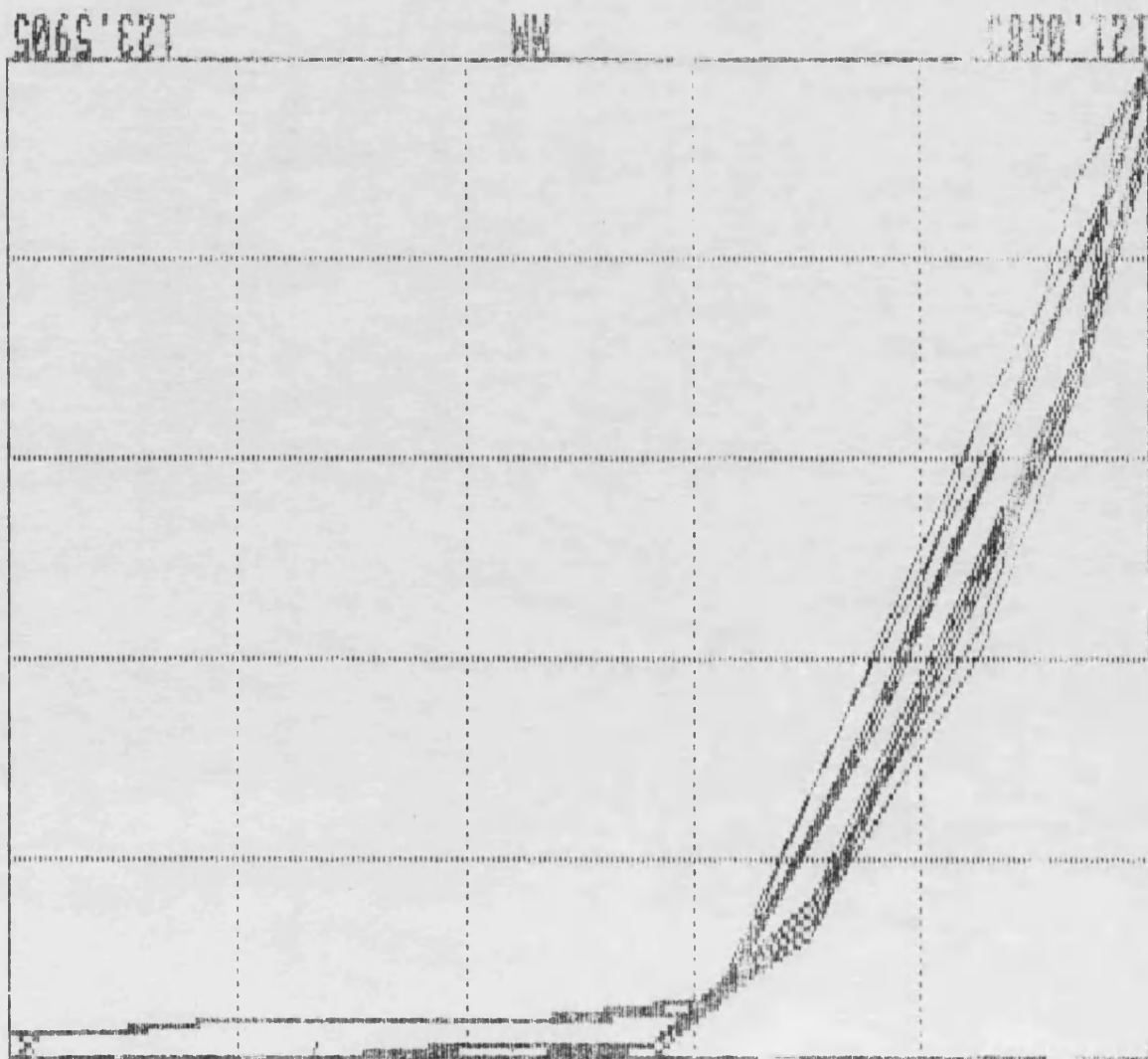
Date Task Started 01/01/83		Time Task Started 00-00-25							
Time	Secs	1	2	3	4	5	6	7	8
		1	2	3	4	5	6	7	8
51	5.0	4	4	-20	13.0	-4.0	-1434	-14.9902	121.4392
52	5.0	8	8	-19	11.0	-5.0	-1435	-20.0683	121.1425
53	5.0	9	9	-19	10.0	-7.0	-1435	-22.3876	121.0683
54	5.0	9	9	-18	7.0	-7.0	-1437	-20.9716	121.0683
55	5.0	9	9	-19	6.0	-6.0	-1437	-16.3330	121.2167
56	5.0	6	6	-20	6.0	-4.0	-1441	-10.4736	121.5134
57	5.0	4	4	-17	12.0	-1.0	-1439	-4.4922	121.8101
58	5.0	2	2	-17	14.0	1.0	-1441	-1.2695	122.1810
59	6.0	5	5	-15	10.0	4.0	-1435	-1.2695	122.5520
60	6.0	-2	-2	-17	15.0	0.0	-1442	-1.1719	122.9971
61	6.0	1	1	-16	14.0	1.0	-1435	-1.1475	123.2938
62	6.0	-1	-1	-15	16.0	1.0	-1435	-1.1230	123.5164
63	6.0	0	0	-16	15.0	1.0	-1436	-1.1230	123.5905
64	6.0	0	0	-16	13.0	1.0	-1438	-1.5381	123.5164
65	6.0	0	0	-16	13.0	1.0	-1441	-1.7578	123.4422
66	6.0	0	0	-16	13.0	1.0	-1440	-1.9043	123.1454
67	6.0	0	0	-17	13.0	2.0	-1441	-1.9287	122.8487
68	6.0	-1	-1	-16	14.0	2.0	-1438	-1.9531	122.4778
69	7.0	0	0	-19	14.0	-1.0	-1434	-2.2705	122.0327
70	7.0	4	4	-16	13.0	2.0	-1436	-8.4717	121.6618
71	7.0	3	3	-19	14.0	-5.0	-1436	-14.9170	121.3650
72	7.0	8	8	-19	11.0	-5.0	-1440	-19.9951	121.1425
73	7.0	8	8	-19	9.0	-7.0	-1439	-22.3144	121.0683
74	7.0	9	9	-20	7.0	-7.0	-1441	-20.8984	121.0683
75	7.0	9	9	-20	7.0	-7.0	-1438	-16.2597	121.2167
76	7.0	7	7	-17	12.0	-4.0	-1440	-10.1318	121.5134
77	7.0	3	3	-17	14.0	-4.0	-1439	-4.4433	121.8101
78	7.0	4	4	-15	9.0	0.0	-1439	-1.2451	122.1810
79	8.0	4	4	-16	9.0	1.0	-1435	-1.2451	122.5520
80	8.0	-2	-2	-15	18.0	0.0	-1435	-1.1475	123.2938
81	8.0	1	1	-16	15.0	0.0	-1439	-1.1475	123.5164
82	8.0	0	0	-15	15.0	1.0	-1439	-1.1475	123.5164
83	8.0	1	1	-16	13.0	0.0	-1442	-1.1719	123.5164
84	8.0	1	1	-15	14.0	1.0	-1439	-1.2695	123.5905
85	8.0	-1	-1	-15	13.0	1.0	-1436	-1.2695	123.4422
86	8.0	0	0	-15	14.0	2.0	-1441	-1.9043	123.1454
87	8.0	0	0	-15	14.0	2.0	-1441	-1.9287	122.8487
88	8.0	0	0	-15	14.0	2.0	-1441	-1.9287	122.8487
89	9.0	0	0	-15	13.0	-1.0	-1439	-2.2949	122.0327
90	9.0	4	4	-14	10.0	-2.0	-1436	-8.5937	121.7359
91	9.0	3	3	-14	1.0	1.0	-1435	-15.2099	121.3650
92	9.0	7	7	-20	8.0	-5.0	-1435	-19.8974	121.0683
93	9.0	8	8	-17	6.0	-6.0	-1439	-22.2900	121.0683
94	9.0	9	9	-15	1.0	-6.0	-1439	-20.5810	121.0683
95	9.0	9	9	-16	1.0	-7.0	-1435	-16.0644	121.2167
96	9.0	6	6	-16	8.0	-4.0	-1436	-10.1866	121.5134
97	9.0	3	3	-14	1.0	1.0	-1435	-4.0771	121.8101
98	9.0	0	0	-14	11.0	0.0	-1441	-1.2451	122.1810
99	10.0	3	3	-14	6.0	0.0	-1438	-1.2207	122.5520
00	10.0	-2	-2	-10	6.0	3.0	-1438	-1.1963	122.9971
01	10.0	0	0	-14	8.0	1.0	-1441	-1.1475	123.2938

Number of Readings 200 No. Columns 8
Time Task Started 01/01/83 Time Task Started 00-00-25

Time	Secs	1	2	3	4	5	6	7	8
02	10.0	0	-12	12.0	2.0	-1438	-1.1230	123.4422	
03	10.0	0	-14	6.0	0.0	-1439	-1.1475	123.5905	
04	10.0	0	-13	7.0	1.0	-1435	-1.5625	123.5164	
05	10.0	0	-13	12.0	1.0	-1435	-1.7578	123.3680	
06	10.0	0	-12	4.0	2.0	-1441	-1.9043	123.1454	
07	10.0	-1	-14	7.0	2.0	-1435	-1.9287	122.8487	
08	10.0	-1	-10	10.0	3.0	-1436	-1.9775	122.4036	
09	11.0	-1	-13	6.0	1.0	-1437	-2.4414	122.0327	
10	11.0	4	-13	4.0	3.0	-1439	-8.5205	121.6618	
11	11.0	3	-22	10.0	-3.0	-1437	-10.9419	121.3650	
12	11.0	7	-28	-3.0	-6.0	-1436	-20.0439	121.2167	
13	11.0	8	-30	-2.0	-6.0	-1435	-22.2167	121.0683	
14	11.0	8	-26	4.0	-6.0	-1437	-20.5810	121.0683	
15	11.0	8	-33	-3.0	-5.0	-1442	-15.7959	121.2167	
16	11.0	7	-35	3.0	-3.0	-1439	-10.0830	121.5134	
17	11.0	2	-32	7.0	1.0	-1439	-4.1504	121.8101	
18	11.0	1	-27	7.0	0.0	-1436	-1.2451	122.1810	
19	12.0	3	-28	1.0	-1.0	-1439	-1.2451	122.6262	
20	12.0	-2	-20	12.0	3.0	-1441	-1.1475	122.9229	
21	12.0	0	-28	3.0	0.0	-1438	-1.1475	123.2938	
22	12.0	0	-27	8.0	2.0	-1442	-1.1475	123.5164	
23	12.0	0	-28	12.0	1.0	-1435	-1.1230	123.5905	
24	12.0	0	-28	7.0	2.0	-1436	-1.5869	123.5164	
25	12.0	0	-29	5.0	2.0	-1441	-1.7534	123.4422	
26	12.0	-1	-26	9.0	2.0	-1435	-1.9287	123.1454	
27	12.0	-1	-24	2.0	2.0	-1438	-1.9287	122.8487	
28	12.0	-1	-25	6.0	3.0	-1437	-1.9775	122.4778	
29	13.0	1	-24	8.0	0.0	-1440	-2.2949	122.0327	
30	13.0	4	-24	1.0	1.0	-1435	-8.8135	121.6618	
31	13.0	3	-22	7.0	-3.0	-1438	-14.8925	121.3650	
32	13.0	7	-28	7.0	-5.0	-1435	-19.8486	121.1425	
33	13.0	8	-31	-2.0	-4.0	-1436	-22.1679	121.0683	
34	13.0	9	-34	-7.0	-7.0	-1436	-20.5078	121.0683	
35	13.0	8	-32	1.0	-0.0	-1435	-19.8447	121.2167	
36	13.0	5	-26	-3.0	-3.0	-1439	-9.7900	121.5134	
37	13.0	2	-33	-3.0	0.0	-1440	-4.1992	121.8101	
38	13.0	1	-30	11.0	0.0	-1440	-1.2451	122.1810	
39	14.0	1	-36	0.0	-2.0	-1436	-1.2207	122.6262	
40	14.0	-2	-32	9.0	3.0	-1435	-1.1719	122.9971	
41	14.0	0	-36	8.0	0.0	-1441	-1.1475	123.2938	
42	14.0	0	-35	5.0	2.0	-1438	-1.1475	123.5164	
43	14.0	-1	-35	3.0	1.0	-1440	-1.1230	123.5905	
44	14.0	-1	-28	6.0	1.0	-1435	-1.5869	123.5905	
45	14.0	-1	-28	9.0	2.0	-1437	-1.7534	123.3680	
46	14.0	0	-29	8.0	2.0	-1439	-1.9043	123.1454	
47	14.0	-1	-23	9.0	2.0	-1440	-1.9287	122.8487	
48	14.0	-1	-21	5.0	3.0	-1437	-1.9775	122.4036	
49	15.0	3	-19	6.0	-1.0	-1434	-2.2705	122.0327	
50	15.0	3	-18	7.0	1.0	-1437	-8.4472	121.6618	
51	15.0	2	-19	4.0	-2.0	-1435	-15.0634	121.3650	
52	15.0	8	-20	1.0	-6.0	-1435	-19.7753	121.1425	

Number of Readings 200 No. Columns 8
Time Task Started 01/01/83 Time Task Started 00-00-25

Time	Secs	1	2	3	4	5	6	7	8
153	15.0	7	-22	8.0	-6.0	-1439	-22.0458	121.0683	
154	15.0	9	-18	-3.0	-7.0	-1439	-20.5613	121.1425	
155	15.0	8	-19	-4.0	-6.0	-1436	-15.7714	121.2167	
156	15.0	5	-18	6.0	-3.0	-1439	-9.8877	121.5134	
157	15.0	2	-17	0.0	0.0	-1438	-3.8330	121.8101	
158	15.0	1	-14	6.0	1.0	-1439	-1.2451	122.1810	
159	16.0	1	-20	10.0	-3.0	-1437	-1.2451	122.6262	
160	16.0	0	-14	6.0	3.0	-1435	-1.1475	122.9971	
161	16.0	-1	-16	7.0	0.0	-1436	-1.1475	123.2938	
162	16.0	0	-13	9.0	2.0	-1435	-1.1230	123.4422	
163	16.0	0	-15	4.0	2.0	-1441	-1.1475	123.5905	
164	16.0	0	-15	8.0	1.0	-1439	-1.5625	123.5905	
165	16.0	-1	-15	10.0	1.0	-1440	-1.7334	123.3680	
166	16.0	-1	-16	3.0	2.0	-1439	-1.9287	123.1454	
167	16.0	-1	-12	6.0	3.0	-1436	-1.9287	122.8487	
168	16.0	0	-13	9.0	3.0	-1441	-1.9775	122.4778	
169	17.0	0	-15	3.0	-1.0	-1435	-2.3437	122.0327	
170	17.0	2	-13	4.0	2.0	-1436	-8.3984	121.6618	
171	17.0	3	-15	8.0	-4.0	-1435	-14.8193	121.3650	
172	17.0	7	-16	-4.0	-5.0	-1442	-19.8486	121.1425	
173	17.0	8	-16	3.0	-6.0	-1436	-22.0458	121.0683	
174	17.0	8	-17	-5.0	-6.0	-1442	-20.4345	121.0683	
175	17.0	6	-19	-4.0	-4.0	-1441	-15.4541	121.2167	
176	17.0	5	-16	-1.0	-3.0	-1439	-9.8633	121.4392	
177	17.0	2	-14	6.0	1.0	-1441	-3.9795	121.8101	
178	17.0	0	-19	5.0	-3.0	-1436	-1.2207	122.6262	
179	18.0	0	-12	9.0	4.0	-1436	-1.1719	122.9971	
180	18.0	-1	-12	6.0	2.0	-1434	-1.1475	123.2938	
181	18.0	-1	-15	8.0	-1.0	-1439	-1.1475	123.5164	
182	18.0	0	-13	6.0	0.0	-1439	-1.1475	123.5905	
183	18.0	-1	-15	10.0	1.0	-1437	-1.1475	123.5905	
184	18.0	-1	-16	4.0	3.0	-1436	-1.1475	123.5905	
185	18.0	-1	-16	6.0	3.0	-1439	-1.1475	123.5905	
186	18.0	-1	-16	6.0	3.0	-1439	-1.1475	123.5905	
187	18.0	-2	-15	8.0	2.0	-1435	-1.9043	123.1454	
188	19.0	2	-15	14.0	0.0	-1435	-2.2705	122.0327	
189	19.0	2	-12	1.0	1.0	-1438	-8.6914	121.6618	
190	19.0	3	-17	5.0	-5.0	-1439	-14.7461	121.3650	
191	19.0	2	-17	7.0	-5.0	-1436	-19.7021	121.1425	
192	19.0	6	-17	5.0	-5.0	-1436	-21.9238	121.0683	
193	19.0	8	-18	-6.0	-5.0	-1441	-20.3125	121.0683	
194	19.0	8	-17	-4.0	-6.0	-1438	-15.6005	121.2167	
195	19.0	8	-14	1.0	-3.0	-1438	-9.5703	121.5134	
196	19.0	6	-16	4.0	-4.0	-1441	-12.6005	121.8101	
197	19.0	2	-13	4.0	1.0	-1435	-3.9795	121.8101	
198	19.0	-1	-11	11.0	3.0	-1438	-1.2451	122.1810	
199	20.0	-2	-16	9.0	0.0	-1438	-1.2451	122.6262	
200	20.0	0	-10	8.0	4.0	-1437	-1.1719	122.9971	



Number of Readings: 200 No. Columns: 8

Date Task Started 01/01/83

Time Task Started 00-11-38

Time Secs	STRING5/16	STRING5/12	STRING3/12	STRING3/16	LVD1	LOAD	STROKE
1	2	3	4	5	6	7	8
0	0.0	0.3492	-0.3290	-1.0155	0.8531	-1	-4.4922
1	0.0	0.3004	-0.2935	-0.9531	0.9019	-1	-1.5381
2	0.0	0.2934	-0.2668	-1.1046	0.9019	-1	-1.4404
3	0.0	0.2794	-0.2757	-0.9710	0.9181	-1	-1.1475
4	1.0	0.2724	-0.2757	-0.9531	0.9181	-1	-1.2207
5	1.0	0.2724	-0.2757	-0.9531	0.9262	-1	-2.1484
6	2.0	0.2794	-0.2846	-0.9531	0.9181	-2	-4.1992
7	2.0	0.2934	-0.2846	-0.9620	0.9181	-2	-7.1289
8	3.0	0.3073	-0.2935	-0.9799	0.8938	-2	-10.8886
9	3.0	0.3283	-0.3113	-1.0155	0.8856	-2	-15.2099
10	4.0	0.3562	-0.3379	-1.0422	0.8531	-2	-19.8486
11	4.0	0.3842	-0.3646	-1.0511	0.8288	-2	-24.0478
12	5.0	0.4121	-0.3913	-1.0778	0.7962	-2	-27.0263
13	5.0	0.4331	-0.4002	-1.0957	0.7800	-2	-28.2226
14	6.0	0.4401	-0.3913	-1.1046	0.7800	-2	-27.8564
15	6.0	0.4401	-0.3824	-1.2026	0.7719	-1	-24.3163
16	7.0	0.4261	-0.4091	-1.0868	0.7800	-1	-19.9951
17	7.0	0.4051	-0.3913	-1.0957	0.7881	-1	-15.3320
18	8.0	0.3912	-0.3646	-1.0689	0.8125	-1	-10.7666
19	8.0	0.3562	-0.3646	-1.0511	0.8288	-1	-7.0312
20	9.0	0.3283	-0.3290	-1.0244	0.8531	-1	-4.1260
21	9.0	0.3004	-0.2935	-1.0066	0.8856	-1	-2.1484
22	10.0	0.2794	-0.2668	-0.9710	0.9100	-1	-1.2207
23	10.0	0.2724	-0.2490	-0.9620	0.9181	-1	-0.9521
24	11.0	0.2724	-0.2134	-0.9620	0.9262	-2	-1.0254
25	11.0	0.2794	-0.1601	-0.9620	0.9262	-2	-1.7822
26	12.0	0.2794	-0.1779	-0.9710	0.9262	-2	-3.7598
27	12.0	0.2864	-0.1868	-0.9710	0.9100	-2	-6.6162
28	13.0	0.3004	-0.1956	-1.0066	0.8938	-2	-10.3027
29	13.0	0.3213	-0.2134	-1.0333	0.8856	-2	-14.5752
30	14.0	0.3492	-0.2490	-1.0244	0.8612	-2	-19.1162
31	14.0	0.3772	-0.2668	-1.0778	0.8288	-2	-23.2910
32	15.0	0.4051	-0.2846	-1.0957	0.8044	-1	-26.2939
33	15.0	0.4191	-0.3024	-1.1046	0.7881	-2	-27.6855
34	16.0	0.4261	-0.3024	-1.0957	0.7881	-1	-27.3193
35	16.0	0.4261	-0.3024	-1.1046	0.7881	-1	-23.8036
36	17.0	0.4191	-0.3024	-1.1046	0.7881	-1	-19.4580
37	17.0	0.3981	-0.2846	-1.0689	0.8044	-1	-14.9414
38	18.0	0.3772	-0.2757	-1.0600	0.8206	-1	-10.4492
39	18.0	0.3492	-0.2668	-1.0422	0.8531	-1	-6.7139
40	19.0	0.3143	-0.2401	-1.0244	0.8775	-1	-3.8574
41	19.0	0.2934	-0.2223	-0.9977	0.8938	-1	-1.9287
42	20.0	0.2794	-0.2134	-0.9620	0.9181	-1	-1.0986
43	20.0	0.2724	-0.2223	-0.9531	0.9262	-1	-0.8545
44	21.0	0.2724	-0.2134	-0.9620	0.9344	-2	-0.9277
45	21.0	0.2724	-0.2223	-0.9531	0.9262	-2	-1.5869
46	22.0	0.2724	-0.2312	-0.9620	0.9262	-2	-3.4424
47	22.0	0.2794	-0.2223	-0.9799	0.9181	-2	-6.2988
48	23.0	0.3004	-0.2312	-0.9977	0.9100	-2	-9.9609
49	23.0	0.3213	-0.2579	-1.0244	0.8856	-2	-14.1357
50	24.0	0.3492	-0.2668	-1.0689	0.8612	-2	-18.6279

Number of Readings: 200 No. Columns: 8

Date Task Started 01/01/83

Time Task Started 00-11-38

Time Secs	STRING5/16	STRING5/12	STRING3/12	STRING3/16	LVD1	LOAD	STROKE
1	2	3	4	5	6	7	8
51	24.0	0.3772	-0.3024	-1.0689	0.8369	-2	-22.7783
52	25.0	0.3981	-0.3290	-1.0957	0.8206	-2	-25.8056
53	25.0	0.4051	-0.3468	-1.1046	0.7962	-1	-27.1972
54	26.0	0.4191	-0.3379	-1.0778	0.7962	-1	-26.8798
55	26.0	0.4261	-0.3468	-1.1135	0.7962	-1	-23.3886
56	27.0	0.4051	-0.3379	-1.0868	0.8044	-1	-19.1650
57	27.0	0.3912	-0.3290	-1.1046	0.8125	-1	-14.6484
58	28.0	0.3702	-0.3201	-1.0600	0.8369	-1	-10.2051
59	28.0	0.3423	-0.2935	-1.0333	0.8531	-1	-6.4941
60	29.0	0.3143	-0.2757	-1.0155	0.8775	-1	-3.6865
61	29.0	0.2864	-0.2579	-0.9977	0.9019	-1	-1.7822
62	30.0	0.2794	-0.2490	-0.9888	0.9181	-1	-0.9766
63	30.0	0.2654	-0.2401	-0.9710	0.9262	-2	-0.7568
64	31.0	0.2724	-0.2490	-0.9531	0.9262	-2	-0.8545
65	31.0	0.2654	-0.2401	-0.9531	0.9344	-2	-1.4893
66	32.0	0.2654	-0.2490	-0.9442	0.9262	-2	-3.2226
67	32.0	0.2794	-0.2490	-0.9888	0.9262	-2	-6.1279
68	33.0	0.3004	-0.2490	-1.0066	0.9100	-2	-9.7168
69	33.0	0.3213	-0.2846	-1.0244	0.8856	-2	-13.8183
70	34.0	0.3423	-0.2846	-1.0689	0.8694	-2	-18.3837
71	34.0	0.3702	-0.3201	-1.0600	0.8369	-2	-22.3388
72	35.0	0.3912	-0.3201	-1.0868	0.8206	-1	-25.4394
73	35.0	0.4121	-0.3379	-1.1046	0.8125	-1	-26.7822
74	36.0	0.4121	-0.3468	-1.1135	0.8044	-1	-26.5136
75	36.0	0.4121	-0.3468	-1.1046	0.8044	-1	-22.9736
76	37.0	0.3981	-0.3379	-1.0868	0.8044	-1	-18.9453
77	37.0	0.3842	-0.3379	-1.0689	0.8288	-1	-14.4531
78	38.0	0.3632	-0.3201	-1.0511	0.8450	-1	-10.0586
79	38.0	0.3353	-0.2935	-1.0244	0.8612	-1	-6.3476
80	39.0	0.3073	-0.2846	-1.0244	0.8856	-1	-3.5400
81	39.0	0.2864	-0.2668	-0.9977	0.9100	-1	-1.6602
82	40.0	0.2724	-0.2668	-0.9888	0.9262	-1	-0.9277
83	40.0	0.2584	-0.2668	-0.9531	0.9262	-2	-0.7324
84	41.0	0.2654	-0.2668	-0.9531	0.9344	-2	-0.8057
85	41.0	0.2654	-0.2668	-0.9531	0.9344	-2	-1.3916
86	42.0	0.2724	-0.2668	-0.9531	0.9344	-2	-3.0273
87	42.0	0.2794	-0.2668	-0.9888	0.9262	-2	-5.9814
88	43.0	0.2934	-0.2846	-1.0066	0.9100	-2	-9.4970
89	43.0	0.3143	-0.3113	-1.0066	0.8938	-2	-13.5742
90	44.0	0.3423	-0.3201	-1.0511	0.8694	-2	-18.0664
91	44.0	0.3772	-0.3290	-1.0600	0.8531	-1	-22.0214
92	45.0	0.3842	-0.3557	-1.0689	0.8288	-1	-25.0976
93	45.0	0.3981	-0.3557	-1.1046	0.8044	-1	-26.5624
94	46.0	0.4051	-0.3646	-1.1046	0.8125	-1	-26.1474
95	46.0	0.4051	-0.3735	-1.1046	0.8044	-1	-22.7783
96	47.0	0.3912	-0.3735	-1.0333	0.8206	-1	-18.7011
97	47.0	0.3772	-0.3557	-1.0778	0.8288	-1	-14.3066
98	48.0	0.3562	-0.3290	-1.0511	0.8450	-1	-9.9121
99	48.0	0.3353	-0.3113	-1.0333	0.8694	-1	-6.2500
100	49.0	0.3073	-0.2935	-1.0155	0.8856	-1	-3.4180
101	49.0	0.2864	-0.3024	-0.9888	0.9181	-2	-1.6113

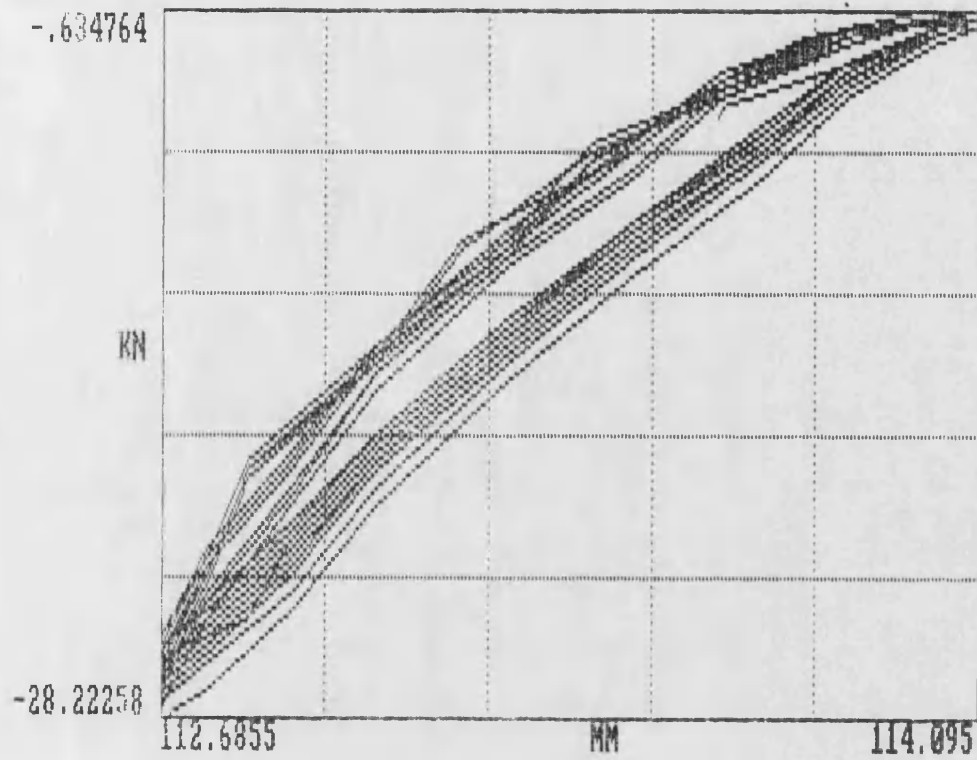
Number of Readings 200 No. Columns 8
Date Task Started 01/01/83 Time Task Started 00-11-38

Time Secs	STRINGS/16	STRINGS/12	STRING3/12	STRING3/16	LVD	LOAD	STROKE
1	2	3	4	5	6	7	8
102	50.0	0.2724	-0.2668	-0.9620	0.9262	-1	-0.8789
103	50.0	0.2654	-0.2490	-0.9710	0.9344	-2	-0.7080
104	51.0	0.2654	-0.2312	-0.9531	0.9344	-2	-0.7568
105	51.0	0.2654	-0.2134	-0.9531	0.9425	-2	-1.3428
106	52.0	0.2654	-0.1779	-0.9710	0.9425	-2	-2.9297
107	52.0	0.2794	-0.1868	-0.9888	0.9262	-2	-5.7861
108	53.0	0.2864	-0.2045	-0.9977	0.9181	-2	-9.3261
109	53.0	0.3143	-0.2223	-1.0244	0.8938	-2	-13.3789
110	54.0	0.3353	-0.2312	-1.0422	0.8694	-2	-17.7978
111	54.0	0.3562	-0.2579	-1.0600	0.8531	-1	-21.7285
112	55.0	0.3772	-0.2668	-1.1046	0.8369	-1	-24.8290
113	55.0	0.3912	-0.2935	-1.0957	0.8206	-1	-26.3183
114	56.0	0.3981	-0.2935	-1.1046	0.8125	-1	-25.9521
115	56.0	0.3981	-0.3024	-1.1046	0.8125	-1	-22.5341
116	57.0	0.3842	-0.3201	-1.0422	0.8125	-1	-18.5546
117	57.0	0.3772	-0.2846	-1.0778	0.8369	-1	-14.1601
118	58.0	0.3562	-0.2757	-1.0511	0.8450	-1	-9.8633
119	58.0	0.3283	-0.2490	-1.0244	0.8694	-1	-6.1523
120	59.0	0.3004	-0.2401	-1.0155	0.8938	-1	-3.2959
121	59.0	0.2794	-0.2401	-0.9888	0.9181	-2	-1.5381
122	60.0	0.2724	-0.2223	-0.9710	0.9262	-2	-0.8545
123	60.0	0.2584	-0.2223	-0.9531	0.9344	-2	-0.6592
124	61.0	0.2584	-0.2223	-0.9531	0.9425	-2	-0.7324
125	61.0	0.2584	-0.2134	-0.9710	0.9425	-2	-1.2939
126	62.0	0.2654	-0.2134	-0.9710	0.9425	-2	-2.8076
127	62.0	0.2794	-0.2312	-0.9888	0.9262	-2	-5.5664
128	63.0	0.2934	-0.2490	-0.9977	0.9181	-2	-9.1797
129	63.0	0.3073	-0.2579	-1.0155	0.8938	-2	-13.1103
130	64.0	0.3353	-0.2846	-1.0244	0.8775	-1	-17.6269
131	64.0	0.3562	-0.2935	-1.0600	0.8531	-1	-21.4599
132	65.0	0.3772	-0.3113	-1.0868	0.8450	-1	-24.4628
133	65.0	0.3912	-0.3113	-1.0957	0.8206	-1	-26.0009
134	66.0	0.3981	-0.3201	-1.1046	0.8206	-1	-25.7568
135	66.0	0.3981	-0.3290	-1.1046	0.8206	-1	-22.3144
136	67.0	0.3842	-0.3113	-1.1135	0.8288	-1	-18.4082
137	67.0	0.3702	-0.2935	-1.0689	0.8450	-1	-14.0136
138	68.0	0.3492	-0.2846	-1.0422	0.8531	-1	-9.7168
139	68.0	0.3213	-0.2668	-1.0244	0.8694	-1	-6.0303
140	69.0	0.3004	-0.2579	-1.0066	0.9019	-1	-3.2226
141	69.0	0.2794	-0.2401	-0.9888	0.9181	-2	-1.4648
142	70.0	0.2654	-0.2401	-0.9620	0.9262	-2	-0.8057
143	70.0	0.2584	-0.2401	-0.9531	0.9425	-2	-0.6592
144	71.0	0.2584	-0.2312	-0.9531	0.9425	-2	-0.7324
145	71.0	0.2584	-0.2312	-0.9620	0.9425	-2	-1.2451
146	72.0	0.2654	-0.2223	-0.9710	0.9506	-2	-2.7100
147	72.0	0.2724	-0.2490	-0.9888	0.9344	-2	-5.3955
148	73.0	0.2864	-0.2579	-0.9977	0.9181	-2	-9.3944
149	73.0	0.3073	-0.2668	-1.0244	0.9019	-2	-13.0127
150	74.0	0.3353	-0.2757	-1.0333	0.8856	-1	-17.3626
151	74.0	0.3562	-0.2935	-1.0689	0.8694	-1	-21.6690
152	75.0	0.3702	-0.3113	-1.0977	0.8450	-1	-25.4319

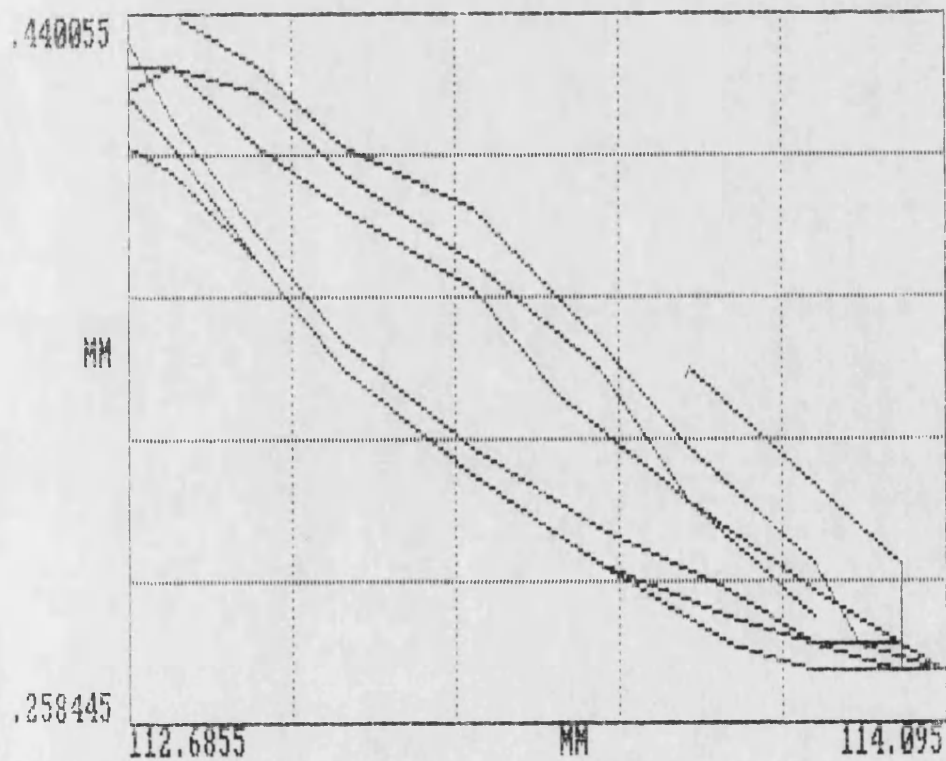
Number of Readings 200 No. Columns 8
Date Task Started 01/01/83 Time Task Started 00-11-38

Time Secs	STRINGS/16	STRINGS/12	STRING3/12	STRING3/16	LVD	LOAD	STROKE
1	2	3	4	5	6	7	8
153	75.0	0.3842	-0.3290	-1.1046	0.8369	-1	-25.7812
154	76.0	0.3912	-0.3290	-1.1135	0.8288	-1	-25.4882
155	76.0	0.3842	-0.3290	-1.0957	0.8206	-1	-22.1679
156	77.0	0.3772	-0.3201	-1.0778	0.8288	-1	-18.2373
157	77.0	0.3702	-0.3024	-1.0778	0.8450	-1	-13.8672
158	78.0	0.3492	-0.2935	-1.0511	0.8612	-1	-9.5947
159	78.0	0.3213	-0.2757	-1.0244	0.8775	-1	-5.9326
160	79.0	0.2934	-0.2579	-1.0155	0.9100	-1	-3.1250
161	79.0	0.2654	-0.2579	-0.9888	0.9262	-2	-1.4160
162	80.0	0.2654	-0.2579	-0.9710	0.9344	-2	-0.7812
163	80.0	0.2584	-0.2579	-0.9531	0.9506	-2	-0.6348
164	81.0	0.2654	-0.2490	-0.9442	0.9425	-2	-0.7568
165	81.0	0.2584	-0.2490	-0.9442	0.9506	-2	-1.1963
166	82.0	0.2724	-0.2490	-0.9531	0.9506	-2	-2.5635
167	82.0	0.2724	-0.2490	-0.9710	0.9425	-2	-5.2734
168	83.0	0.2864	-0.2668	-0.9888	0.9262	-2	-8.7646
169	83.0	0.3073	-0.2935	-0.9977	0.9100	-2	-12.8174
170	84.0	0.3353	-0.3024	-0.9710	0.8856	-2	-17.1875
171	84.0	0.3562	-0.3113	-0.9888	0.8694	-2	-21.0205
172	85.0	0.3702	-0.3290	-1.0600	0.8531	-1	-24.0478
173	85.0	0.3842	-0.3379	-1.0689	0.8450	-1	-25.5370
174	86.0	0.3912	-0.3468	-1.0868	0.8369	-1	-25.2197
175	86.0	0.3912	-0.3379	-1.0778	0.8369	-1	-21.9482
176	87.0	0.3842	-0.3379	-1.0778	0.8450	-1	-18.0908
177	87.0	0.3702	-0.3290	-1.0511	0.8531	-1	-13.7207
178	88.0	0.3423	-0.3113	-1.0244	0.8694	-1	-9.4726
179	88.0	0.3143	-0.2846	-1.0155	0.8856	-2	-5.7861
180	89.0	0.2934	-0.2668	-1.0868	0.9181	-2	-3.0517
181	89.0	0.2724	-0.2668	-0.9799	0.9344	-1	-1.3916
182	90.0	0.2654	-0.2490	-0.9710	0.9425	-2	-0.7568
183	90.0	0.2584	-0.2134	-1.0511	0.9588	-2	-0.6348
184	91.0	0.2584	-0.1956	-0.9710	0.9588	-2	-0.7324
185	91.0	0.2584	-0.1956	-0.9531	0.9506	-2	-1.1719
186	92.0	0.2584	-0.1779	-1.0422	0.9506	-2	-2.5391
187	92.0	0.2654	-0.2045	-0.9799	0.9425	-2	-5.1269
188	93.0	0.2864	-0.2045	-0.9977	0.9344	-1	-8.7402
189	93.0	0.3143	-0.2134	-1.0957	0.9181	-2	-12.6709
190	94.0	0.3353	-0.2312	-1.0422	0.8938	-1	-17.0654
191	94.0	0.3423	-0.2579	-1.0689	0.8775	-1	-20.7763
192	95.0	0.3702	-0.2490	-1.1369	0.8612	-1	-23.8575
193	95.0	0.3772	-0.2668	-1.0957	0.8450	-1	-25.4150
194	96.0	0.3842	-0.2935	-1.1046	0.8369	-1	-24.9999
195	96.0	0.3912	-0.2757	-1.1135	0.8450	-1	-21.7773
196	97.0	0.3772	-0.2757	-1.0778	0.8531	-1	-17.9199
197	97.0	0.3562	-0.2846	-1.0511	0.8612	-1	-13.5986
198	98.0	0.3423	-0.2401	-1.1274	0.8775	-2	-9.3261
199	98.0	0.3143	-0.2401	-1.0244	0.9019	-1	-5.7373
200	99.0	0.2864	-0.2223	-0.9977	0.9181	-1	-3.0029

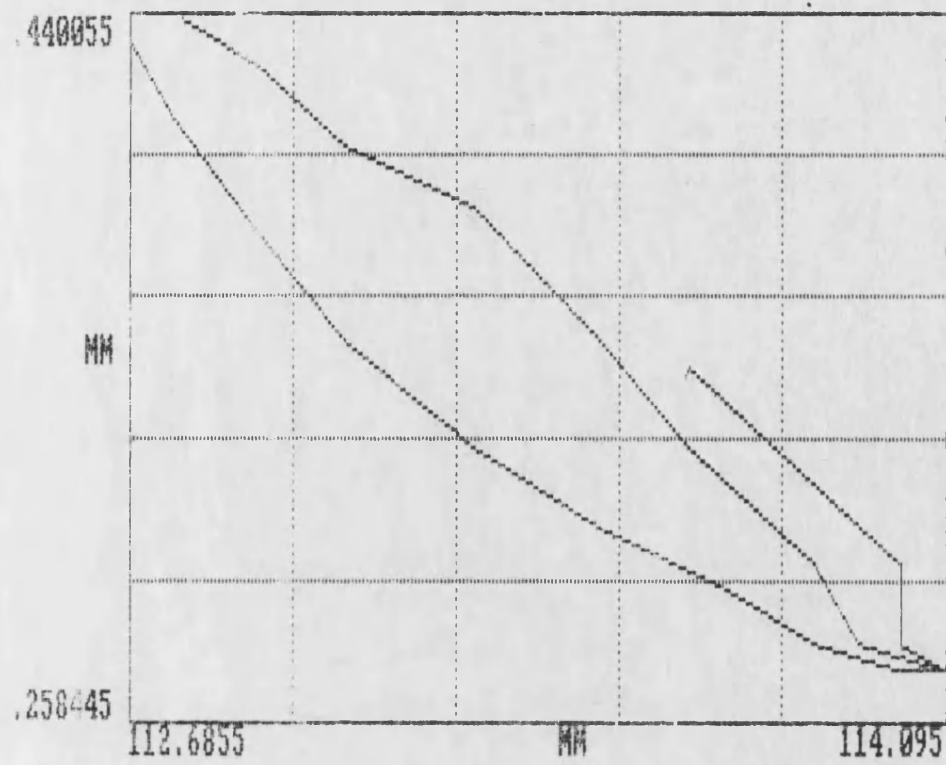
LOAD v STROKE



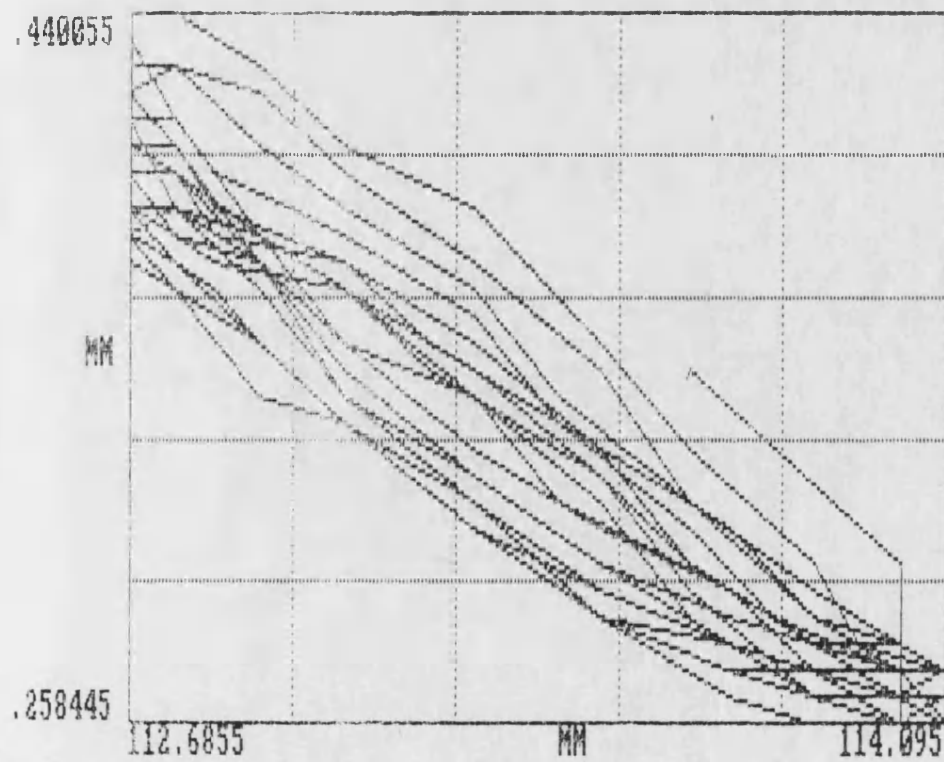
STRING5/16 v STROKE



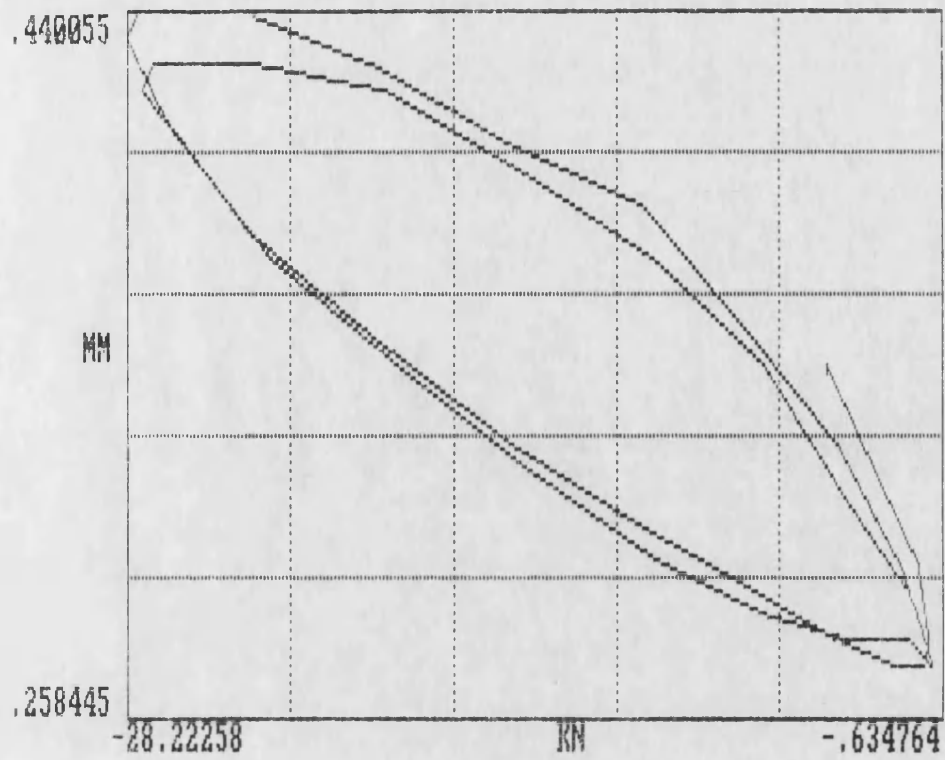
STRING5/16 v STROKE



STRING5/16 v STROKE



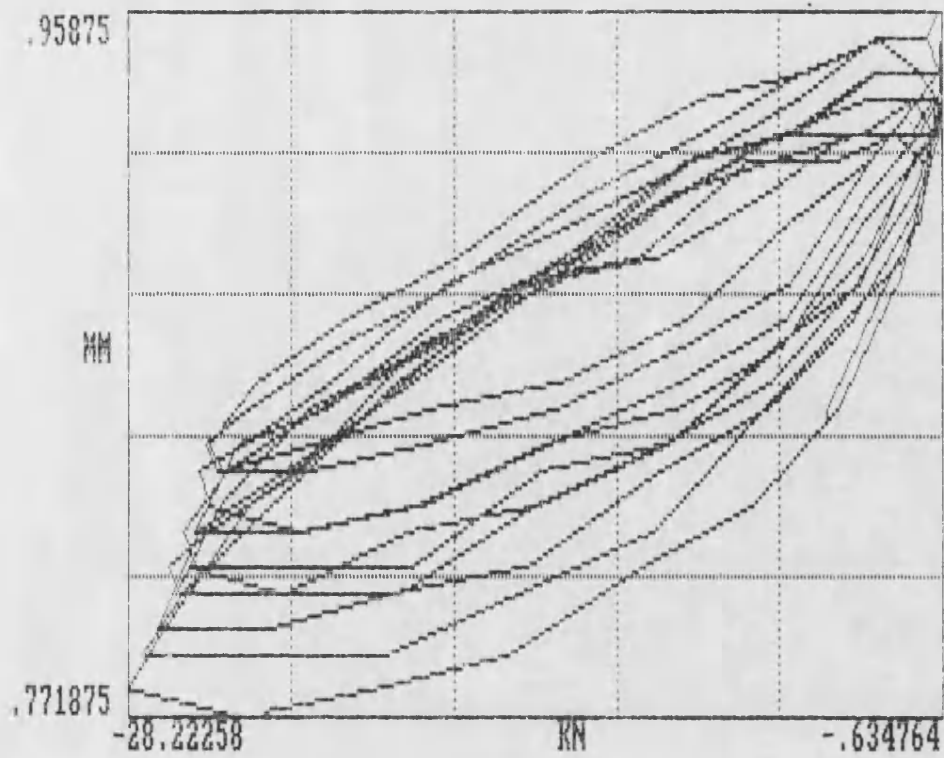
STRING5/16 v LOAD



STRING3/16 v LOAD



STRING3/16 v LOAD



STRING5/16 v LOAD

